Hampshire Country Club Planned Residential Development Village of Mamaroneck, Westchester County, New York Draft Environmental Impact Statement

M Traffic Impact Study



Hampshire Country Club Proposed Residential Development Village of Mamaroneck, NY

PREPARED FOR

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August 2017

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Executive Summary

Introduction

VHB Engineering, Surveying and Landscape Architecture, P.C. (VHB) has been retained by Hampshire Recreation, LLC to conduct a traffic impact study documenting the potential traffic impacts associated with the proposed Hampshire Country Club Planned Residential Development (PRD) in the Village of Mamaroneck, Westchester County, NY. The traffic impact study quantifies both the existing traffic conditions along area roadways surrounding the site and the projected future traffic conditions expected with and without the proposed development of the site.

This traffic study has been prepared as part of the Draft Environmental Impact Statement (DEIS) for the proposed action and is in accordance with the requirements of the New York State Environmental Quality Review Act (SEQRA) and the Scoping Document (adopted 11/18/2015) for the proposed action. This document provides a detailed description of the study methodology, analysis, and key findings.

Project Description

The Project site is located on the existing 106.2-acre Hampshire County Club property and is generally bounded by East Cove Road to the east, Eagle Knolls Road to the west, South Cove Road to the south and Old Boston Post Road to the north. The Project Site is currently developed with recreational membership club facilities, including an 18-hole golf course, clubhouse, swimming pool, tennis courts, maintenance facilities, and other support uses. The Village/Town of Mamaroneck municipal boundary line passes through the Project Site, creating a 98.9-acre portion in the Village of Mamaroneck and a smaller 7.3-acre portion within Town of Mamaroneck. The proposed PRD is to consist of 44 single-family homes and 61 townhomes. The existing 18-hole golf course would be downsized to a 9-hole course to facilitate the development of the PRD, which would have approximately 36 acres of common open space.

Access points to the site are currently provided from Cove Road and Eagle Knolls Road. A third access point from Cooper Avenue provides access to the golf course maintenance facility. These three existing access points will be modified as part of the Proposed Action. Cove Road will be relocated and will form the central corridor for the project. Eagle Knolls Road will be relocated from its existing location and will terminate in a cul-de-sac. Cooper Avenue will be extended into the Site and will intersect with Cove Road. This roadway extension is currently envisioned to be a one-way, exit only road for development residents to provide access to Boston Post Road (US Route 1) via Old Boston Post Road.



Study Locations

Per the Scoping Document, the following 7 key intersections were identified as requiring analysis:

- 1) Boston Post Road (US Route 1) and Hommocks Road/Weaver Street (signalized)
- 2) Hommocks Road and Eagle Knolls Road (unsignalized)
- 3) Orienta Avenue and East Cove Road (unsignalized)
- 4) Boston Post Road (US Route 1) and Orienta Avenue/Delancey Avenue (signalized)
- 5) Old Boston Post Road and Cooper Avenue (unsignalized)
- 6) Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road (signalized)
- 7) Fairway Lane and Orienta Avenue (unsignalized)

Existing & Future Traffic Volumes

To assess existing traffic conditions in the vicinity of the Proposed Action, peak period manual turning movement traffic volume counts were recorded at the seven study intersections in March 2016. The intersection counts included tallies of automobiles, trucks, buses, pedestrians and bicyclists. Automatic traffic recorder (ATR) 24-hour counts were also conducted for a one-week period in March 2016 on Boston Post Road, Hommocks Road and Orienta Avenue. The existing traffic volumes were grown to account for anticipated increases in background traffic by the time the project is completed, establishing the future traffic volume conditions without the proposed Project. The future traffic volumes include increases associated with 7 proposed vicinity developments anticipated to be contructed prior to the subject development.

Project-Development Traffic

Traffic anticipated to be generated by the project was forecast based on published trip generation data. Adjustments were made to the residential trips to account for the reduction of trips due to the smaller 9-hole golf course. The Proposed Action is expected to generate a total of 61 new trips during the AM peak hour, 73 new trips during the PM peak hour and 61 new trips during the Saturday peak hour.

The site-generated traffic volumes were assigned to the area roadways based on the anticipated arrival and departure patterns which were determined based on a review of the existing roadway network, existing traffic patterns and proposed access to the Project.

Existing and Future Traffic Conditions

Capacity analyses were conducted at the study intersections to assess the quality of traffic flow in the study area under existing conditions and future conditions with and without the proposed action. Under existing conditions, all intersections, with one exception, operate at acceptable levels of service. The exception is the signalized intersection of Boston Post Road and Hommocks Road/Weaver Street which currently operates at an overall level of service "E" during the AM peak hour.

In the future, with the forecast increases in traffic volumes but without the proposed residential development (No-Build conditions), there will be a slight increase in overall delays at the three



signalized intersections along Boston Post Road, generally on the order of 2 seconds or less. The levels of service will remain unchanged from those experienced under existing conditions. At the unsignalized intersections, the minor-street turning movements operate at level of service (LOS) "B" or better during each peak hour.

In the future, with the added traffic from the Proposed Action (Build conditions), there will be a slight increase in overall delays at the three signalized intersections along Boston Post Road, generally on the order of 1 second or less. The levels of service will remain unchanged from those experienced under No-Build conditions. At the unsignalized intersections, the minorstreet turning movements will continue to operate at LOS "B" or better during each peak hour with only minor increases in delay of 1.1 seconds or less.

Queuing analyses indicate that the average queues (50th percentile) experienced on the turning movements at the three signalized study intersections will be at acceptable lengths under Existing, No-Build and Build conditions. At two of the signalized intersections (Boston Post Road with Hommocks Road/Weaver Street and Boston Post Road with Richbell Road/Old Boston Post Road) some of the maximum (95th percentile) queues will exceed the storage lengths.

Conclusions

Based on the findings above, it is concluded that the proposed PRD will not have a significant adverse impact on area traffic operating conditions. Furthermore, the proposed modifications to the internal roadways including wider roads and the addition of a sidewalk along Cove Road will provide a benefit for residents on either side of the property, including those who travel back and forth to Hommocks Middle School.



1 Introduction

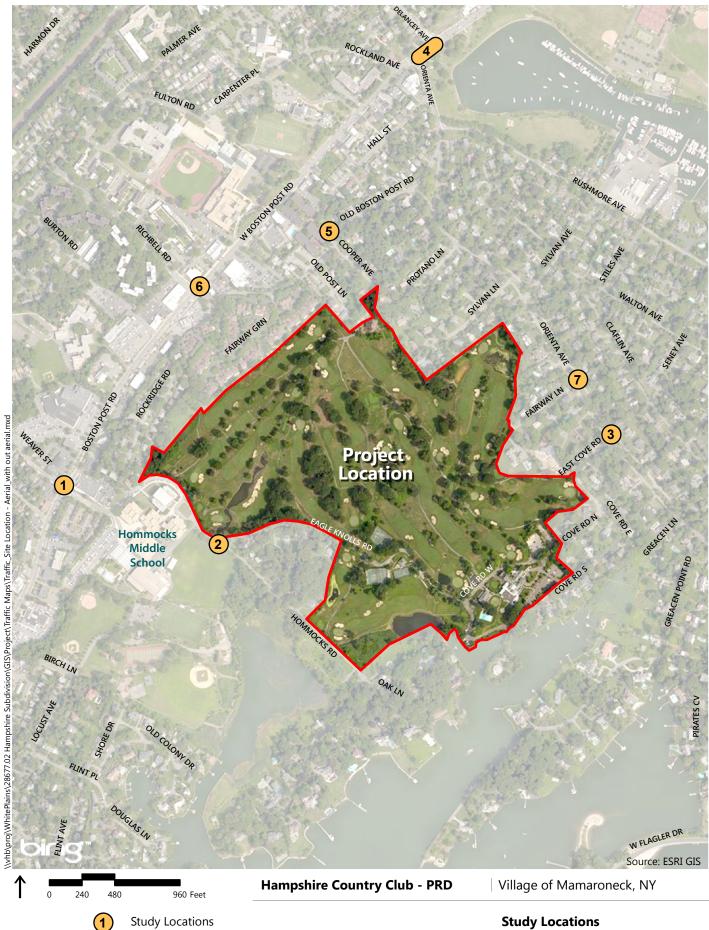
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This traffic study has been prepared as part of the Draft Environmental Impact Statement (DEIS) for the proposed action and is in accordance with the requirements of the New York State Environmental Quality Review Act (SEQRA) and the Scoping Document (adopted 11/18/2015) for the proposed action. This document provides a detailed description of the study methodology, analysis, and key findings.

Project Description

The Project site, as depicted on **Exhibit 1**, is located on the existing 106.2-acre Hampshire County Club property and is generally bounded by East Cove Road to the east, Eagle Knolls Road to the west, South Cove Road to the south and Old Boston Post Road to the north. The Project Site is currently developed with recreational membership club facilities, including an 18hole golf course, clubhouse, swimming pool, tennis courts, maintenance facilities, and other support uses. The Village/Town of Mamaroneck municipal boundary line passes through the Project Site, creating a 98.9-acre portion in the Village of Mamaroneck and a smaller 7.3-acre portion within Town of Mamaroneck. The proposed PRD is to consist of 44 single-family homes and 61 townhomes. The existing 18-hole golf course would be downsized to a 9-hole course to facilitate the development of the PRD, which would have approximately 36 acres of common open space.







Access points to the site are currently provided from Cove Road and Eagle Knolls Road. A third access point from Cooper Avenue provides access to the golf course maintenance facility. These three existing access points will be modified as part of the Proposed Action. Cove Road will be relocated and will form the central corridor for the project. Eagle Knolls Road will be relocated from its existing location and will terminate in a cul-de-sac. Cooper Avenue will be extended into the Site and will intersect with Cove Road. This roadway extension is currently envisioned to be a one-way, exit only road for development residents to provide access to Boston Post Road (US Route 1) via Old Boston Post Road.

Study Methodology

The focus of this study was to evaluate traffic flows and operating conditions on the roadways and intersections projected to be used by motorists traveling to and from the proposed development and to quantify the potential traffic impacts on these roadways and intersections.

As identified in the Scoping Document, the project study area consists of the 7 intersections listed below.

- 1) Boston Post Road (US Route 1) and Hommocks Road/Weaver Street
- 2) Hommocks Road and Eagle Knolls Road
- 3) Orienta Avenue and East Cove Road
- 4) Boston Post Road (US Route 1) and Orienta Avenue/Delancey Avenue Halstead Avenue (CR 54) and Surface Lot # 2 (MTA)
- 5) Old Boston Post Road and Cooper Avenue
- 6) Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road
- 7) Fairway Lane and Orienta Avenue

Traffic operating conditions at the study intersections were analyzed during the weekday AM and PM peak hours and the Saturday midday peak hour, representing the periods when the greatest cumulative impacts of project-related traffic are likely to occur.



Existing Conditions

Evaluation of the traffic impacts associated with the proposed Project requires a thorough understanding of the existing roadway system in the vicinity of the site. The existing conditions observed in the study area include an inventory of roadway, sidewalk and intersection geometry, traffic control devices, traffic signal timings, and the collection of traffic volumes. This information is provided in the following section.

Study Roadways and Intersections

Boston Post Road, designated as US Route 1, is a north-south urban principal arterial under the jurisdiction of the New York State Department of Transportation (NYSDOT). It runs west of the project site and provides two travel lanes in each direction with additional turn lanes at key intersections. The roadway is relatively straight and level with horizontal radii of generally 1,100 feet or greater and vertical grades of two percent or less.

Within the study area, travel lanes measure 10 to 11 feet wide and concrete curbs and sidewalks are provided along each side of the roadway. The sidewalk varies in width from 5 feet to 15 feet. The pavement is in generally fair to good condition with some surface distress. Parking is permitted, with some restrictions, along the east (northbound) side of Boston Post Road from a point just north of the intersection with Old Boston Post Road/Richbell Road to Rockland Avenue. Along the southbound side of the road, parking is permitted between Orienta Avenue and the northern driveway to Mamaroneck High School. The posted speed limit on this section of roadway is 30 miles per hour (mph)

A 2016 Automatic Traffic Recorder (ATR) count on Boston Post Road near Mamaroneck High School indicates a daily traffic volume of 19,320 on weekdays and 18,549 on Saturdays.

Hommocks Road, is a local road which runs east from Boston Post Road and serves the Hommocks Middle School, Hommocks Pool and Ice Rink and the residences further to the east. The western portion of the road is in the Town of Mamaroneck and is posted with the 30 mph Town speed limit. The eastern portion of the road is in the Village of Mamaroneck. Hommocks



Road provides one travel lane measuring 11 to 12-feet wide in each direction. The roadway is generally level with grades of one percent or less. Hommocks Road has an "S" curve near the Middle School; otherwise, the roadway is generally straight within the study area.

A sidewalk is provided along the south side of the road from Boston Post Road to and extending along the frontage of the middle school with sidewalk widths ranging from 5 feet to 10 feet. On the north side of the roadway, a sidewalk is provided between Boston Post Road and the Middle School main driveway with widths varying from 5 feet near the Middle School to 20 feet adjacent to Walgreens. Except for an area along the south side of the road in front of the Middle School, which permits one-hour parking on weekdays, there is no on-street parking. The roadway's asphalt pavement is in fair condition.

An Automatic Traffic Recorder count in indicated that the average weekday traffic volume on Hommocks Road, just north of Eagle Knolls Road, is 708 vehicles.

Weaver Street, designated as NYS Route 125, is a State principal arterial roadway that connects White Plains to the north to Boston Post Road in Mamaroneck to the south. Within the study area, Weaver Street provides two 12-foot travel lanes and has a posted speed limit of 30 mph. There are areas of the roadway with horizontal curves, with the sharpest curve in the study area located near Howell Avenue and having a radius of 425 feet. As it approaches Boston Post Road, Weaver Street has a two percent downhill grade.

Sidewalks ranging in width from 4-feet to 8-feet are provided on both sides of the road in the vicinity of its intersection with Boston Post Road. Parking is prohibited on both sides of the roadway and the pavement is in generally good condition.

The NYSDOT count on Weaver Street shows a 2015 AADT estimate of 8,755 vehicles.

Eagle Knolls Road is a local public roadway between its terminus at Hommocks Road and extending to the east to the Proposed Action's property line. Within the site, Eagle Knolls Road is a private roadway. The western portion of the roadway is in the Town of Mamaroneck and the eastern portion is part of the Village of Mamaroneck. Eagle Knolls Road provides one 10 to 11-foot travel lane in each direction. The pavement in the public portion of the roadway is in fair condition; while the pavement within the private section is in poor condition.

Sidewalks are not provided along Eagle Knolls Road and public parking is not permitted as "No Parking" signs are posted on the private portion of the road.

East Cove Road is a private road and connects Orienta Avenue to private residences and the Hampshire Country Club. It provides one 10-foot travel lane per direction with varying pavement conditions. Between its intersection with Orienta Avenue and the entrance to the Hampshire County Club property, the pavement is in generally fair to good condition. Within the Country Club property, the pavement is in fair to poor condition. Sidewalks are not provided and parking is not permitted on the portion of the roadway within the Hampshire Country Club property.

The roadway has generally level terrain with grades of two percent or less. The horizontal curvature of East Cove Road is generally straight with some curves; the sharpest curve is located approximately 300 feet to the west of Orienta Avenue and has a radius of 75-feet.



Orienta Avenue is a collector roadway that extends from Boston Post Road to Flagler Drive and is under the jurisdiction of the Village of Mamaroneck. A 15-foot wide service road is provided to the east of Orienta Avenue, in the area between Bleeker Avenue and Protano Lane. The service road is also designated as a bike path for use by pedestrians and bicyclists. Orienta Avenue provides two 10-foot travel lanes in each direction and has a posted speed limit of 25 mph. Sidewalks are provided in the section between Boston Post Road and Rushmore Avenue, between Old Boston Post Road and the service road and between the service road and Bleeker Avenue. Parking is prohibited on both sides of the roadway and the pavement is in generally fair to good condition.

A 2016 Automatic Traffic Recorder (ATR) count on Orienta Avenue to the north of Rushmore Avenue indicates a daily traffic volume of 6,818 on weekdays and 5,682 on Saturdays. Further to the east of this location, a NYSDOT ATR count on Orienta Avenue near Fairway Lane estimates an average daily traffic volume of 3,052 vehicles.

Delancey Avenue is a two-lane, 30-foot wide local roadway extending from Boston Post Road through a residential area to its terminus near the Metro-North Railroad tracks. The pavement is generally in good condition. Within the study area, parking is permitted along the north side of Delancey Avenue. Sidewalks, measuring 4-feet wide, are provided on both sides of the road between Boston Post Road and Palmer Avenue; sidewalks are not provided to the west of Palmer Avenue. Truck traffic is not permitted along Delancey Avenue.

Delancey Avenue has a 7 percent decrease in elevation traveling from Munro Avenue to Boston Post Road. Elsewhere the roadway is fairly level. The horizontal alignment of the roadway is relatively straight.

Cooper Avenue is a two-lane local road extending a short distance through a residential area from Old Boston Post Road to its terminus at the driveway to the Hampshire Country Club's maintenance facility. The roadway width varies from 16 feet to 18 feet and parking is permitted on the east side of the road. The pavement is in poor condition with obvious signs of surface distress. Traveling from Old Boston Post Road, the elevation decreases approximately 5 percent. The horizontal roadway alignment is generally straight. Although there is no posted speed limit, the Village speed limit of 30 mph would be in effect.

Fairway Lane is a two-lane local road extending from Orienta Avenue through a small residential area to its terminus in a cul-de-sac. The roadway width varies from 15 feet to 18 feet and parking is permitted on both sides of the road. The pavement is in fair condition with some signs of surface distress. Traveling from Orienta Avenue to the cul-de-sac, the vertical elevation decreases approximately 3 percent. The roadway has a straight horizontal alignment. There are no sidewalks along Fairway Lane. Although there is no posted speed limit, the Village speed limit of 30 mph would be in effect.

Old Boston Post Road is a one-lane, local road that provides one-way travel in the southbound direction from Orienta Avenue in the north to its terminus at Boston Post Road (US Route 1), opposite Richbell Road to the south. The roadway width varies from 20 feet to 33 feet and parking is permitted on the west side of the road in some areas. Old Boston Post Road has a posted speed limit of 25 mph and the pavement is in generally good condition. A sidewalk is



provided on the west side of the road across the frontage of the Orienta Gardens apartment complex. A 6-foot striped pedestrian walkway is provided on the eastern edge of the road starting at the Old Boston Post Road Cut-off near Orienta Avenue and continuing to the McDonald's exit driveway, near Boston Post Road.

Old Boston Post Road has a 2.6 percent increase in elevation traveling from Orienta Avenue to Old Post Lane. Between Old Post Lane and Boston Post Road, the elevation decreases by 1.5 percent. The horizontal curvature of Old Boston Post Road is generally straight with some curves; the sharpest curves are located near Fairway Green and near the roadway terminus at Boston Post Road.

Descriptions of the 7 study locations are provided below.

1) Boston Post Road (US Route 1) and Hommocks Road/Weaver Street

Boston Post Road provides two through lanes and an exclusive left turn lane in each direction at this signalized, four-way intersection. The eastbound Weaver Street and westbound Hommocks Road approaches each provide an exclusive left turn lane, a shared through/right turn lane and one receiving lane. Crosswalks and pedestrian displays are provided on each leg and the intersection is controlled by a multi-phase traffic signal, which includes a protected phase for the left turn movements on Boston Post Road and a separate, actuated pedestrian-only phase.

2) Hommocks Road and Eagle Knolls Road

The unsignalized intersection of Hommocks Road & Eagle Knolls Road is a threelegged T-intersection. One lane per direction is provided on each roadway. The intersection is controlled by stop signs on each approach.

3) Orienta Avenue and East Cove Road

The unsignalized intersection of Orienta Avenue with East Cove Road is a three-legged T-intersection. Each roadway provides one approach lane and one receiving lane. Stop signs are provided on each approach to control traffic.

4) Boston Post Road (US Route 1) and Orienta Avenue/Delancey Avenue

Boston Post Road provides two through lanes in each direction at this signalized, fourway intersection. Delancey Avenue and Orienta Avenue are offset from each other by 130 feet. Delancey Avenue forms the eastbound approach and provides a left turn lane and a right turn lane and one receiving lane. At Delancey Avenue, pedestrian crosswalks are provided on the north and west legs of the intersection. The westbound Orienta Avenue approach consists of exclusive left turn and right turn lanes and one receiving lane. At Orienta Avenue, pedestrian crosswalks are provided on the south and east legs of the intersection. The intersection is controlled by a four-phase traffic signal.



5) Old Boston Post Road and Cooper Avenue

The unsignalized intersection of Old Boston Post Road and Cooper Avenue is a threelegged T-intersection. Old Boston Post Road is a one-way roadway in the southbound direction with one travel lane. Cooper Avenue provides one left-turn lane. The intersection is controlled by a stop sign on the Cooper Avenue approach. A sidewalk is provided on the west side of Old Boston Post Road along the frontage of the Orienta Gardens apartment complex. Along the east side of the Old Boston Post Road, there is a striped pedestrian lane. Crosswalks are not provided at this intersection.

6) Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road

Boston Post Road provides two through lanes in each direction and an exclusive left turn lane in the northbound direction at this signalized, four-way intersection. Old Boston Post Road is a one-way westbound roadway with an exclusive left-turn lane and a shared through/right-turn lane. The eastbound Richbell Road approach has one leftturn lane and one right-turn lane. Pedestrian displays and crosswalks are provided on each leg. The intersection is controlled by a multi-phase traffic signal, which includes a protected phase for the northbound left turn movement on Boston Post Road and a separate, actuated pedestrian-only phase.

7) Fairway Lane and Orienta Avenue

The unsignalized intersection of Orienta Avenue with Fairway Lane is a three-legged Tintersection. Each roadway provides one approach lane and one receiving lane. A Stop sign is provided on the Fairway Lane approach. There are no sidewalks or pedestrian crosswalks at this intersection.

Existing Pedestrian Crossings

Sidewalks are provided connecting all of the businesses on Boston Post Road between Hommocks Road/Weaver Street and Orienta Avenue/Delancey Avenue. Signalized crossings of Boston Post Road are provided at Hommocks Road/Weaver Street, Richbell Road/Old Boston Post Road, the High School driveway and Orienta Avenue/Delancey Avenue. All of the intersections were observed to be properly marked to accommodate pedestrians and appeared to be functioning safely. Crossing guards were provided at the intersections of Boston Post Road with Hommocks Road/Weaver Street and with Richbell Road/Old Boston Post Road.

Sidewalks are provided on both sides of Hommocks Road from Boston Post Road to the driveway to the school's main parking lot where there are unsignalized crosswalks. These crosswalks are staffed by a crossing guard during morning and afternoon school dismissal periods. East of the parking lot driveway, a sidewalk continues on the school side of Hommocks Road all the way to the school's rear driveway, allowing students complete access to the campus from Boston Post Road without having to walk in the street.



Public Transit

The site is afforded convenient access to public transit, including rail and bus service. The MTA's Metro-North Railroad's New Haven line runs parallel with Boston Post Road and has two stations in proximity to the project site, the Mamaroneck and Larchmont rail stations. The New Haven line provides service between Grand Central Terminal in New York City and New Haven, CT. Connections to Amtrak service are also available along the New Haven line at the New Rochelle and Stamford, CT stations. There are 91 Metro North trains each weekday on the New Haven line between New York City and the Mamaroneck and Larchmont stations (46 southbound trains, 45 northbound trains). On weekends, there are 75 trains on Saturdays (37 southbound; 38 northbound) and 63 trains on Sundays (31 southbound; 32 northbound).

Westchester County runs the Bee-Line Bus Service within the study area. Bus route #70, also known as the Bonnie Briar Commuter, is the only route that operates in vicinity of the Proposed Action. Route #70 provides weekday service that operates in a loop with the starting and ending points at the Larchmont train station. Route #70 travels along Boston Post Road between Weaver Street and Richbell Road and operates 4 buses during the morning peak commuter period and 7 buses during the PM peak period. At the Larchmont station, connections can be made to other Bee-Line buses (#61, #66, and #71).



Map indicating Bee-Line Bus routes within the study area



Existing Traffic Data

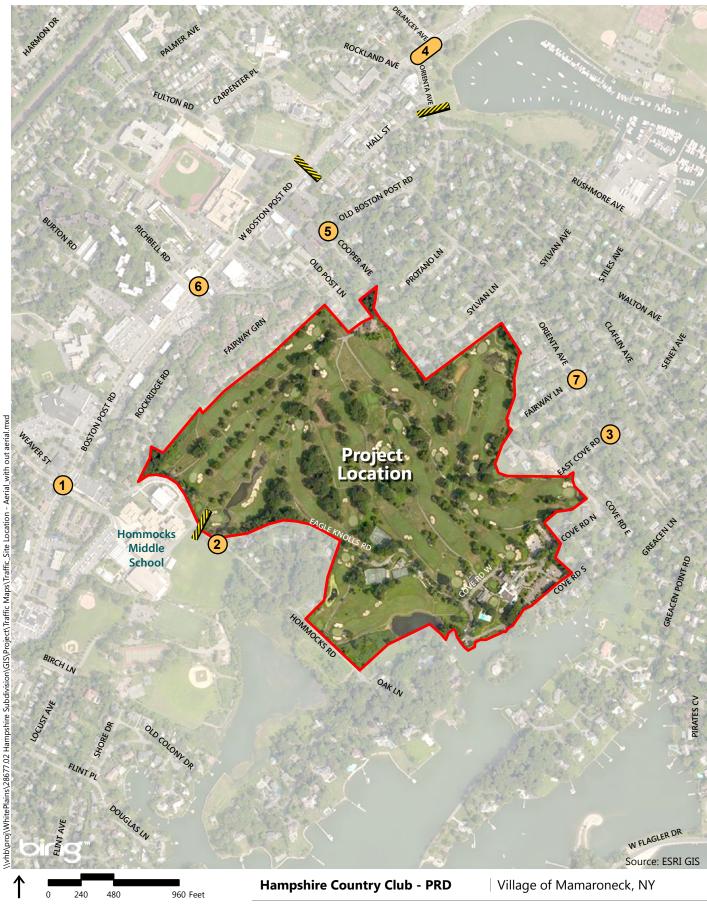
To assess existing traffic conditions in the vicinity of the Proposed Action, peak period manual turning movement traffic volume counts were recorded at the seven study intersections in March 2016. The intersection counts included tallies of automobiles, trucks, buses, pedestrians and bicyclists. Automatic traffic recorder (ATR) 24-hour counts were also conducted for a one-week period in March 2016 on Boston Post Road, Hommocks Road and Orienta Avenue. The ATR counts collected traffic volumes and vehicle classifications (automobiles, trucks and buses). The manual and ATR count locations are shown on **Exhibit 2**.

In consultation with Village planning staff, the manual counts were recorded during a typical weekday AM peak period (7:00 to 9:15 AM) and a typical weekday PM peak period (2:00 to 6:15 PM) which encompassed the peak arrival and departure periods at the Hommocks Middle School. Manual counts were also conducted in March 2016 during a typical Saturday midday peak period (11:00 AM to 1:00 PM). All counts were conducted during periods with scheduled activities at the Hommocks Park Ice Rink (house league hockey games, group skating lessons or public skating sessions) and Hommocks Pool (early morning swim, open swim, swim lessons or lifeguarding). The count sheets are provided in the Appendix.

The traffic counts were tabulated and peak hour factors (PHF) were calculated and then applied to the volumes to identify the hour within the weekday and Saturday count periods which had the greatest peak-hour-factored volumes. The hour with the highest factored volumes was chosen for analysis. The peak hours are identified as 7:30 to 8:30 AM, 3:45 to 4:45 PM and 11:45 AM to 12:45 PM for the weekday AM, PM and Saturday midday periods, respectively. The existing peak hour volumes were compared to the ATR counts to confirm their validity and were balanced and increased as needed to provide a conservative approach. The Existing peak hour traffic volumes are shown on **Exhibits 3 and 4**.

A review of the exhibits indicates that overall, the AM, PM and Saturday peak hour volumes are similar. The Saturday peak hour volumes are slightly higher (from 0.4 to 0.9 percent higher) than the AM and PM peak hour volumes, although AM peak hour volumes tend to be more concentrated around the start of the school day at the Hommocks Road school and the high school.

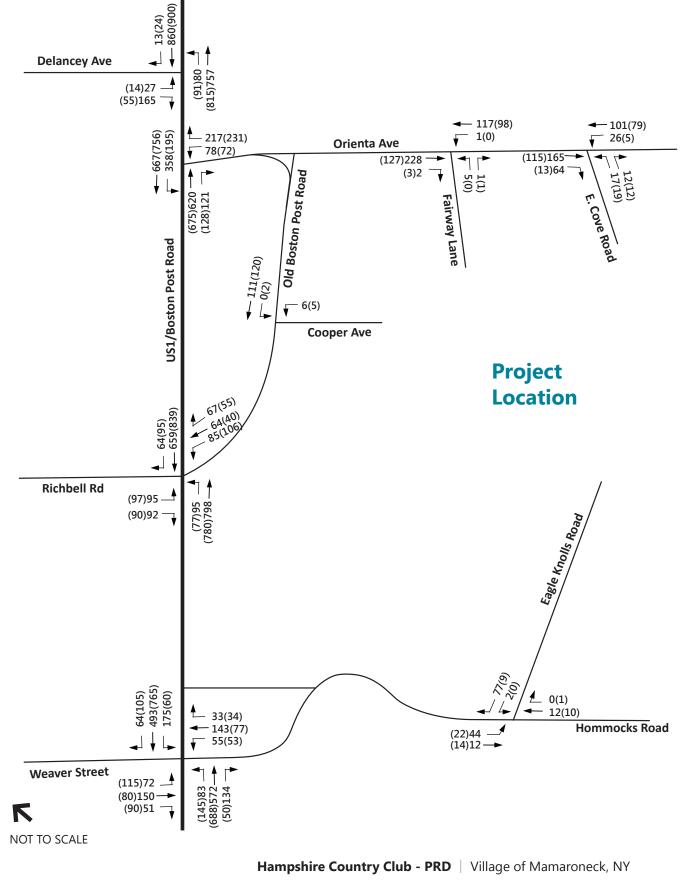




1 Intersection Manual Count Location

Count Locations

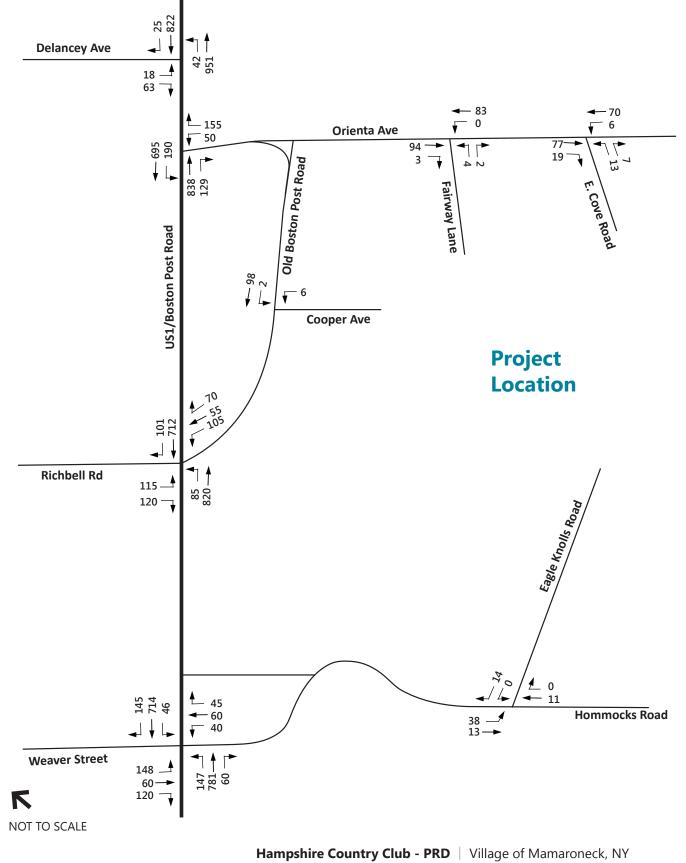




00= AM Peak Hour

Existing Weekday Peak Hour Traffic Volumes





Existing Saturday Peak Hour Traffic Volumes



Pedestrian and Bicyclist Activity

The intersection counts included tallies of pedestrians and bicyclists, which are summarized in **Table 1**, below.

Table 1: Summary of Pedestrian and Bicyclist Peak Hour Counts

Intersection	AM Peds/Bikes	PM Peds/Bikes	Sat Peds/Bikes
Boston Post Road (US Route 1) and Hommocks Road/Weaver Street	245/6	64/4	74/9
Hommocks Road and Eagle Knolls Road	11/10	4/6	16/0
Orienta Avenue and East Cove Road	2/4	1/6	13/1
Boston Post Road (US Route 1) and Orienta Avenue/Delancey Avenue	24/6	31/0	43/11
Old Boston Post Road and Cooper Avenue	16/0	5/0	19/0
Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road	106/5	80/0	51/12
Fairway Lane and Orienta Avenue	2/6	2/2	2/2

As indicated in the table, pedestrian activity was at its greatest during the AM peak hour, with the highest concentration of pedestrians at the intersection of Boston Post Road and Hommocks Road/Weaver Street. At this intersection, a total of 245 pedestrians were counted during the AM peak hour, the majority of which were students walking to Hommocks Middle School. A total of 64 pedestrians were counted at this intersection during the PM peak hour and 74 pedestrians were observed during the Saturday peak hour. At the Boston Post Road intersection with Old Boston Post Road and Richbell Road, a total of 106 pedestrians were counted during the AM peak hour, 80 during the PM peak hour and 51 during the Saturday peak hour. All other study intersections had fewer pedestrians with the least amount observed at the Orienta Avenue intersections with East Cove Road and Fairway Lane. Only a handful of bicyclists (12 or fewer) were observed at any study location, with the highest number (11 and 12) occurring during the Saturday peak hour at the intersections of Boston Post Road with Old Boston Post Road and Orienta Avenue/Delancey Avenue.

Traffic Circulation Patterns on and surrounding the Site

Primary access to the project site is currently provided from Eagle Knolls Road and East Cove Road; access to the golf course maintenance area is provided through Cooper Avenue. Vehicles from the south generally approach the site via Hommocks Road and Eagle Knolls Road. Vehicles from the north generally approach the site via Orienta Avenue and East Cove Road. Hommocks Road provides access to the Hommocks Middle School and the residences on Eagle Knolls Road, Hommocks Road and Oak Lane. Orienta Avenue provides access to the residences and businesses to the north of the site. Old Boston Post Road provides access to the residences to the west of the site.

Within the Hampshire Country Club's property, Eagle Knolls Road and East Cove Road are private roads. A review of the existing traffic volumes shown on Exhibits 3 and 4 indicates that these roadways are used as a short cut by traffic between Orienta Avenue and Hommocks Road,



most notably on weekday mornings when some residents to the east of the site travel back and forth to the school.

Hommocks Middle School

The Hommocks Middle School campus also includes the Hommocks Park Ice Rink and Hommocks Pool. VHB observed vehicular, pedestrian and bicyclist circulation during the peak morning arrival period and during the peak afternoon dismissal period at the Hommocks Middle School. As school bus transportation is provided only for students who live more than 2 miles from the school, the majority of students walk, bike or are driven to school by a parent/guardian. The circulation paths during the peak morning period for walkers, bicyclists, vehicle and bus drop-offs are described below and shown on **Exhibit 5**.

The first bell is at 8:00 AM with most students arriving between 7:30 and 7:55 AM. In the afternoon, dismissal is at 2:57 with most students departing between 3:00 and 3:20 PM. In the morning and afternoon, crossing guards are assigned to the Boston Post Road and Hommocks Road/Weaver Street intersection and at the Boston Post Road and Richbell Road/Old Boston Post Road intersection. At these two signalized intersections, crosswalks are provided on each approach leg and the traffic signals have an exclusive pedestrian phase during which all vehicular traffic is stopped. A crossing guard is also assigned on Hommocks Road in front of the School. Crosswalks are provided on the main school driveway and on Hommocks Road to the east of the school driveway. The majority of students walking or biking to/from the school from Boston Post Road use the sidewalk adjacent to Walgreen's and then cross Hommocks Road when directed by the crossing guard.

Motorists dropping off or picking up students enter the main school driveway and circulate around to the drop-off/pick-up area in front of the school entrance. Drivers then exit the driveway onto Hommocks Road when directed to by the crossing guard. School buses travel along Hommocks Road to the bus drop-off/pick-up area located on the northern part of the campus.

The Larchmont/Mamaroneck Safe Routes to School Committee (L/M SRTS) was established in 2008 to promote the health and fitness among students by providing safe walking and bicycling routes to area schools. Walking and biking to school is encouraged at all Mamaroneck schools and students and parents are provided tips on biking and pedestrian safety to increase awareness among drivers and pedestrians. At the Hommocks Middle School, per the L/M SRTS, it is quite busy during the arrival and dismissal periods with pedestrians, cyclists, buses and cars. Prior to the beginning of the school year in 2015, the School (with help from law enforcement) established a drop off lane and a "through" lane in the front parking lot to increase efficiency and improve safety. More information on the Safe Routes to School initiatives is provided in the Appendix.



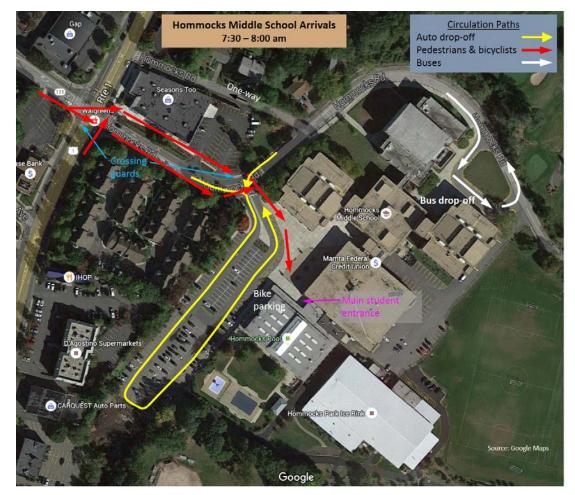


Exhibit 5 – Hommocks Middle School Circulation Patterns

Crash History Analysis

Historical crash data for the study intersections were obtained from the New York State Department of Transportation (NYSDOT) for the latest available three-year period from January 1, 2013 to December 31, 2015. The data was reviewed and tabulated according to location, crash severity (fatalities or injuries), crash type (rear-end, right-angle, etc.) and contributing factors. The accident data are summarized by roadway corridor and by intersection in **Tables 2 and 3**, respectively. A detailed breakdown of the crash data, including collision diagrams, is provided in the Appendix.



Table 2 - Accident Summary by Corridor

Corridor	2013	2014	2015	Total 2013 to 2015
Boston Post Road (US Route 1)	34	35	32	101
Orienta Avenue	0	1	1	2
Hommocks Road/Weaver St (NY Route 125)	1	1	2	4
Old Boston Post Road	1	0	0	1
Eagle Knolls Road	0	0	0	0
East Cove Road	0	0	0	0
Fairway Lane	0	0	0	0
Cooper Avenue	0	0	0	0
Richbell Road	0	1	3	4
Total	36	38	38	112

Table 3 - Accident Summary by Study Location

Intersection	Total No. of	Accident	Severity	No. of Accidents involving			
	Accidents	Fatalities	Injuries	Pedestrians	Bicyclists		
Boston Post Road (US Route 1) and Hommocks Road/Weaver Street*	17	0	5	1	1		
Hommocks Road and Eagle Knolls Road*	0	0	0	0	0		
Orienta Avenue and East Cove Road*	0	0	0	0	0		
Boston Post Road (US Route 1) and Orienta Avenue/Delancey Avenue*	19	0	5	1	0		
Old Boston Post Road and Cooper Avenue*	0	0	0	0	0		
Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road*	43	0	17	6	2		
Fairway Lane and Orienta Avenue*	0	0	0	0	0		
Boston Post Road (US Route 1) and Rockland Avenue	14	0	6	0	0		
Boston Post Road (US Route 1) and Rockridge Road	14	0	6	0	1		
Total	107	0	39	8	4		

Note: * Study location.

As indicated in Table 2, during the three-year period there was a total of 112 crashes with 101 crashes (90 percent) reported on Boston Post Road, 2 crashes on Orienta Avenue, 4 on Hommocks Road/Weaver Street, 1 on Old Boston Post Road and 4 on Richbell Road. No accidents were reported on Eagle Knolls Road, East Cove Road, Fairway Lane or Cooper Avenue. It is noted that there was one (1) accident reported in the Hampshire Country Club parking lot, where one vehicle backed into another and there was no injury.



Of the 112 crashes within the study area, 79 occurred at the study intersections, with the remaining 33 crashes occurring at other locations along the roadway corridors. As shown in Table 3, the highest number of crashes in the 3-year period occurred at the Boston Post Road (US Route 1) and Old Boston Post Road/Richbell Road intersection with a total of 43 crashes. That intersection also had the most crashes involving pedestrians (6) and bicyclists (2). A further tabulation of the accidents was conducted to show the manner of collision, as summarized in Table 4.

	Total No.				Mann	er of Co	lision			
Intersection	of Accidents	Rear End	Right Angle	Left turn	Right Turn	Over- taking	Head- on	Ped	Bike	Other
Boston Post Road (US Route 1) and Hommocks Road/Weaver Street*	17	6	1	2	1	4	1	1	1	0
Hommocks Road and Eagle Knolls Road*	0	-	-	-	-	-	-	-	-	-
Orienta Avenue and East Cove Road*	0	-	-	-	-	-	-	-	-	-
Boston Post Road (US Route 1) and Orienta Ave/Delancey Ave.*	19	8	1	4	-	4	-	1	1	-
Old Boston Post Road and Cooper Avenue*	0	-	-	-	-	-	-	-	-	-
Boston Post Road (US Route 1) and Old Boston Post Road/ Richbell Road*	43	4	10	8	3	6	-	6	2	4
Fairway Lane and Orienta Avenue*	0	-	-	-	-	-	-	-	-	-
Boston Post Road (US Route 1) and Rockland Ave	14	5	2	-	-	5	-	-	-	2
Boston Post Road (US Route 1) and Rockridge Rd	14	5	2	3	-	2	-	-	-	2
Total	107	28	16	17	4	21	1	8	4	8

Table 4 - Accident Summary - Manner of Collision

Note: * Study location.

As shown in Table 4, of the 107 crashes at intersections, the most predominant type were rearend collisions with a total of 28 crashes (26 percent), followed by overtaking (21 crashes/20 percent) and left-turn (17 crashes/16 percent). Collision diagrams for each intersection are provided in the Appendix.



3

Future Conditions

An analysis of future conditions, both with and without the proposed development ("Build" and "No-Build" conditions, respectively), was performed for each of the peak hours to evaluate the effect of the proposed action on future traffic in the area. The No-Build condition represents the future traffic conditions that can be expected to occur, if the proposed development does not materialize. The No-Build condition serves as a comparison to the Build condition, which represents expected future traffic conditions resulting from both project and non-project-generated traffic.

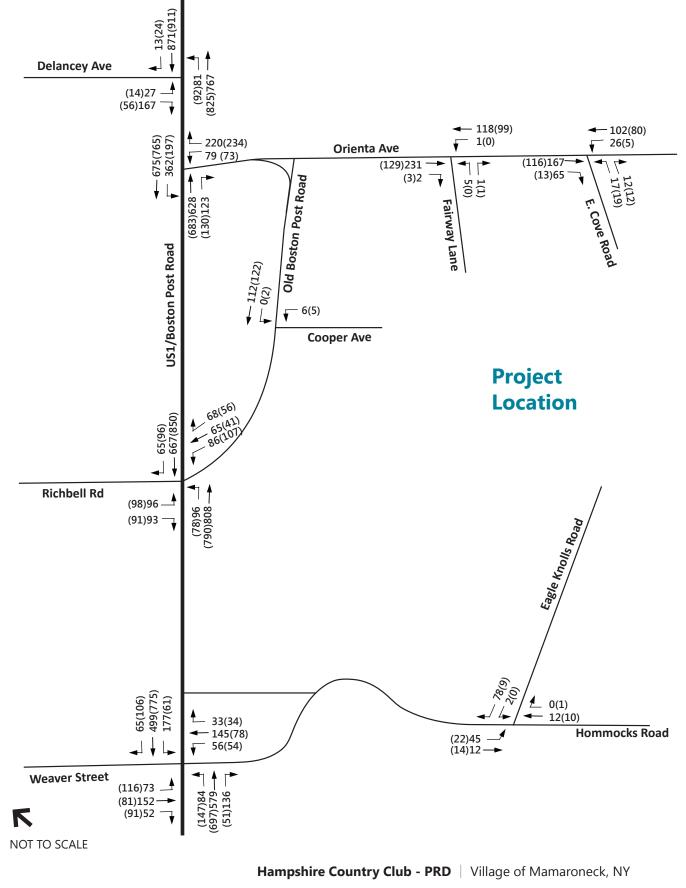
No-Build Condition

Traffic growth is typically a function of the expected land development, economic activity and changes in demographics in the region. To estimate the rate at which traffic can be expected to grow during the study period, both historical growth and planned area developments are reviewed and considered, as described below.

Background Traffic Growth

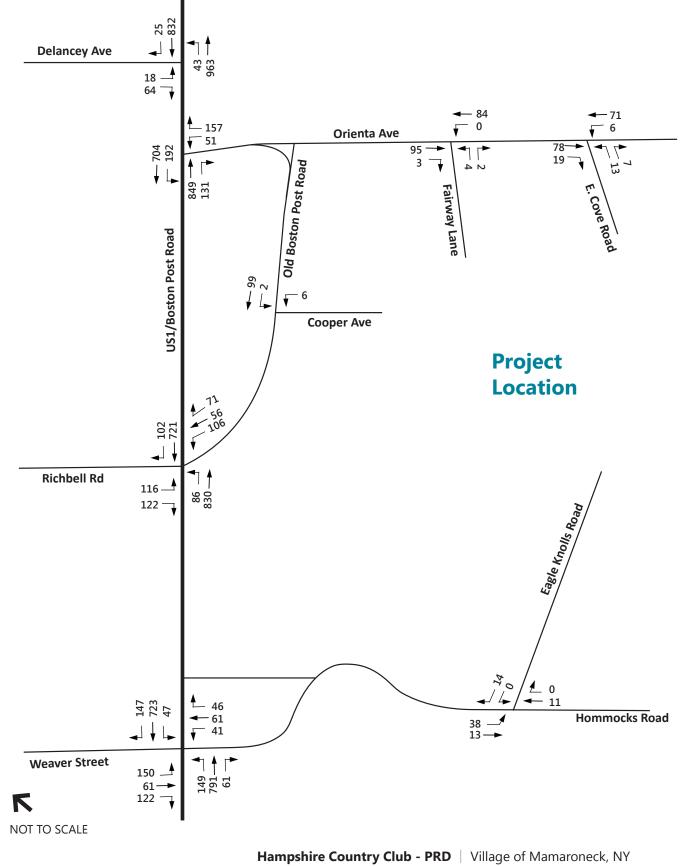
A review of historical data provided by NYSDOT indicates that traffic has decreased by approximately 0.4% per year between 1996 and 2014, with more recent data (2011 to 2014) indicating a 0.8% per year decline. In consultation with the Village of Mamaroneck Planner, it has been determined that an increase of 0.25% per year would be appropriate and would provide for a representative analysis. The existing traffic volumes for all three peak hours were increased by a total of 1.3 percent to represent the grown volumes. The Weekday and Saturday peak hour volumes are shown on **Exhibits 6 and 7**.





00= AM Peak Hour





Grown Saturday Peak Hour Volumes



Planned Vicinity Developments

The Planning Boards of the Village and Town of Mamaroneck provided information on proposed vicinity developments in the area. A total of 7 residential developments were identified; 6 in the Village of Mamaroneck and 1 project in the Town of Mamaroneck, as noted in **Table 5**.

Table 5 - Vicinity Developmer	ıts
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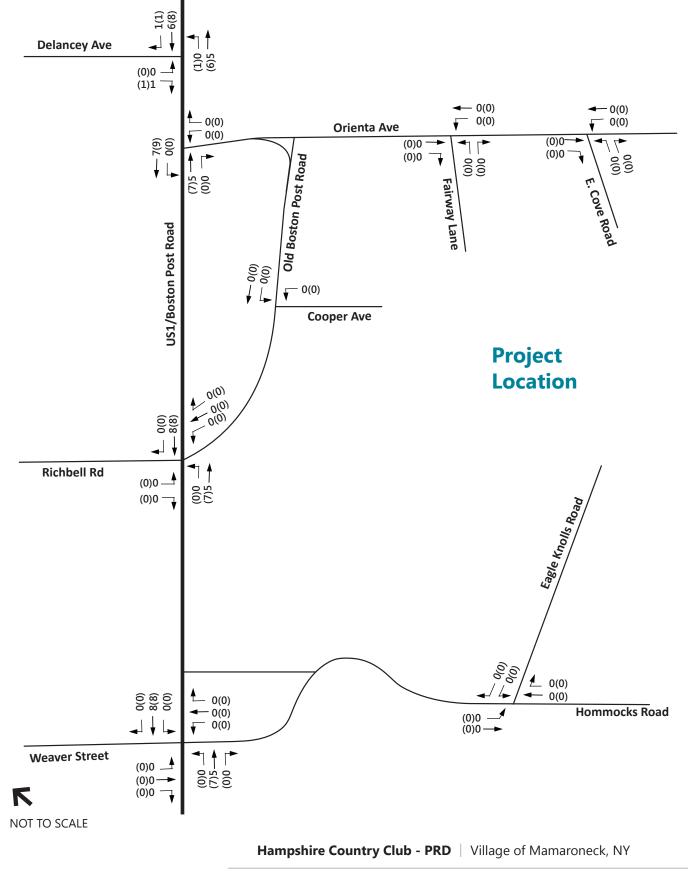
Development	Size
690 Mamaroneck Avenue	21 units
270 Waverly Avenue	96 units
620 W. Boston Post Road	6 units
422 E. Boston Post Road	13 units
151 Mamaroneck Avenue ⁽¹⁾	10 units
532 W. Boston Post Road	7 units
The Cambium (Town)	149 units

Note: (1) Subsequent to preparing the traffic analyses in this study, VHB was advised that this project is no longer going forward; however, the volumes are included in the analyses.

The traffic volumes associated with the above developments were obtained from traffic studies, if available, or were estimated by VHB using standard trip generation methodology. Altogether, the 7 developments are projected to increase traffic in the study area by a further 0.7 percent. The vicinity development trips added to the study area intersections, are indicated on **Exhibits 8 and 9**.

The vicinity development volumes were added to the grown volumes resulting in the future No-Build peak hour traffic volumes shown on **Exhibits 10 and 11**.



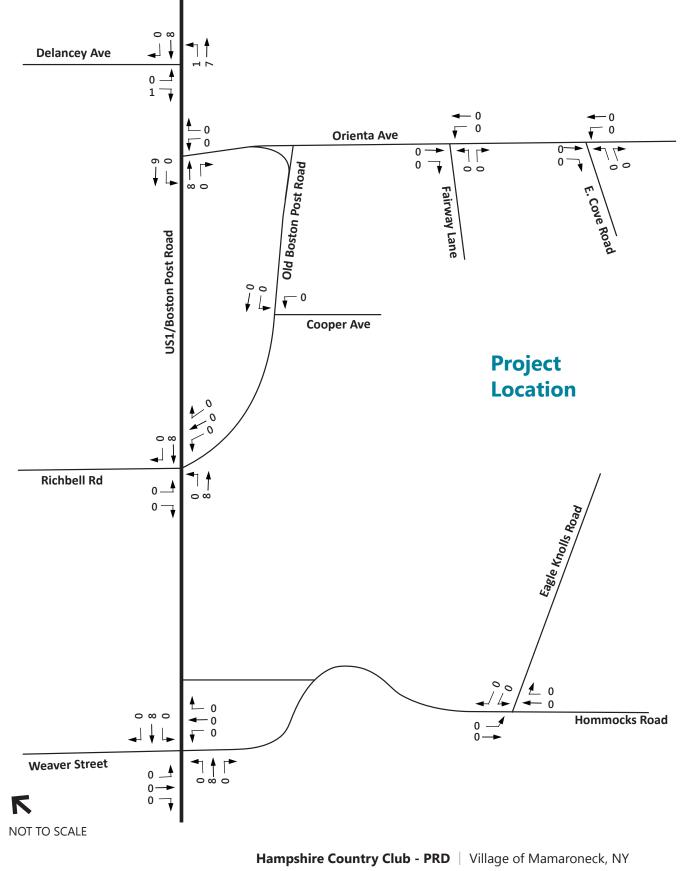


00= AM Peak Hour

(00)=PM Peak Hour

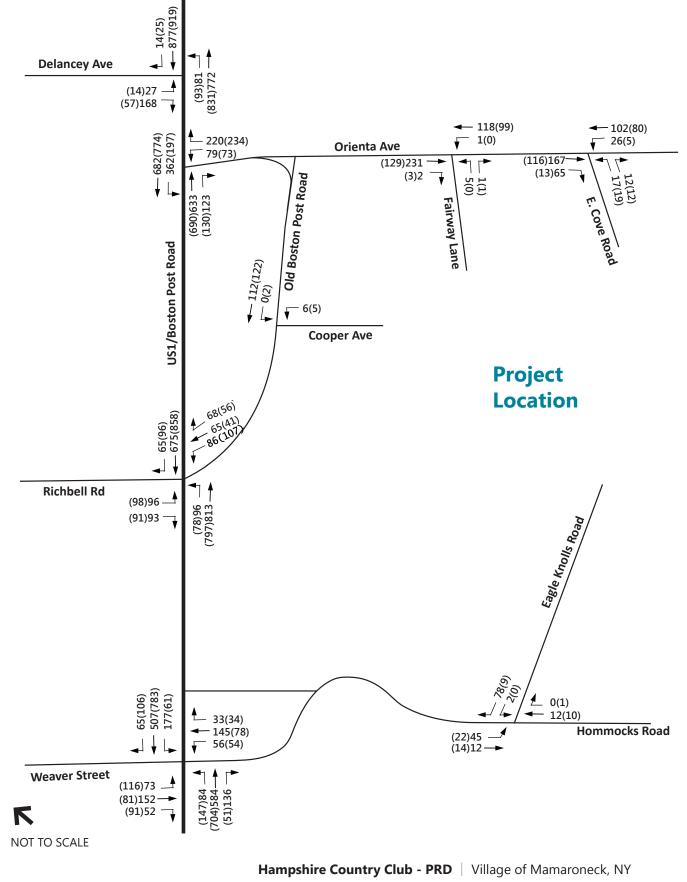
Vicinity Development Weekday Hour Volumes





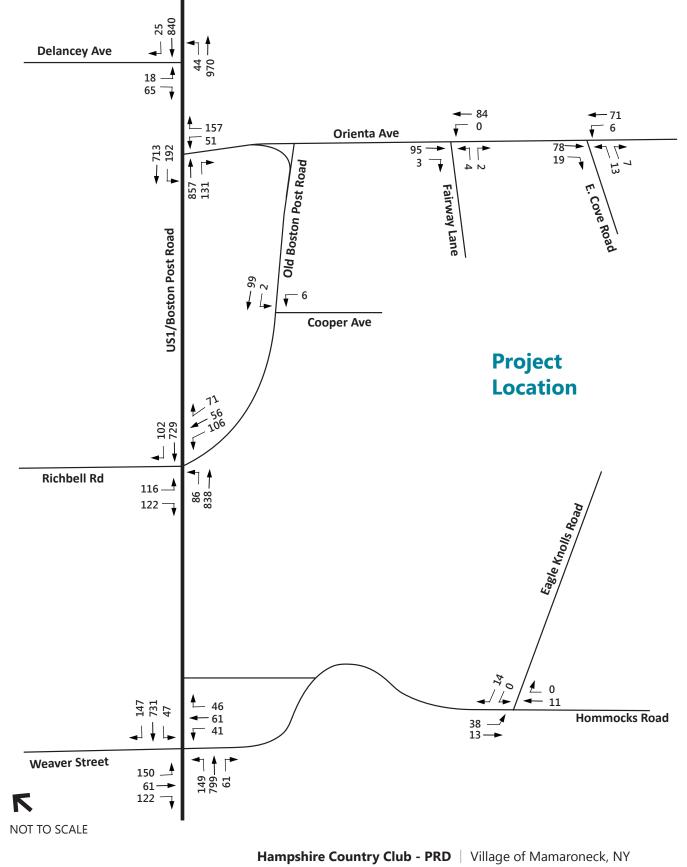
Vicinity Development Saturday Peak Hour Volumes





No-Build Weekday Peak Hour Traffic Volumes





No-Build Saturday Peak Hour Traffic Volumes



Site-Generated Traffic

The Proposed Action is to consist of 105 residential units, comprised of 44 single-family detached homes and 61 townhouses. The existing 18-hole golf course will be reduced to a 9-hole course to facilitate the development of the project. The existing membership club facilities (including a clubhouse, pool and parking areas) will remain.

To evaluate the traffic impact of the Proposed Action, it is necessary to determine the traffic volumes expected to be generated by the 105-unit residential development and how much traffic activity at the existing country club will be reduced by the elimination of 9 holes of the golf course. A review was undertaken of the available trip generation data sources, including the reference published by the Institute of Transportation Engineers ("ITE"), *Trip Generation Manual*, Ninth Edition. This widely utilized reference source contains trip generation rates for related uses, "Single-Family Detached Housing" (Land Use Code 210) and "Residential Condominium/Townhouse" (Land Use Code 230).

The existing road network through the site connects the Hommocks Road School with the residential neighborhood to the north of the site and approximately 23 homes are accessed off of either Eagle Knolls Road or East Cove Road. Current levels of traffic activity at the existing Hampshire Country Club were identified based on a review of the existing traffic volumes which indicated that that the facility currently generates 33 trips during the weekday AM peak hour (19 in and 14 out), 50 trips during the weekday PM peak hour (21 in and 29 out) and 69 trips during the Saturday peak hour (47 in and 22 out). These values compare reasonably well with ITE values for an 18-hole golf course (37, 53 and 83 in the AM. PM and Saturday peak hours, respectively).

Of the above trips currently generated by the Hampshire Country Club, it was assumed that two trips in each of the peak hours are staff arriving at or departing the facility and that there will be no change in this number as a result of the elimination of nine holes of golf. It was further assumed that none (0) of the weekday AM peak-hour trips, 10 of the weekday PM peak-hour trips and 14 of the Saturday midday peak hour trips (0% of the Country Club's AM trips and 20% of the Country Clubs PM and Saturday trips) are non-golf-related member trips and that there will be no change in this number as a result of the elimination of nine holes of golf. Subtracting these trips from the 33 AM, 50 PM and 69 Saturday peak-hour Country Club trips leaves 31 trips currently associated with the 18-hole course in the AM peak hour, 38 trips associated with the course in the PM peak hour and 53 trips associated with the course in the Saturday peak hour.

It was conservatively assumed that the elimination of 9 holes of the golf course would reduce this golf-course traffic generation by 37 percent or 11 trips in the AM peak Hour, 14 trips in the PM peak hour and 20 trips on the Saturday peak hour.

In addition, to account for expected pedestrian trips, including internal trips between the singlefamily homes, town homes and the clubhouse/golf course, a 5 percent credit was applied to the residential trips (a 4-trip reduction in each of the peak hours). The resulting new trips from the Project on the local roadways are summarized in **Table 6**. Table 6 also provides a



comparison between the trip generations for the existing Hampshire Country Club and the trip generations for the proposed Project.

Land Use		No. of Units Units Hour Total (in/out)		Hour		Hour		PM Peak Hour Total (in/out)		Hour		t) Saturday Pea Hour Total (in/out			
Single-Family Home	44	4	1 (11/3	0)	50 (33/17)		2	48 (26/22)							
Townhouse	61	35 (10/25)		35 (10/25)		40 (27/13)		87 (20/1	7)						
Total Residential	105	76 (21/55)		90 (60	/30)	٤	85 (46/3	9)							
- Internal Credit (5%)	-		-4 (-2/-2)		-4 (-2/-2)		-4 (-2/-2)		-4 (-2	/-2)		-4 (-2/-2)			
- Golf Course Trip Credit ⁽¹⁾	-	-	-11 (-8/-3)		-11 (-8/-3)		-11 (-8/-3)		-11 (-8/-3)		-11 (-8/-3) -13 (-9/-4)		-20 (-11/-9)		-9)
Total New Trips		61 (11/5		0)	73 (49/24)		61 (33/28)		8)						
	Trip G	ienerat	ion Cor	nparis	son										
Development	AM	Peak H	lour	PI	M Peak H	lour	SAT	F Peak H	lour						
Development	in	out	total	in	out	total	in	out	total						
Existing Hampshire Country Club	19	14	33	21	29	50	47	22	69						
Proposed Development ⁽²⁾	30	64	94	70	53	123	80	50	130						
Net Change (Proposed - Existing)	11	50	61	49	24	73	33	28	61						

Table 6 - Project Trip Generations

Source: ITE Trip Generation Manual, Ninth Edition.

Note: (1) Assumed 37% of the existing golf course trips would be eliminated.

(2) Including 9-hole golf course and clubhouse and 5% internal capture credit.

As shown in Table 6, the Proposed Action is expected to generate a total of 61 new trips during the AM peak hour, 73 new trips during the PM peak hour and 61 new trips during the Saturday peak hour.

Trip Distribution and Assignment

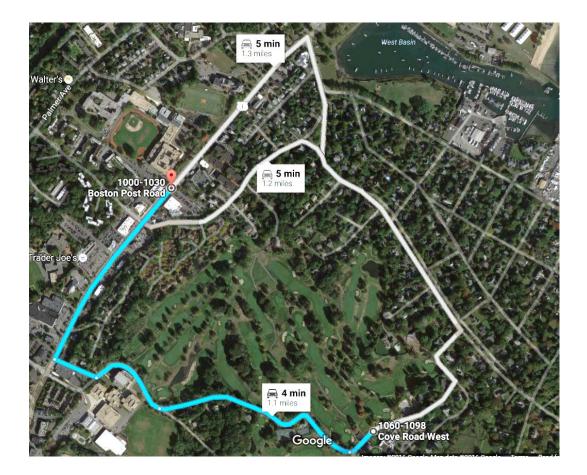
The three existing access points to the project site (Cove Road, Eagle Knolls Road and Cooper Avenue) will be modified as part of the Proposed Action. The privately-owned portion of Cove Road within the Project site will be relocated, and this road will form the central corridor for the project which will connect with Eagle Knolls Road. Portions of Eagle Knolls Road will also be relocated from its existing location and will terminate in a cul-de-sac. Cooper Avenue, which currently extends from Old Boston Post Road to its terminus at the driveway to the golf course maintenance facility will be extended into the Site and will intersect with Cove Road.

As part of the development of the site plan, consideration was given to what configuration access to Cooper Avenue should take. This evaluation determined that allowing project traffic to exit via Cooper Avenue would have the greatest overall benefit, as it would encourage motorists travelling from the site to Richbell Road or any destination on Boston Post Road



between Hommocks Road and the Mamaroneck High School to do so without passing through the busiest intersection in the study area (Boston Post Road with Hommocks Road/Weaver Street) or by the Hommocks Road School. Because of the one-way orientation of Old Boston Post Road, allowing project traffic to enter via Cooper Avenue would not achieve the same outcome. If Cooper Avenue provides two-way access, the same benefits to the intersection of Hommocks Rd with Boston Post Road would accrue as in the one-way exit configuration, while if emergency access only were provided at Cooper Avenue, potential impacts to the intersection of Richbell Road and Boston Post Road would be less than the other access options.

To provide a conservative analysis, for each of the study area intersections (except for the intersections of Old Boston Post Road with Cooper Avenue and Richbell Road/Boston Post Road) it was assumed that all project traffic would enter and exit via Hommocks Road or Orienta Avenue. For the intersections of Old Boston Post Road with Cooper Avenue and Richbell Road/Boston Post Road, it was assumed that there would be two-way access provided to the Project Site via Cooper Avenue (which would result in the greatest project impact at these intersections).



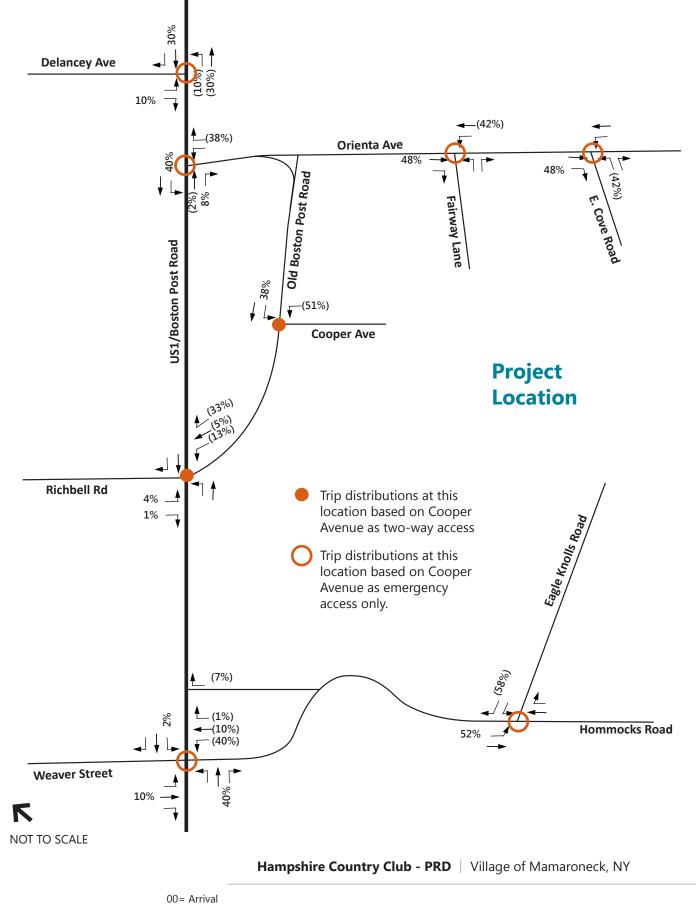


Trip arrival and departure patterns, which show how the newly-generated trips will travel to and from the site, were determined based on a review of the existing roadway network, existing traffic patterns and proposed access to the project. The trip origin and destination percentages for the project-generated trips are shown in **Table 7**.

Trip Origin/Destination	Percent of Site Traffic
Boston Post Road (US Route 1) from/to the north	30
Boston Post Road (US Route 1) from/to the south	40
Weaver Street (NYS Route 125) from/to the west	10
Delancey Avenue from/to the west	10
Richbell Road from/to the west	5
From/to Local streets	5

The distribution percentages at each study location are shown on **Exhibit 12**





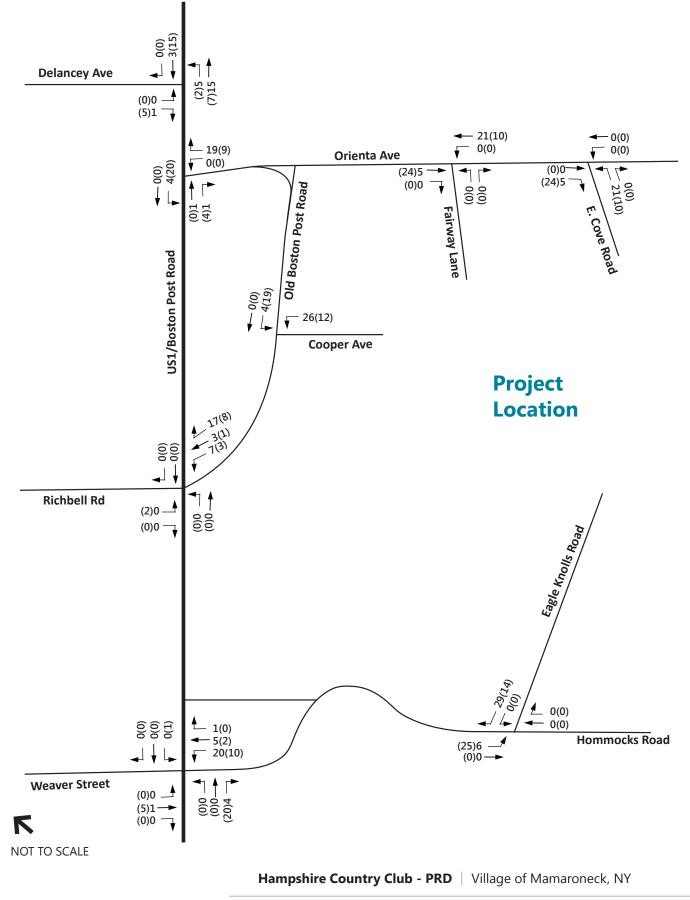
(00)=Departure

Trip Distributions



The trip distributions shown on Exhibit 12 were then applied to the project trips shown in Table 6 and the resulting volumes were assigned to the local roadway network. These project-generated volumes are shown on **Exhibits 13 and 14**.

The project-generated volumes were added to the No-Build traffic volumes shown on Exhibits 10 and 11 resulting in the Build traffic volumes for the AM, PM and Saturday peak hours shown on **Exhibits 15 and 16**.



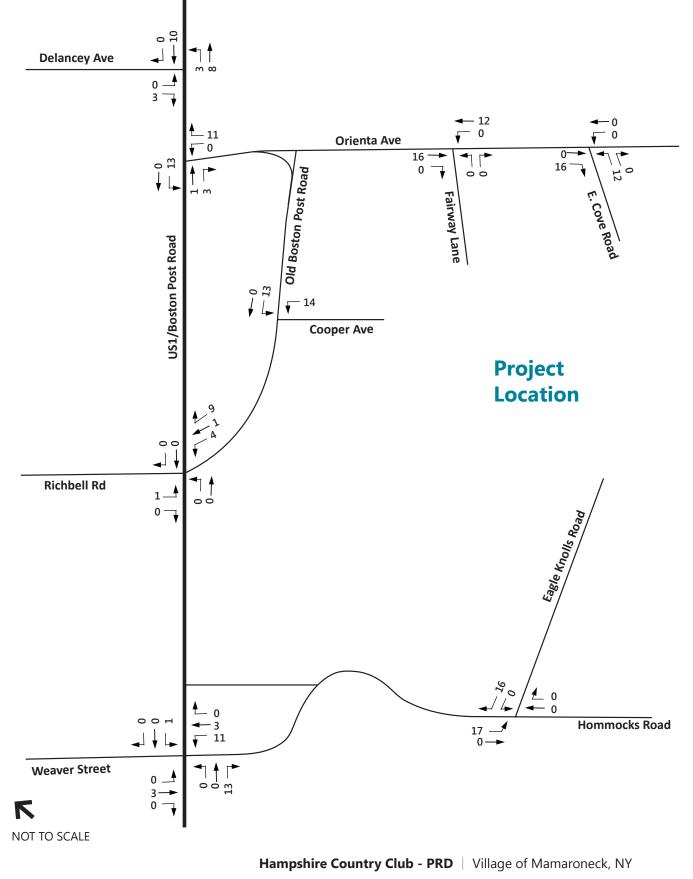
00= AM Peak Hour

(00)=PM Peak Hour

Project Generated Weekday Peak Hour Volumes

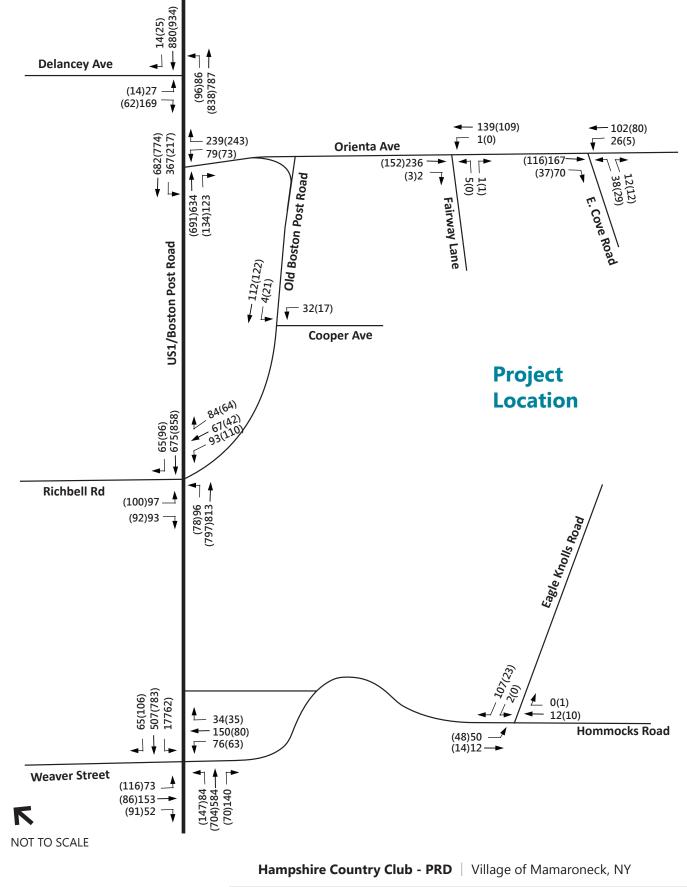
Vhb | Exhibit 13





Project Generated Saturday Peak Hour Volumes



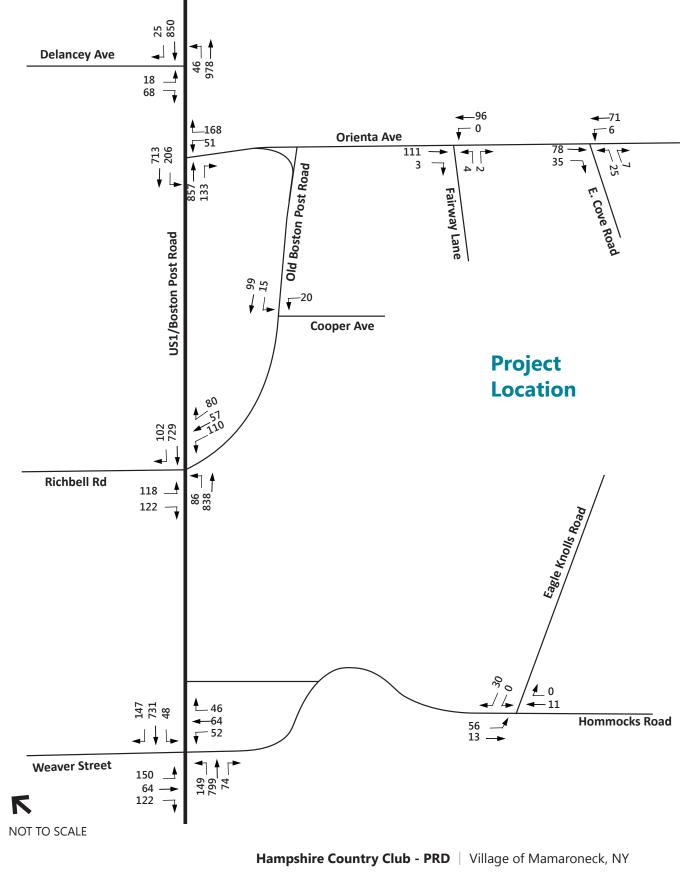


00= AM Peak Hour

(00)=PM Peak Hour

Build Weekday Peak Hour Traffic Volumes





WrbpgWritePark2667702-tarpatie6.bckianggatickRQLRESTaffNqpsTaffTrpacBatemert_12_16_2016TaffTrpacBatemerEvt26_12_16_2016trcd

Build Saturday Peak Hour Traffic Volumes



4

Traffic Operations

To assess the quality of traffic flow in the study area during the peak hours, intersection capacity analyses were conducted for Existing, No-Build, and Build (with the proposed residential development) traffic volume conditions. The following section summarizes the methods of capacity analyses used in this study and documents the results.

Method of Capacity Analysis

The intersection capacity analyses were conducted based on the evaluation criteria contained in the 2010 Highway Capacity Manual¹ (HCM). As documented in the HCM, intersection performance is influenced by a number of factors, including: traffic demand; lane configurations; lane widths; turning restrictions; roadway grades; speeds; and signal phasing and timing settings for signalized intersections. The existing physical roadway characteristics and signal phasing and timing settings at the signalized study intersection were determined by collecting field measurements.

Synchro 9 software was used to model the study intersections based on the parameters mentioned above. Synchro 9 software is widely used by traffic engineering professionals, is approved for use by NYSDOT, and is consistent with the procedures in the HCM.

Capacity analyses results are reported using a variety of performance measures, including "Level of Service" (LOS). The level of service designation is an index based on the average control delay experienced by a vehicle traveling through the intersection. Similar to a report card, LOS designations are letter-based, ranging from A to F, with LOS A representing the best operating condition (lowest vehicle delays) and LOS F representing the worst operating condition (highest vehicle delays).

LOS is reported differently for signalized and unsignalized intersections. For signalized intersections, the analysis considers the operation of all traffic entering the intersection, and the LOS can be reported for individual turning movements, approaches, or for the intersection as a whole. For unsignalized intersections, the most critical lane group delay on each approach is typically reported and the overall intersection LOS is not calculated. Thus the LOS designation is for the critical movement exiting the side street, which is generally the left turn out of the

¹ <u>Highway Capacity Manual 2010</u>; Transportation Research Board, National Research Council, Washington, DC (2010).



side street or side driveway. As such, LOS is reported only for left-turns from the main street and for all movements from the side street.

Intersection Capacity Analysis

Intersection capacity analyses were conducted for the Existing condition and future No-Build and Build conditions for each of the key intersections. The results of the capacity analyses for the weekday AM and PM peak hours are summarized in **Table 8** and the capacity analyses results for the Saturday peak hour are summarized in **Table 9**. The detailed Synchro capacity analysis worksheets are contained in the Appendix.



Table 8 - Capacity Analysis Summary – Weekday AM & PM Peak Hours

Approach intersection Approach (Group Corop (Corop Corop (Corop Corop (Corop Co					Exis	ting			No-E	Build		Build			
IntersectionApproachGroupCosDelayLOSDelayL				AN	1 Peak	PM	Peak	AN	l Peak	PM	Peak	AN	1 Peak	PM	Peak
Boson Post Rd (US Route 1), Rd/Weaver St L E Sa.0 D 48.1 E Sp.1 D 48.7 E 60.3 D 47.6 Boson Post Rd (US Route 1), Rd/Weaver St WB L D 55.6 D 47.1 D 52.3 D 47.3 D 52.2 D 47.6 Kommocks Rd/Weaver St WB L D 50.6 D 44.6 D 50.5 D 44.6 D 50.6 D 44.7 E 65.2 D 41.7 E 56.2 D 41.7 E 56.2 C 27.8 C 30.9 E 74.5 C 27.4 C 32.0 D 40.9 D 38.0 D			Lane	н	lour	Н	lour	н	lour	Н	lour	F	lour	н	lour
Boston Post Ri (US Route 1) & Route 1) & R	Intersection	Approach	Group	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
Boson Post Rig (US ñoure) 16 $\left \left \left$		FR	L	Е	58.0	D	48.4	Е	59.1	D	48.7	Е	60.3	D	48.8
Boston Post Rd (US Route 1) IWB Hommocks Rd/Weaver st IWB IR ID 50.6 D 44.4 D 50.9 D 44.6 D 51.6 D 44.7 WG Route 1) Rd/Weaver st IMB IR IL D 39.7 D 53.1 D 41.7 E 56.2 D 44.7 E 56.2 D 44.7 E 56.2 D 44.7 E 56.2 D 44.7 E 56.2 D 38.8 E 77.8 C 23.0 D 40.7 C 38.0 D 40.9 D 38.0		LD	TR	D	51.6	D	47.1	D	52.1	D	47.3	D	52.2	D	47.6
(US Route 1) 8 Hommocks Rd/Weaver 3: (Poston Doct Pd	\//B	L	D	54.1	D	46.9	E	55.3	D	47.1	E	64.1	D	47.9
		VVD	TR	D	50.6	D	44.4	D	50.9	D	44.6	D	51.6	D	44.7
Rd/Weaver styInt<	• •	ND	L	D	39.7	D	53.1	D	41.7	E	56.2	D	41.7	E	56.2
Image Image <t< td=""><td></td><td>IND</td><td>TR</td><td>Е</td><td>68.7</td><td>С</td><td>30.7</td><td>E</td><td>72.9</td><td>С</td><td>30.9</td><td>E</td><td>74.5</td><td>С</td><td>31.4</td></t<>		IND	TR	Е	68.7	С	30.7	E	72.9	С	30.9	E	74.5	С	31.4
Image Image <t< td=""><td>-</td><td>SB</td><td>L</td><td>Е</td><td>75.5</td><td>С</td><td>25.8</td><td>E</td><td>76.2</td><td>С</td><td>26.4</td><td>E</td><td>76.2</td><td>С</td><td>27.4</td></t<>	-	SB	L	Е	75.5	С	25.8	E	76.2	С	26.4	E	76.2	С	27.4
Hommocks Rd & Eagle Knolls Rd (unsignalized) WB IR A 7.6 A 7.6 A 7.6 A 7.6 A 8.1 A 8.1 A 7.6 Generationalized) MB TR A 7.6 A 7.7 A 8.4 A 7.7 A 8.7 A 8.7 A 7.7 Generationalized) MB LT A 8.2 A 7.7 A 8.9 A 7.6 A 8.13 A 7.8 Boston Post Rd GU D A 9.0 A		50	TR	D	37.4	D	40.2	D	38.0	D	40.9	D	38.0	D	40.9
Lase Lin Lin A 7.6 A 7.0 A 7.0 A 7.0 A 7.8 A 7.1 Bage NB TR A 7.6 A 7.0 A 7.3 A 8.7 A 7.1 Orienta Ave & East Cove Rd (unsignalized) EB LR A 8.2 A 7.7 A 8.9 A 7.7 A 9.1 A 7.8 A 7.8 Boston Post Rd (US Route 1) & Orienta Ave/ Delancey Ave EB L D 43.9 D 43.8 D 43.6 D 44.5 D 44.5 D 44.5 D 44.5 D 44.6 D 44.6 D 44.6 D 44.6 D 44.6 D 42.1 D 44.8 D 43.7		Inte	rsection	E	55.4	D	38.8	E	57.3	D	39.4	E	58.3	D	39.6
Initialized Initialized <thinitialized< th=""> <thinitialized< th=""></thinitialized<></thinitialized<>	Hommocks Rd &	WB	LR	Α	7.6	А	6.5	А	7.6	А	6.5	А	8.1	А	6.6
Orienta Ave & EB LR A B.J. A A B.A A A A A D.A A A D.A A D.A A D.A A D.A A D.A B A D.A B A D.A B A	Eagle Knolls Rd	NB	TR	Α	7.6	Α	7.0	Α	7.6	Α	7.0	Α	7.8	Α	7.1
East Cove Rd (unsignalized) INB LT A 8.9 A 7.7 A 8.9 A 7.7 A 9.1 A 9.8 A 7.7 A 9.1 A 9.1 A 9.8 A 9.9 A 8.10 B 10.2 A 8.2 Boston Post Rd (US Route 1) & Orienta Ave/ Delancey Ave (usingnalized) \mathbb{A} \mathbb{B} 10.0 43.8 D 43.8 D 43.8 D 43.6 D 44.8 D 43.6 D 44.6 D 42.1 D 43.6 A 9.0 A 8.6 A 9.0 A 8.6 A 9.0 A 8.6 10.2 10.2	(unsignalized)	SB	LT	Α	8.3	Α	7.3	Α	8.4	Α	7.3	Α	8.7	Α	7.6
Image Image <t< td=""><td>Orienta Ave &</td><td>EB</td><td>LR</td><td>Α</td><td>8.2</td><td>А</td><td>7.6</td><td>А</td><td>8.2</td><td>А</td><td>7.6</td><td>А</td><td>8.7</td><td>Α</td><td>7.8</td></t<>	Orienta Ave &	EB	LR	Α	8.2	А	7.6	А	8.2	А	7.6	А	8.7	Α	7.8
Boson Post Rd (US Route 1) & R L D 43.9 D 43.8 D 43.6 <th< td=""><td>East Cove Rd</td><td>NB</td><td>LT</td><td>Α</td><td>8.9</td><td>Α</td><td>7.7</td><td>Α</td><td>8.9</td><td>Α</td><td>7.7</td><td>Α</td><td>9.1</td><td>Α</td><td>7.8</td></th<>	East Cove Rd	NB	LT	Α	8.9	Α	7.7	Α	8.9	Α	7.7	Α	9.1	Α	7.8
Boston Post Rd (US Route 1) & Orient Ave/ Delance/ Ave (usinginalized) L B 10.0 A 10.0 A 10.0 A 10.0 A 10.0 A 10.0 B 10.2.0 B 10.0 A 10.0 A 10.0 A 10.0 A 10.0 A A 10.0 A A 10.0 A A 0.0 A A 0.0 A A 0.0 A A 0.0 A A A A A D A A A D A A D A A D A A D A A D A A D A A D A	(unsignalized)	SB	TR	Α	9.8	Α	8.1	Α	9.9	Α	8.1	В	10.2	Α	8.2
Boston Post Rd (US Route 1) & Orienta Ave/ Delancey Ave (unsignalized) R R B 10.5 B 13.0 B 10.4 B 12.8 B 10.4 B 12.8 B 10.4 B 12.8 B 10.4 B 12.5 Orienta Ave/ Delancey Ave (unsignalized) MB TR D 44.5 D 42.1 D 44.8 D 42.2 D 44.8 D 42.5 MB TR D 41.6 D 36.6 A 9.0 A 8.6 A 9.0 A 8.7 MB TR C 22.8 C 23.0 C 27.8 C 23.4 C 23.4 </td <td></td> <td>FB</td> <td>L</td> <td>D</td> <td>43.9</td> <td>D</td> <td>43.8</td> <td>D</td> <td>43.6</td> <td>D</td> <td>43.6</td> <td>D</td> <td>43.6</td> <td>D</td> <td>43.4</td>		FB	L	D	43.9	D	43.8	D	43.6	D	43.6	D	43.6	D	43.4
	Poston Doct Pd		R	В	10.5	В	13.0	В	10.4	В	12.8	В	10.4	В	12.5
Orienta Ave/ Delancey Ave R R A 9.0 A 8.6 A 9.0 A 8.6 A 9.0 A 8.7 Delancey Ave Delancey Ave NB TR D 41.6 D 36.6 D 42.1 D 37.0 D 42.2 D 37.2 SB TR C 22.8 C 23.0 C 23.3 C 23.4		\//B	L	D	44.5	D	42.1	D	44.8	D	42.2	D	44.8	D	42.5
Delancey Ave NB TR D 41.6 D 36.6 D 42.1 D 37.0 D 42.2 D 37.2 SB TR C 22.8 C 23.0 C 23.3 C 23.4 C 23.4 C 23.4 C 23.7 Interretion C 25.7 C 21.0 C 27.8 C 21.5 C 23.4 C 23.7 Old Boston Post Rd & Cooper Ave (unsignalized) WB L A 9.6 A 9.3 A 9.6 A 9.3 A 9.3 </td <td>• •</td> <td>VVD</td> <td>R</td> <td>Α</td> <td>9.0</td> <td>Α</td> <td>8.6</td> <td>Α</td> <td>9.0</td> <td>Α</td> <td>8.6</td> <td>Α</td> <td>9.0</td> <td>Α</td> <td>8.7</td>	• •	VVD	R	Α	9.0	Α	8.6	Α	9.0	Α	8.6	Α	9.0	Α	8.7
Intersection C 25.7 C 21.0 C 27.8 C 21.5 C 28.0 C 21.6 Old Boston Post Rd & Cooper Ave (unsignalized) WB L A 9.6 A 9.3 A 9.6 A 9.6 A 9.6 A 9.6 A 9.3 A 9.6 A 9.3 A 9.3 </td <td>-</td> <td>NB</td> <td>TR</td> <td>D</td> <td>41.6</td> <td>D</td> <td>36.6</td> <td>D</td> <td>42.1</td> <td>D</td> <td>37.0</td> <td>D</td> <td>42.2</td> <td>D</td> <td>37.2</td>	-	NB	TR	D	41.6	D	36.6	D	42.1	D	37.0	D	42.2	D	37.2
Old Boston Post Rd & Cooper Ave (unsignalized) WB L A 9.6 A 9.3 A 9.6 A 9.3 A 9.3 A 9.3 A 9.9 A 9.6 9.6 Ave (unsignalized) SB LT A 0.0 A 0.1 A 0.0 A 0.1 A 9.3 A 9.9 A 9.6 Boston Post Rd (US Route 1) & Old Boston Post Rd (US Route 1) & Old Boston Post Rd/Richbell Rd L D 48.1 D 43.9 D 44.2 D 51.1 D 44.1 Boston Post Rd (US Route 1) & Old Boston Post Rd/Richbell Rd L D 48.1 D 43.9 D 44.2 D 51.1 D 44.1 D 41.0 D 39.8 D 44.2 D 41.1 D 39.8 Old Boston Post Rd/Richbell Rd L D 39.7 D 39.3 D 44.2 D 44.0 D 39.9	-	SB	TR	С	22.8	С	23.0	С	23.3	С	23.4	С	23.4	С	23.7
Rd & Cooper WB L A 9.6 A 9.5 A 9.6 A 9.3 A 9.9 A 9.6 Ave (unsignalized) SB LT A 0.0 A 0.0 A 0.0 A 0.1 A 0.3 A 9.5 A 9.5 Methods LT A 0.0 A 0.0 A 0.0 A 0.0 A 0.1 A 0.3 A 9.5		Inte	rsection	С	25.7	С	21.0	С	27.8	С	21.5	С	28.0	С	21.6
(unsignalized) SB LT A 0.0 A 0.1 A 0.0 A 0.1 A 0.1 A 0.3 A 1.2 (unsignalized) EB L D 48.1 D 43.9 D 49.3 D 44.2 D 51.1 D 44.1 Boston Post Rd MB L D 41.0 D 39.8 D 41.5 D 40.1 D 41.1 D 39.8 Boston Post Rd WB L D 39.7 D 39.8 D 40.2 D 40.1 D 40.2 D 39.9 Old Boston Post Rd WB L D 42.7 D 39.8 D 40.2 D 40.1 D 40.2 D 39.9 Old Boston Post Rd MB L B 18.8 B 13.8 B 18.9 B 14.0 B 14.3 MB </td <td></td> <td>WB</td> <td>L</td> <td>А</td> <td>9.6</td> <td>А</td> <td>9.3</td> <td>А</td> <td>9.6</td> <td>А</td> <td>9.3</td> <td>А</td> <td>9.9</td> <td>А</td> <td>9.6</td>		WB	L	А	9.6	А	9.3	А	9.6	А	9.3	А	9.9	А	9.6
Boston Post Rd (US Route 1) & Old Boston Post Rd/Richbell Rd L D 41.0 D 39.8 D 41.5 D 40.1 D 41.1 D 39.8 Boston Post Rd (US Route 1) & Old Boston Post Rd/Richbell Rd L D 39.7 D 39.8 D 40.2 D 40.1 D 40.2 D 39.9 MB L D 42.7 D 39.3 D 43.3 D 39.7 D 44.0 D 39.9 MB L B 18.8 B 13.8 B 14.3 B 14.3 MB TR C 28.6 C 24.0 <t< td=""><td>-</td><td>SB</td><td>LT</td><td>А</td><td>0.0</td><td>А</td><td>0.1</td><td>А</td><td>0.0</td><td>А</td><td>0.1</td><td>А</td><td>0.3</td><td>А</td><td>1.2</td></t<>	-	SB	LT	А	0.0	А	0.1	А	0.0	А	0.1	А	0.3	А	1.2
Boston Post Rd (US Route 1) & Cl R D 41.0 D 39.8 D 41.5 D 40.1 D 41.1 D 39.8 Boston Post Rd (US Route 1) & Cl WB L D 39.7 D 39.8 D 40.2 D 40.1 D 40.2 D 39.9 Old Boston Post Rd/Richbell Rd R D 42.7 D 39.3 D 43.3 D 39.7 D 44.0 D 39.9 Rd/Richbell Rd R L B 18.8 B 13.8 B 18.8 B 14.3 B 14.3 $Rd/Richbell Rd$ R <td></td> <td>ED</td> <td>L</td> <td>D</td> <td>48.1</td> <td>D</td> <td>43.9</td> <td>D</td> <td>49.3</td> <td>D</td> <td>44.2</td> <td>D</td> <td>51.1</td> <td>D</td> <td>44.1</td>		ED	L	D	48.1	D	43.9	D	49.3	D	44.2	D	51.1	D	44.1
(US Route 1) & Old Boston Post Rd/Richbell Rd TR D 42.7 D 39.3 D 43.3 D 39.7 D 44.0 D 39.9 Rd/Richbell Rd NB L B 18.8 B 13.8 B 18.9 B 14.0 B 19.3 B 14.3 Rd/Richbell Rd T B 18.8 B 13.2 B 18.8 B 13.3 B 19.3 B 13.6 SB TR C 28.6 C 24.0 C 28.6 C 24.0 C 28.6 C 29.1 C 24.6 SB TR C 28.6 C 24.0 C 28.6 C 28.0 C 29.1 C 24.6 Intersection C 27.1 C 22.7 C 27.3 C 23.0 C 28.0 C 23.3 Orienta Ave & Fairway Ln EB LR		ED	R	D	41.0	D	39.8	D	41.5	D	40.1	D	41.1	D	39.8
(US Route 1) & Old Boston Post Rd/Richbell Rd TR D 42.7 D 39.3 D 43.3 D 39.7 D 44.0 D 39.9 MB L B 18.8 B 13.8 B 18.9 B 14.0 B 19.3 B 14.3 Rd/Richbell Rd T B 18.8 B 13.2 B 18.8 B 13.3 B 19.3 B 14.3 SB TR C 28.6 C 24.0 C 28.6 C 24.3 C 29.1 C 24.6 SB TR C 28.6 C 24.0 C 28.6 C 24.3 C 29.1 C 24.6 Intersection C 27.1 C 22.7 C 27.3 C 23.0 C 28.0 C 23.3 Orienta Ave & Fairway Ln EB LR B 10.9 A 9.0 B 10.2 A 9.2 Momoder NB LT A	Boston Post Rd		L	D	39.7	D	39.8	D	40.2	D	40.1	D	40.2	D	39.9
Rd/Richbell Rd NB L B 16.8 B 15.8 B 16.9 B 14.0 B 19.3 B 14.3 Rd/Richbell Rd T B 18.8 B 13.2 B 18.8 B 13.3 B 19.3 B 13.6 SB TR C 28.6 C 24.0 C 28.6 C 24.3 C 29.1 C 24.6 Intersection C 27.1 C 22.7 C 27.3 C 23.0 C 28.0 C 23.3 Orienta Ave & Fairway Ln EB LR B 10.9 A 9.0 B 10.9 A 9.0 B 10.9 A 9.0 B 11.2 A 9.2 Bairway Ln NB LT A 0.1 A 0.0 A 0.1 A 0.0 A 0.0 A 0.1 A 0.0	(US Route 1) &	VV B	TR	D	42.7	D	39.3	D	43.3	D	39.7	D	44.0	D	39.9
Intersection T B 18.8 B 13.2 B 18.8 B 13.3 B 19.3 B 13.6 SB TR C 28.6 C 24.0 C 28.6 C 24.3 C 29.1 C 24.6 Intersection C 27.1 C 22.7 C 27.3 C 23.0 C 28.0 C 23.3 Orienta Ave & Fairway Ln EB LR B 10.9 A 9.0 B 10.9 A 9.0 B 10.2 A 9.2 Fairway Ln NB LT A 0.1 A 0.0 A 0.1 A 0.0 A 0.1 A 0.0 A 0.0 A 0.0 A <t< td=""><td></td><td>ND</td><td>L</td><td>В</td><td>18.8</td><td>В</td><td>13.8</td><td>В</td><td>18.9</td><td>В</td><td>14.0</td><td>В</td><td>19.3</td><td>В</td><td>14.3</td></t<>		ND	L	В	18.8	В	13.8	В	18.9	В	14.0	В	19.3	В	14.3
Intersection C 27.1 C 22.7 C 27.3 C 23.0 C 28.0 C 23.3 Orienta Ave & Fairway Ln EB LR B 10.9 A 9.0 B 10.9 A 9.0 B 11.2 A 9.2 Fairway Ln NB LT A 0.1 A 0.0 A 0.1 A 0.0	Rd/Richbell Rd	INB	Т	В	18.8	В	13.2	В	18.8	В	13.3	В	19.3	В	13.6
Orienta Ave & Fairway Ln EB LR B 10.9 A 9.0 B 10.9 A 9.0 B 10.9 A 9.0 B 11.2 A 9.2 Fairway Ln NB LT A 0.1 A 0.0 A 0.1 A 0.0 A 0.0 A 0.0 A 0.0 A 0.0 A 0.0 A 0.1 A 0.0		SB	TR	С	28.6	С	24.0	С	28.6	С	24.3	С	29.1	С	24.6
Fairway Ln NB LT A 0.1 A 0.0 A 0.1 A 0.0		Inte	rsection	С	27.1	С	22.7	С	27.3	С	23.0	С	28.0	С	23.3
	Orienta Ave &	EB	LR	В	10.9	А	9.0	В	10.9	А	9.0	В	11.2	А	9.2
(unsignalized) SB TR A 0.0	•	NB	LT	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0
	(unsignalized)	SB	TR	Α	0.0	А	0.0	А	0.0	А	0.0	Α	0.0	А	0.0

Source: VHB, using Synchro 9 software. Delay is reported in seconds per vehicle.



late and stime	Awww.eah	Lane	E	disting	No	-Build	E	Build
Intersection	Approach	Group	LOS	Delay	LOS	Delay	LOS	Delay
	50	L	D	45.4	D	45.7	D	45.8
	EB	TR	D	43.8	D	43.9	D	44.0
	WB	L	D	43.0	D	43.1	D	43.5
Boston Post Rd (US	VV B	TR	D	41.1	D	41.1	D	41.2
Route 1) & Hommocks	ND	L	D	47.5	D	49.8	D	49.8
Rd/Weaver St	NB	TR	С	32.8	С	33.1	С	33.4
	SB	L	С	27.1	С	28.2	С	29.2
	JD	TR	D	41.4	D	42.1	D	42.1
	Inte	rsection	D	38.9	D	39.4	D	39.6
Hamma dia Del 8 Facila	WB	LR	А	6.6	А	6.6	А	6.7
Hommocks Rd & Eagle Knolls Rd (unsignalized)	NB	TR	А	7.1	А	7.1	А	7.2
Kilolis ku (ulisiglializeu)	SB	LT	А	7.5	Α	7.5	А	7.7
Oriente Aug & Fast Cours	EB	LR	А	7.4	Α	7.4	А	7.6
Orienta Ave & East Cove Rd (unsignalized)	NB	LT	А	7.5	А	7.5	А	7.6
Nu (unsignalizeu)	SB	TR	А	7.5	Α	7.5	Α	7.5
	EB	L	D	45.4	D	45.2	D	45.1
Boston Post Rd (US	LD	R	В	13.1	В	13.0	В	12.8
	WB	L	D	40.1	D	40.3	D	40.5
Route 1) & Orienta		R	А	8.5	А	8.5	А	8.4
Ave/Delancey Ave	NB	TR	D	40.0	D	40.8	D	41.0
	SB	TR	С	20.9	С	21.2	С	21.4
	Inte	rsection	С	24.1	С	24.7	С	24.7
Old Boston Post Rd &	WB	L	А	9.3	А	9.3	А	9.6
Cooper Ave (unsignalized)	SB	LT	А	0.1	А	0.1	А	1.0
		L	D	40.8	D	41.6	D	42.2
	EB	R	А	9.6	А	9.6	А	9.6
	14/15	L	D	35.7	D	36.2	D	36.2
Boston Post Rd (US	WB	TR	С	26.2	С	26.7	С	26.5
Route 1) & Old Boston Post Rd/Richbell Rd		L	В	14.6	В	14.6	В	14.8
Post Ra/Richbell Ra	NB	Т	В	14.8	В	14.8	В	15.0
	SB	TR	С	24.7	С	24.7	С	24.9
	Inte	rsection	С	21.2	С	21.3	С	21.5
Oriente Aux O Fair	EB	LR	А	9.3	Α	9.3	А	9.5
Orienta Ave & Fairway	NB	LT	А	0.0	Α	0.0	Α	0.0
Ln (unsignalized)	SB	TR	А	0.0	А	0.0	А	0.0

Table 9 - Capacity Analysis Summary – Saturday Peak Hour

Source: VHB, using Synchro 9 software. Delay is reported in seconds per vehicle.

<u>Existing Conditions</u> - As indicated in Tables 8 and 9, under existing conditions, the signalized intersection of Boston Post Road and Hommocks Road/Weaver Street currently operates at an overall level of service (LOS) "E" during the AM peak hour. LOS "E" is also experienced on individual movements (eastbound and southbound left turn movements and northbound through movement) during the AM peak hour. The intersection operates at acceptable LOS "D" during the PM and Saturday hours, with all individual movements operating at LOS "D" or



better. The two other signalized study intersections operate at an overall LOS "C" during the peak hours.

At the unsignalized intersections, the minor street turning movements operate at LOS "B" or better during each peak hour.

<u>Future No-Build Conditions</u> - In the future, without the proposed residential development (No-Build conditions), but with the forecast increases in traffic volumes, there will be a slight increase in overall delays at the three signalized intersections along Boston Post Road, generally on the order of 2 seconds or less. The levels of service will remain unchanged from those experienced under existing conditions.

At the unsignalized intersections, the minor street turning movements will continue to operate at LOS "B" or better during each peak hour with imperceptible increases in delay of up to 0.1 seconds.

<u>Future Build Conditions</u> – In the future, with the added traffic from the Proposed Action, there will be a slight increase in overall delays at the three signalized intersections along Boston Post Road, generally on the order of 1 second or less. The levels of service will remain unchanged from those experienced under No-Build conditions.

At the unsignalized intersections, the minor street turning movements will continue to operate at LOS "B" or better during each peak hour with only minor increases in delay of 1.1 seconds or less.

Queuing Analysis

In addition to providing the level of service values, the Synchro analyses also provide a calculation of the average (50th percentile) and maximum (95th percentile) queues expected on individual lane groups. The queues and available storage lengths for the Existing, No-Build and Build volume conditions are summarized in **Tables 10 to 12**.



Table 10 – Summary of Existing Queues

			Available			Exis	sting				
		Lane	Storage	AM Pe	ak Hour	PM Pea	ak Hour	Sat Pea	ak Hour		
Intersection	Approach	Group	Length	50th	95th	50th	95th	50th	95th		
	50	L	145'	73'	112'	103'	178'	118'	198'		
	EB	TR	-								
Destau Dest Dd /UC	WB	L	150'	54'	87'	45'	93'	30'	66'		
Boston Post Rd (US Route 1) & Hommocks	VV D	TR	-								
Rd/Weaver St	NB	L	180'	49'	69'	75'	115'	70'	111'		
Nuj weaver st	IND	TR	-								
	SB	L	140'	135'	176'	30'	54'	21'	42'		
	30	TR	-								
Hommocks Rd & Eagle	WB	LR									
Knolls Rd (unsignalized)	NB	TR	N/A	- All-Way	stop inte	rsection - d	queue not	calculated	1		
(1)	SB	LT									
Oriente Aug 8 Feet Cours	EB	LR									
Orienta Ave & East Cove Rd (unsignalized) ⁽¹⁾	NB	LT	N/A - All-Way stop intersection - queue not calculated								
	SB	TR									
	EB	L	-								
	LD	R	70'	0'	61'	0'	37'	0'	40'		
Boston Post Rd (US Route 1) & Orienta	WB	L	450'	58'	110'	49'	99'	33'	74'		
Ave/Delancey Ave	VV D	R	450'	0'	70'	0'	74'	0'	59'		
	NB	TR	-								
	SB	TR	-								
Old Boston Post Rd &	WB	L	200'+	0'	1'	0'	0'	0'	1'		
Cooper Ave					-		-		-		
(unsignalized)	SB	LT	-								
	EB	L	-	67'	132'	36'	135'	38'	148'		
		R	140'	62'	121'	33'	122'	0'	51'		
Boston Post Rd (US	WB	L	100'	57'	113'	39'	139'	34'	131'		
Route 1) & Old Boston		TR	-								
Post Rd/Richbell Rd	NB	L	175'	40'	78'	10'	61'	11'	68'		
		Т	-								
	SB	TR	-								
Orienta Ave & Fairway Ln (unsignalized)	EB	LR	450'+	0'	1'	0'	0'	0'	1'		
	NB	LT	-								
	SB	TR	-								

Note: (1) Synchro does not provide queue length calculations for movements at all-way stop intersections. However, the low volume of traffic and Level-of-Service "A" conditions suggest average queues of 25 feet or less and 95th percentile queues of 50 feet or less.



Table 11 – Summary of No-Build Queues

			Available			No-	Build				
		Lane	Storage	AM Pe	ak Hour	PM Pea	ak Hour	Sat Pea	ak Hour		
Intersection	Approach	Group	Length	50th	95th	50th	95th	50th	95th		
		L	145'	74'	115'	104'	179'	120'	201'		
	EB	TR	-								
		L	150'	56'	90'	46'	94'	30'	68'		
Boston Post Rd (US	WB	TR	-								
Route 1) & Hommocks Rd/Weaver St		L	180'	49'	70'	76'	118'	71'	113'		
Ruj Weaver St	NB	TR	-								
	SB	L	140'	138'	179'	30'	55'	21'	43'		
	28	TR	-								
	WB	LR		-				-	•		
Hommocks Rd & Eagle Knolls Rd (unsignalized)	NB	TR	N/A	A - All-Wo	ay stop inte	ersection -	queue no	t calculate	ed		
	SB	LT									
Oriente Aug 8 Fest	EB	LR									
Orienta Ave & East Cove Rd (unsignalized)	NB	LT	N/A	N/A - All-Way stop intersection - queue not calculated							
Cove Ru (unsignalizeu)	SB	TR									
	EB	L	-								
Destan Dest Dd /UC	LD	R	70'	0'	62'	0'	38'	0'	40'		
Boston Post Rd (US Route 1) & Orienta	WB	L	450'	60'	111'	50'	100'	33'	74'		
Ave/Delancey Ave	VV D	R	450'	0'	70'	0'	75'	0'	60'		
Ave, belance y Ave	NB	TR	-								
	SB	TR	-								
Old Boston Post Rd &	WB	L	200'+	0'	1'	0'	0'	0'	1'		
Cooper Ave (unsignalized)	SB	LT	-								
		L	-	68'	135'	37'	136'	39'	151'		
	EB	R	140'	64'	124'	34'	123'	0'	52'		
Boston Post Rd (US	14/5	L	100'	58'	115'	40'	141'	35'	133'		
Route 1) & Old Boston	WB	TR	-								
Post Rd/Richbell Rd		L	175'	40'	78'	10'	63'	12'	69'		
	NB	Т	-								
	SB	TR	-								
	EB	LR	450'+	0'	1'	0'	0'	0'	1'		
Orienta Ave & Fairway	NB	LT	-								
Ln (unsignalized)	SB	TR	-								
	55							1	1		

Note: (1) Synchro does not provide queue length calculations for movements at all-way stop intersections. However, the low volume of traffic and Level-of-Service "A" conditions suggest average queues of 25 feet or less and 95th percentile queues of 50 feet or less.



Table 12 – Summary of Build Queues

			Available			Bu	uild				
		Lane Storage		AM Peak Hour		PM Peak Hour		Sat Pea	ak Hour		
Intersection	Approach	Group	Length	50th	95th	50th	95th	50th	95th		
		L.	145'	74'	121'	104'	180'	120'	202'		
	EB	TR	-								
		L	150'	78'	134'	54'	108'	39'	81'		
Boston Post Rd (US Route 1) &	WB	TR	-								
Hommocks Rd/Weaver St		L	180'	49'	70'	76'	118'	71'	113'		
	NB	TR	-								
	SB	L	140'	138'	179'	31'	55'	22'	44'		
	28	TR	-								
Hommocks Rd & Eagle Knolls	WB	LR									
Rd (unsignalized)	NB	TR	N/A	- All-Way	stop inte	rsection - a	queue not	calculated	1		
Ku (ulisignalizeu)	SB	LT									
Orienta Ave & East Cove Rd	EB	LR									
(unsignalized)	NB	LT	N/A - All-Way stop intersection - queue not calculated								
(unsignalized)	SB	TR									
	EB	L	-								
		R	70'	0'	61'	0'	40'	0'	41'		
Boston Post Rd (US Route 1) &	WB	L	450'	60'	111'	50'	100'	33'	75'		
Orienta Ave/Delancey Ave	VVD	R	450'	0'	73'	0'	76'	0'	62'		
	NB	TR	-								
	SB	TR	-								
Old Boston Post Rd & Cooper	WB	L	200'+	0'	5'	0'	2'	0'	2'		
Ave (unsignalized)	SB	LT	-								
	EB	L	-	70'	138'	38'	138'	40'	155'		
	LD	R	140'	64'	124'	34'	124'	0'	52'		
Boston Post Rd (US Route 1) &	WB	L	100'	64'	123'	41'	144'	36'	137'		
Old Boston Post Rd/Richbell	VV D	TR	-								
Rd	NB	L	175'	42'	78'	11'	63'	12'	69'		
		Т	-								
	SB	TR	-								
Orienta Ave & Fairway Ln	EB	LR	450'+	0'	1'	0'	0'	0'	1'		
(unsignalized)	NB	LT	-								
(unsignalized)	SB	TR	-								

Note: (1) Synchro does not provide queue length calculations for movements at all-way stop intersections. However, the low volume of traffic and Level-of-Service "A" conditions suggest average queues of 25 feet or less and 95th percentile queues of 50 feet or less.

The existing queues provided in Table 10 were compared to the available storage lengths which indicated that the maximum (95th percentile) queue exceeded the provided storage at two intersections. During the AM peak hour at the Boston Post Road intersection with Hommocks Road and Weaver Street, the southbound left turn queue is 176 feet where the available storage is 140 feet. The eastbound left-turn from Weaver Street exceeds the 145-foot available storage during the PM (178 feet) and Saturday (198 feet) peak hours. At the Boston Post Road and Old Boston Post Road/Richbell Road intersection, the calculated maximum queue for the



westbound left turn from Old Boston Post Road exceeds the available 100-foot left-turn storage during the AM (113'), PM (139') and Saturday (131') peak hours. The average (50th percentile) queues at all locations is less than the available storage. At the unsignalized intersections, the queue lengths measure less than the provided storage.

As indicated in Table 11, under future No-Build conditions, with the forecast increases in traffic volumes, there will be a slight increase in the length of the queues at the three signalized intersections along Boston Post Road, generally on the order of 3 feet or less. The average (50th percentile) queues at all locations will remain at acceptable lengths. At the unsignalized intersections, the 50th and 95th percentile queue lengths will continue to be acceptable.

As indicated in Table 12, under future Build conditions, with the added traffic from the Proposed Action, at the three signalized study locations there will be a slight increase in the length of the maximum (95th percentile) queues on the turning lane movements that exceeded the available storage under No-Build conditions, generally on the order of 8 feet or less. On Boston Post Road, the maximum queue on the southbound left turn into Hommocks Road currently exceeds the available storage area during the AM peak hour and will continue to do so in the future without the project. The Proposed Action will not add any traffic to this movement during the AM peak hour; therefore, the backups will not increase from future No-Build conditions. The Proposed Action will not have any impacts on this movement during the PM and Saturday peak hours as only 1 vehicle will be added during each peak hour.

The average (50th percentile) queues at all locations will remain at acceptable lengths. At the unsignalized intersections, the 50th and 95th percentile queue lengths will continue to be acceptable.

Sight Distance Analysis

Sight distance analyses were conducted at the four unsignalized study intersections to determine if sufficient sight lines are provided. The sight distances at each location were measured and compared to the requirements provided in the American Association of State Highway and Transportation Officials' (AASHTO) publication, *A Policy on Geometric Design of Highways and Streets (2011)*. Two of the intersections are controlled by Stop signs on all approaches (Orienta Avenue and East Cove Road; Hommocks Road and Eagle Knolls Road). Per AASHTO, at these two all-way stop intersections, the first stopped vehicle on one approach should be visible to the drivers of the first stopped vehicles on the other approaches. At the two other unsignalized intersections (Orienta Avenue and Fairway Lane; Old Boston Post Road and Cooper Avenue). Stop signs are provided on the minor street approaches (Fairway Lane and Cooper Avenue). AASHTO sight distance requirements at these locations are generally based on travel speeds, grades, number of lanes to cross and type of traffic control. The sight distance analysis is summarized in **Table 13**.



Intersection	Control	Approach/	Sight Distance			
intersection	control	Movement	Required	Available		
Orienta Avenue & East Cove Road	All-way Stop	All approaches	First stopped vehicle visible	Yes		
Hommocks Road & Eagle Knolls Road	All-way Stop	All approaches	First stopped vehicle visible	SB – Yes NB & WB – No ⁽¹⁾		
Orienta Avenue & Fairway Lane	Stop (Fairway Ln)	EB LR	280' looking left 280' looking right	410' left 512' right		
Old Boston Post Rd & Cooper Avenue	Stop (Cooper Ave)	NB L	280' to the right	120' right ⁽¹⁾		

Table 13 - Sight Distance Analysis

Note: Required sight distances based on AASHTO publication, *A Policy on Geometric Design of Highways and Streets (2011)*. (1) – Sight distance can be increased to the minimum required by the removal of foliage.

As shown in Table 13, acceptable sight distances are provided at the Orienta Avenue and East Cove Road all-way stop intersection. At the Hommocks Road and Eagle Knolls Road all-way stop intersection, the drivers on the Eagle Knolls Road approach and the northbound Hommocks Road approach have somewhat limited visibility due to foliage on the southeast corner of the intersection which partially obstructs the view, as indicated in the photograph below. If a small bush at the corner of the intersection were removed and the tree next to it pruned so the branches do not hang down within 4 feet of the ground, adequate sight distance would be provided.





At the intersection of Cooper Avenue with Old Boston Post Road, a lot of vegetation has grown since the August 2013 photograph below was taken. This new vegetation has significantly reduced sightlines and should be removed to restore the required 280 feet of sight distance.



For the on-site intersections, a review of the site plan indicates that a minimum of 200 feet can be provided from all intersections which will be sufficient to accommodate vehicles traveling at the posted Village-wide speed limit of 30 mph.



6

On-Site Roadways and Circulation

Site Roadways and Intersections

Site Roadways

As noted previously, the three existing access points to the project site (Cove Road, Eagle Knolls Road and Cooper Avenue) will be modified as part of the Proposed Action. The privatelyowned portion of Cove Road within the Project site will be relocated and will form the central corridor for the project. Eagle Knolls Road will be relocated from its existing location and will intersect with the relocated Cove Road prior to terminating in a cul-de-sac. Cooper Avenue will be extended into the Site and will intersect with Cove Road. This roadway extension is currently envisioned to be a one-way, exit only road for development residents to provide access to Boston Post Road (US Route 1) via Old Boston Post Road. A new internal roadway, "Road A", will intersect with Cove Road and terminate in a cul-de-sac.

Each roadway will be 28 feet wide, and, cumulatively, the roadways in the development will be able to accommodate 125 parked vehicles. From a practical perspective, as occurs in many similar developments, on-street parking will, in most circumstances, be limited to the occasional vehicle scattered around the development (a total of 241 parking spaces are required by the Code – 2.3 per unit while each unit will have 4 parking spaces – for a total of 420). Thus, the 28-foot wide roadways will be sufficient to provide one 10-foot wide lane for travel in either direction while allowing 8 feet on one side of the road or the other for a car to be parked. Cyclists, for the most part, will travel in the outside 5 feet of each lane (leaving adequate width to accommodate two-way traffic), negotiating the occasional parked vehicle. Share the road signage could be added if the volume of cycling activity justifies it. Cyclists may also choose to cycle on the development's sidewalks, as permitted under Village and NY State law, provided that the bicycles are not operated "in a manner that is unsafe for pedestrians" (Village of Mamaroneck Code §112-2 B.)

At its west end, Cove Road will narrow down as it leaves the property to match the existing section width. The relocated Cove Road will have a sidewalk run along its entire length. Each internal intersection will be designed to provide sufficient sight distance for vehicles traveling within the site.

At the present time, the portions of Eagle Knolls Road, Cove Road and Cooper Avenue within the Project Site are private roads. In the future, with the proposed Project and planned



modifications to these roadways, those portions of the road within the Project Site will remain as private roads. The proposed homeowners' association will be responsible for maintenance of the roadways within the Project Site.

With respect to rights of access over those portions of Eagle Knolls Road and Cove Road under private ownership, the proposed Project will not prohibit the area residents who currently use the private roads to access Hommocks Road from Eagle Knolls Road or the public portions of Cove Road beyond the Project Site.

There are currently three (3) private homes on Eagle Knolls Road, two of which will be to the west of the intersection of realigned Cove Road with Eagle Knolls Road and one of which will be on the cul-de-sac section of Eagle Knolls Road. The proposed termination of Eagle Knolls Road will require residents of and visitors to the one private home on Eagle Knolls Road which lies to the east of the intersection of realigned Cove Road with Eagle Knolls Road to travel approximately 100 feet to the west on the Eagle Knolls Road cul-de-sac to connect to the external roadway network. This is expected to have almost no impact on the residents of this home. The only impact of the termination of Eagle Knolls Road in a cul-de-sac for the residents of the two private homes to the west of the intersection of realigned Cove Road is that they will have to turn left onto realigned Cove Road when they are headed to the Orienta Avenue neighborhood of the Village or to the clubhouse, instead of proceeding straight.

The improved Cove Road, including the proposed sidewalk, will greatly enhance east-west access for both motorists and pedestrians who live on either side of Hampshire Country Club. In addition, the Proposed Action will significantly improve the safety of Eagle Knolls and Cove Road by elevating low-lying portions of these roads above the floodplain. The road pavement conditions will be upgraded from their present condition.

Emergency access and evacuation will be provided via the three access routes to the Project Site. These roadways will be designed so that fire trucks and other emergency vehicles will be able to easily access and circulate within the Site. Elevating Cove Road will also improve emergency evacuation for the entire neighborhood.

Site Intersections

A qualitative analysis was conducted at the three newly created "T" intersections with Cove Road (Cooper Avenue Extension, Road "A" and Eagle Knolls Road) to identify future traffic operating conditions. Each approach at the three intersections will have one lane with Stop signs controlling the minor leg approaches (Cooper Avenue Extension, Road "A" and Cove Road at its intersection with Eagle Knolls Road). The project-generated traffic volumes were assigned to the internal intersections based on the distributions identified on Exhibit 12 and the location of the residential units along the internal roadways. The project trips were then added to the No-Build volumes to develop the Build volumes on the internal roads. A review of the Build volumes along the relocated Cove Road indicates that the AM peak hour volumes are 72 percent higher than the PM peak hour volumes and 52 percent higher than the Saturday peak hour volumes (primarily as a result of traffic to and from the Hommocks Middle School).

A Synchro analysis was conducted with the higher AM peak hour volumes which indicate that the minor street approaches at all three internal intersections will operate at level of service A



Level of service "A" generally means that queuing on a minor street approach is rare and that there are little or no delays. A further analysis was conducted in which the AM peak hour volumes were increased by a magnitude of five. This sensitivity analysis indicated that, even with the substantial increase in traffic volumes, the minor street approaches at each intersection would operate at acceptable LOS B. During the PM and Saturday peak hours, it can be concluded that traffic operating conditions will be better than the AM peak hour conditions as the PM and Saturday volumes are much lower than the AM volumes.

Pedestrian and Bicyclist Circulation

Pedestrian and bicycle circulation would be facilitated on the Project Site through the redeveloped and improved road network. The Proposed Action would include sidewalks on the north side of the extended and rerouted Cove Road, which would provide a path for residents and children biking or walking through the proposed development to access community facilities nearby, including Hommocks Middle School, Hommocks Ice Rink and Hommocks Pool, and the commercial corridor along Boston Post Road/U.S. Route 1. The other proposed roadways, which will be very low volume roadways (less than 1 vehicle every 2 minutes during the busiest hour) would not include sidewalks or bicycle pathways. This is in keeping with much of the road network immediately surrounding the Project Site, primarily the portions of Hommocks Road, Cove Road, Cooper Avenue, and Fairway Lane immediately adjacent to the Project Site, which do not contain designated bicycle pathways or sidewalks. The existing and proposed roadway network would also be wide enough to accommodate on-road cycling (as discussed in "Site Roadways" earlier in this Chapter).

7

Parking

Existing Parking

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The existing parking at the Hampshire Country Club is located, primarily, in parking lots adjacent to the clubhouse. A total of 207 permanent (lined) parking spaces are provided. The parking provided for the membership club meets the zoning requirements for the MR district. Although public parking on the private roads within the property is prohibited by the Country Club, during larger events at the clubhouse, when valet parking is provided, parking for an additional 50 vehicles can be accommodated along these roadways within the property as a contingency measure to ensure that cars are never parked along the portion of the roads shared by adjacent neighbors. Valet Parking on property would occur on a very limited basis, generally once or twice a year, such as at the member's annual Memorial Day barbecue.

The club has an active social calendar with over 160 events scheduled annually (predominantly for member events but occasionally for outside/community groups). While there is a large variety in the club's social events, parking surveys were conducted on the property for two events which were deemed representative of both regular and larger events. The parking surveys were conducted on Thursday August 17, 2017 during a weekday non-member event (50-person golf outing) and on Saturday evening August 19, 2017 during a large member event (200-person wedding). During the weekday event, parked vehicles from members participating in Club activities (tennis, swimming pool, etc.) were also counted in the parking surveys. Member activity at the Club during the Saturday evening event (i.e., members using the club facilities that were not attending the wedding) was minimal. The existing parking demand is summarized in **Table 14**. The parking data is provided in the Appendix.

	Non-Member Event Parking Demand ⁽¹⁾	Member Event Parking Demand ⁽²⁾
Maximum Parking Demand	95	120 ⁽²⁾

Table 14 – Existing Parking Demand

Notes: (1) Thursday August 17, 2017 parking surveys for a weekday Golf outing with 50 participants (indudes parked vehicles from members participating in other Club activities such as tennis, swimming pool, etc.).

(2) Expected large member event parking demand is 120 vehicles, but the 200-attendee wedding surveyed on Saturday August 19, 2017 had a demand of only 90 vehicles.



As indicated in Table 14, a total of 95 vehicles were parked during the weekday event. Although 90 vehicles were parked at the Saturday member event, the expected parking demand for a large member event is 120 vehicles.

Proposed Parking

<u>Country Club Parking</u> - In the future, with the Proposed Action, a total of 163 parking spaces would be provided at the clubhouse and parking for an additional 16 vehicles will be available during large club events, for a total of 179 spaces. Parking regulations, per Village Code §342-56(A), require 2 spaces for each 3 individual, family or other type of memberships. The club had 264 memberships as of 2017 which require 176 parking spaces per the Village code. With the downsizing of the golf course offset by the potential new memberships generated by the planned residential development, it is anticipated that the membership total will remain at its current level in the future with the Proposed Action. Therefore, the 179 parking spaces to be provided will be in compliance with Village parking requirements. The clubhouse's banquet hall can accommodate up to 250 guests for weddings or other events. The 179 parking spaces will also be able to accommodate the parking for events.

<u>Residential Parking</u> - For the PRD, four spaces will be provided for each residential unit, including two in the driveway and two in the garage, yielding 210 enclosed spaces and 210 driveway apron spaces for a total of 420 private residential parking spaces. In addition, on-street parking within the PRD development will be permitted on one side of all streets (2 x 10 foot travel ways and 8 feet for parking). It is calculated that parking for approximately 125 vehicles will be able to be accommodated on street.

Village Code §342-52(I) states that "Off-street parking shall be provided within each planned residential development at the rate of not less than two spaces for each one-family detached dwelling, and one space per dwelling unit, plus one-half (1/2) space per bedroom for each dwelling unit in an attached or semi-detached dwelling. No less than one-third (1/3) nor more than two-thirds (2/3) of the minimum required off-street parking spaces shall be enclosed. Of the unenclosed spaces, an amount equal to at least one-third (1/3) of the total number of required spaces shall not be reserved for the use of specific dwelling units and shall, at all times, remain open and available for the use of visitors and guests, as well as other residents."

Applying the Code mandates that a minimum of 241 parking spaces be provided, 88 for the single family homes and 153 for the semi/attached carriage houses, each of which has 3 bedrooms. Between 80 and 160 of the required parking spaces must be enclosed and at least 80 of the unenclosed parking spaces must be available for use by anyone.

A total of 545 parking spaces (420 private + 125 on-street) are proposed for the PRD, which is well more than the 241 required. The 125 vehicles which will be able to be accommodated on street will be well more than 80 required for use by any one at any time.

8

Conclusions

Based on the results of the analyses conducted for the purpose of this report, VHB has arrived at the following conclusions:

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- Under existing conditions, the Boston Post Road intersection with Hommocks Road/Weaver Street currently operates at an overall level of service "E" during the AM peak hour with notable delays on the eastbound and southbound left turn movements and northbound through movement. All other intersections currently operate at acceptable LOS C" or better during the peak hours.
- Queuing analyses for the existing condition indicate that, at all study locations, the average queues (50th percentile) do not exceed the available storage lengths. The maximum (95th percentile) queues exceed available storage lengths at 2 signalized intersections.
- In the three-year period from 2013 through 2015, a total of 112 crashes occurred in the study area with 79 crashes at the study intersections. Most of the crashes (90 percent) occurred on Boston Post Road and the study intersection experiencing the highest number of crashes was the Boston Post Road intersection with Richbell Road/Old Boston Post Road with a total of 43 crashes. There were 39 crashes with injuries and there were no fatalities. Eight crashes involved pedestrians and 4 crashes involved bicyclists.
- Sight distance analyses at the unsignalized study intersections indicate that, with appropriate landscaping modifications, sufficient sight distance can be provided at all locations.
- In the future, under No-Build conditions and with the forecast increases in background traffic and traffic from 7 vicinity developments, compared to existing conditions, there will be a slight increase in overall delays, generally on the order of 2 seconds or less.
- Queuing analyses under No-Build conditions indicate that there will be a slight increase in the length of the queues at the three signalized intersections along Boston Post Road, generally averaging 3 feet or less.
- The proposed Project will add 61 new trips to the surrounding roadways during the weekday AM peak hour, 73 new trips during the weekday PM peak hour and 61 new trips during the Saturday midday peak hour.



- ➢ In the future, under the Build condition with the added traffic from the Project, there will be only a slight increase in overall delays at the signalized locations during the peak hours, generally on the order of 1 second or less. The minor street movements at the unsignalized intersections will operate at level of service "B" or better during each peak hour with only minor increases in delay of 1.1 seconds or less.
- Queuing analyses with the added traffic from the proposed project indicate that there will be a slight increase in the length of the maximum queues on the turning lane movements that exceeded the available storage under No-Build conditions, generally averaging 8 feet or less. The average queues at all locations will continue to be at acceptable lengths.
- The proposed Project will include an improved internal roadway network which will be wide enough to accommodate on-road cycling. A sidewalk will be constructed on the north side of Cove Road thereby providing a path for residents and children biking or walking through the proposed development to access community facilities nearby and the nearby Boston Post Road commercial corridor.
- Providing a two-way site access from Cooper Avenue will reduce project traffic past the Hommocks Middle School and through the busy intersection of Boston Post Road with Hommocks Road/Weaver Street.
- The proposed development will not have a significant adverse impact on area traffic operating conditions. However, the Applicant is proposing to implement the following improvements:
 - Improved internal roadway network which will be wide enough to accommodate on-road cycling.
 - Improved road surface, profile and alignment of Cove Road across the site for residents on either side of the property, including those who travel back and forth to Hommocks Middle School;
 - Improved pedestrian environment with the completion of a sidewalk across the property;
 - Improved emergency evacuation routes with the raising of Cove Road above the flood elevation.

Based on these findings, it is concluded that the proposed action will not have a significant adverse impact on area traffic operating conditions.



Appendix

Description

Safe Routes to School Information







SRTS Goals

✓ Where it's safe, get children walking and biking

✓ Where it's not safe, make changes



SRTS Partnership

Cross section of community leaders

- ☑ Elected officials
- ✓ Municipal employees
- ✓ Law enforcement
- ✓ School administrators
- ☑ PTA representatives
- ☑ Rye YMCA





SRTS: The 5 E's

- ✓ Engineering
 ✓ Enforcement
- ✓ Enforcement
- ✓ Education
- ✓ Encouragement
- ✓ Evaluation



L/M SRTS Programs

Where it's not safe, make changes Engineering









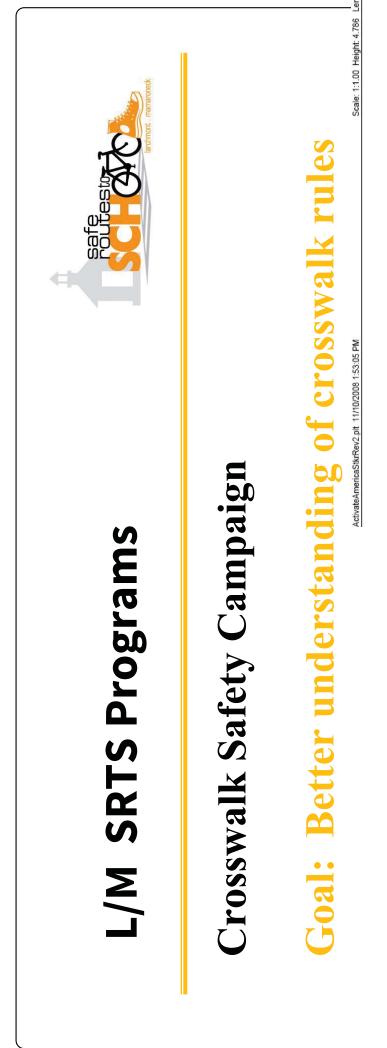
L/M SRTS Programs

Safety Pledge

Goal: Long term behavior change!

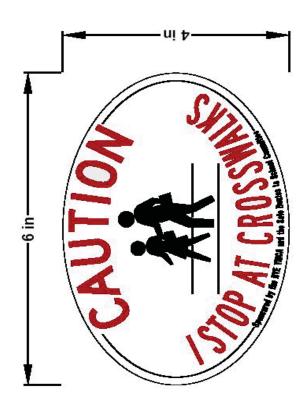
- ✓ Educate parents
- ✓ Educate children
- Increase awareness among drivers and pedestrians $\mathbf{>}$

	Students' Safety Pledge I promise to:	 Always follow the directions of the crossing guard or traffic signals Always cross the street in a crosswalk Always look left, right and left again before crossing the street Always look before crossing the end of a driveway Always look before crossing the end of a driveway Never dart out between parked cars Never dart out between parked cars Never at belt when riding in a car Always wear a bike helmet when riding my bike Always use hand signals when turning on my bike
L/M SRTS Programs	Parents' Safe Driving Pledge I promise to:	 Never speed through residential streets and school zones Never talk or text on a hand-held device while driving Never talk or text on a hand-held device while driving Always wear a seat belt and make sure passengers wear seat belts Always yield to students walking or bicycling, especially in crosswalks Never run stop signs or red lights Never park or stop in crosswalks Never lights Never leave the driver's seat when dropping off or picking up Never park illegally (i.e.at a yellow, red or blue curb) Never leave the car idling for more than 3 minutes



✓Educate drivers & pedestrians about correct crosswalk usage

✓Reach elementary & middle school students and parents



the basic rules of STOP, LOOK and LISTEN. Our community encourages children to walk to school as much as possible How safe is your student when crossing a busy street as a pedestrian or cyclist? We parents need to teach more than Crosswalk Safety: Myth vs. Reality

– and we want them to arrive safely. Please review these procedures with your children and follow the rules yourself – you are the best role model for your children.

Myth: Pedestrians always have the right of way.

Reality: No, not always. Legally, pedestrians have the right-of-way within a crosswalk if there is no traffic light. If there is a traffic light, whoever has the green light - pedestrian or car- has the right of way. Pedestrians are also expected to exercise "due care" for their own safety, and are NOT permitted to suddenly obstruct the path of a moving vehicle that is close enough to be a hazard.

Myth: You are safe in a crosswalk.

Reality: Painted lines do not protect you from harm, even if you have the legal right of way. This is extremely important at crosswalks where there is no traffic signal or stop sign. If you are not crossing at a marked crosswalk, pedestrians do not have the right of way and must yield to vehicles. Your best protection is your own attention!

Myth: A green light or walk signal means "GO"

Reality: A green light or walk signal indicates that it is your turn to cross, but first make sure that the intersection is clear -- and watch for red light runners! Also, make sure that any right-turning cars will yield to you. Cyclists should dismount and walk across if they wish to cross a busy intersection.

Myth: If you see the driver, the driver sees you. Reality: The driver may not see you in time to stop. To be safe, make eye contact with any driver whose path will cross yours, and proceed only when certain the car will stop. If there is a median, make separate decisions

Additional Safety Tips:

about crossing each direction of traffic.

Always walk on the sidewalk; if there is no sidewalk, walk facing traffic
Bicycles ride in the same direction as cars
Cross only at corners: avoid the dangerous practice of "jaywalking" or crossing between parked cars
Continue to look Left, Right, and Left again as you cross, it's easy to miss an oncoming car
When you are near the street, don't push, shove, or chase your friends
Watch out for cars and trucks at every driveway and intersection as you walk.





L/M SRTS Programs



Crosswalk Safety – A few tips to make your walk to school SAFE:

pedestrians have the right-of-way in a crosswalk, but make eye contact with the driver to be sure he/she •When there is a traffic light, whoever has the green light has the "right-of-way". If there is no light, sees you

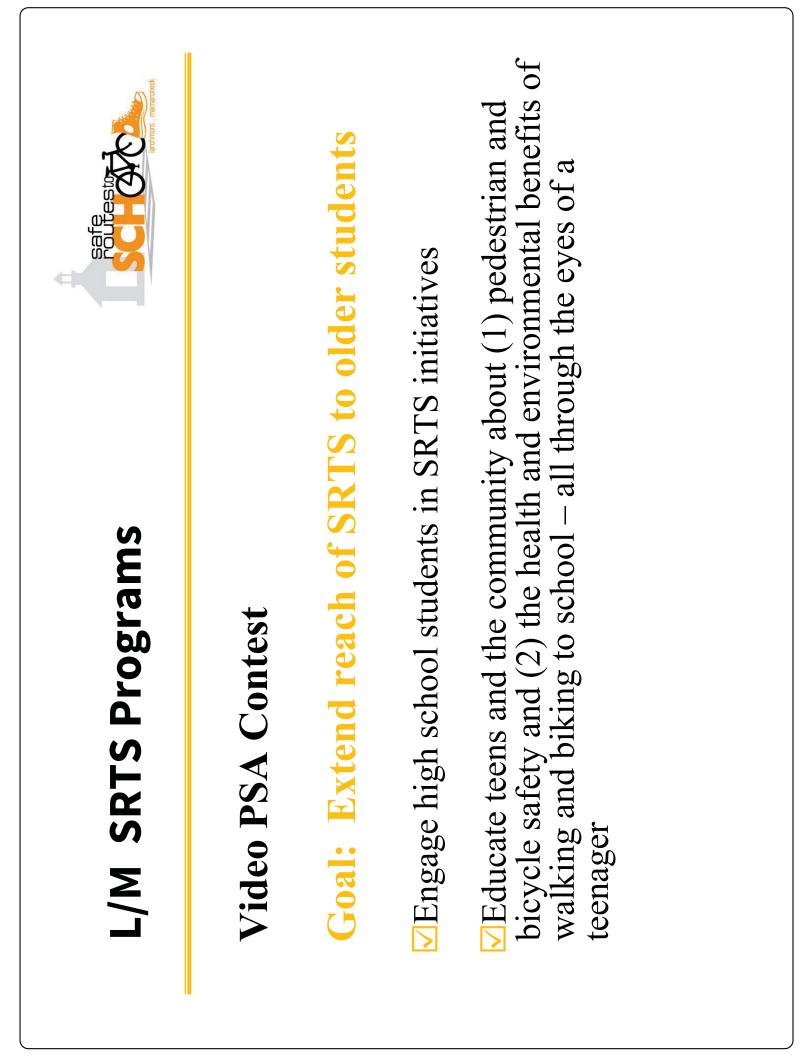
•A walk signal does mean it's your turn to cross, but check to make sure there are no cars coming. •Just because you have the right-of-way doesn't mean a car may not still be a hazard. Your best protection is your own attention •Always walk on the sidewalk. If there is no sidewalk, walk facing traffic. (Bicycles ride in the same direction the cars drive.)

•Cross only at corners or crosswalks. Always look both ways.

• Bikers and skateboards should always get off their bike/skateboard and walk when crossing a busy intersection.

- When you are near the street, don't push, shove or chase your friends.
- Crossing the street involves your feet, but also your eyes and ears... Don't wear headphones or text while crossing the street!

•It's good for your health and the health of the planet! Stay fit while reducing your carbon footprint!





L/M SRTS Video Contest Winners!





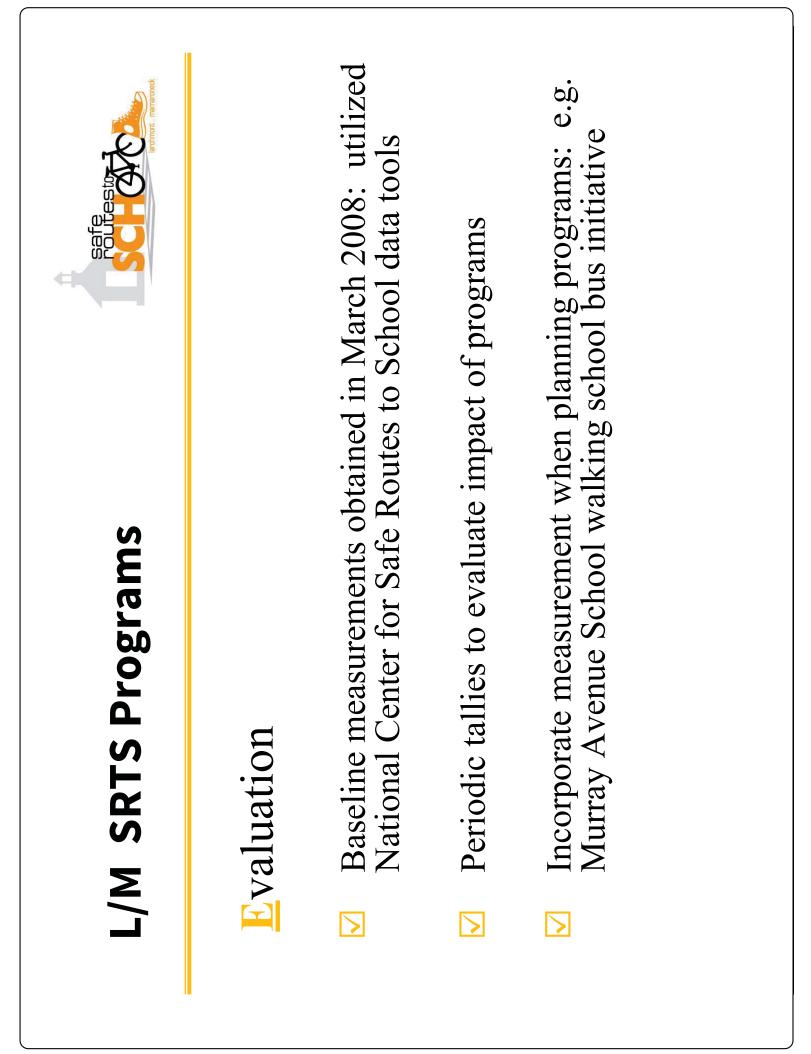


L/M SRTS Programs

Walking School Buses: a year-round initiative







We will continue to search for grants to fund infrastructure and programs
State Farm "Good Neighbor Grant": expansion of walking school bus program
✓ National Center for Safe Routes to School mini-grant: expansion of walking school bus program
✓ <u>Mamaroneck Schools Foundation</u> : traffic study
✓ <u>Transportation Enhancement Program</u> : Old Boston Post Road curbing and sidewalk
✓ NYS DOT Safe Routes to School: Hommocks and Central School sidewalks and all-elementary SRTS programs
Collaboration with MUFSD and municipalities to seek funding:
L/M SRTS: Seeking Support



L/M SRTS: The Word is Out



18 • THE SOUND AND TOWN REPORT • October 16, 2005

that parents in the school School (SRTS) Comm

Mamaroneck and Larchmont's Weekly Newspape October 16, 2009

Elections 2009 www.mysoundandtown.com

Packed house at Mamaroneck Village Board Trustees adopt local law, address parking

Vol. 11/Nambar 41

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Shoes trump cars during sworth's Walk to School Week



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duy": high sets years war 2000. All of our year walless sets years also helps challenels are the post also helps of the of the or the in addition to be all in the is sets collined, valida or in or all or addition to the all of the ord all or addition to the all of the ord all or addition to the all of the ord all or additions are all of the ord and the ord the ord the ord the orden all challenels. All of the ord of the orden all challenels. All of the ord of the orden all challenels. All of the orden all of the orden all challenels. All of the orden all of the orden all challenels. All of the orden all of the orden all challenels. All of the orden all of the orden all challenels. All of the orden all challenels.

will slow the driv end, the walkers

and estimate and the provident of the envi-destination safety, concern for the envi-rant and "the positive impact of build-connections between families, schools as broader community," according to a trues treater notin the second memory friggerald noted that the message was also necked at drivers, to remind them to "be autious, that they're in a community that accurages walking to school. Hopefully it

from the school dist

explained that the purpose a was "to create awareness of "arking to school," which the henefits of daily exercise.

All "log-tu ... school building on wear-e level that has the most walke ----unced over the PA s

40. -40. -41395 in a s. -40 a work throughout th. -40 a work through through the -40 a work through through through the -40 a work through through through the -40 a work through through

The committee will continue to en valking and biking to school, F aid. "In order to capture the enthu Math to School Week we will be on

In the Chatsworth School yard, boys poully display earned by all who walled or biled to school.

PATMINGENE 2000 Constrained and a second constrained and a second constraint of the second co

By hulls BRYT Mun Toon Superviser Valents from sch Mun Toon Superviser Valents Orkedt as a subti 1950, and a that Areas School in the Stock and a sch the walked to School even is the cannot are stati-ted and school versite and school and that Back that, O'Keeffe and s5 be-Multi-

New Domain - New Design Same Great Coverage!

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COMMUNITY BRIEFS 5 CLASSIFIED/PUZZLE 16 SPORTS 19

IN THIS ISSUE

A little wet weather didn't deter these walking school bue down the sidewalks to Murray Arenue School from both di



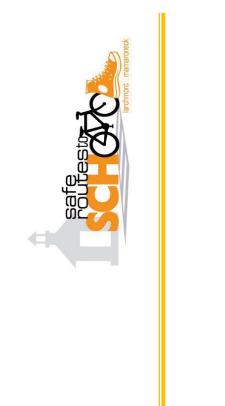
L/M SRTS: The Word is Out

Mam'k Walking School Buses a Model at County Conference (Larchmont Gazette 10/29/09)

Transportation at the County Center in White Plains. Ms. Larsen presented Larchmont's walking school bus program to well over 100 conference participants who included state, county and city transportation officials as well as school administrators _archmont resident Kim Larsen, who chairs the Larchmont and Mamaroneck Safe Routes to School (SRTS) Committee, was a featured speaker at the October 16, 2009 SRTC conference hosted by the Westchester County Department of from all over the Lower Hudson Valley. Kim Larsen displays some of the signs used during Walk to School Week in the Mamaroneck School District. "This month's Larsen. "We've found that children really enjoy the sociability of the walking school bus. It adds another layer of fun to the district-wide Walk to School Week featured numerous walking school buses at each of elementary school," reported Ms. walk to school."

"Larchmont has developed a fun and creative walking school bus program and we hope other communities will soon follow organizer of the conference. Additional speakers included Westchester County Executive Andy Spano, Transportation their lead," commented Naomi Klein, principal planner for the Westchester County Department of Transportation and Commissioner Lawrence Salley and County Health Commissioner Dr. Joshua Lipsman.



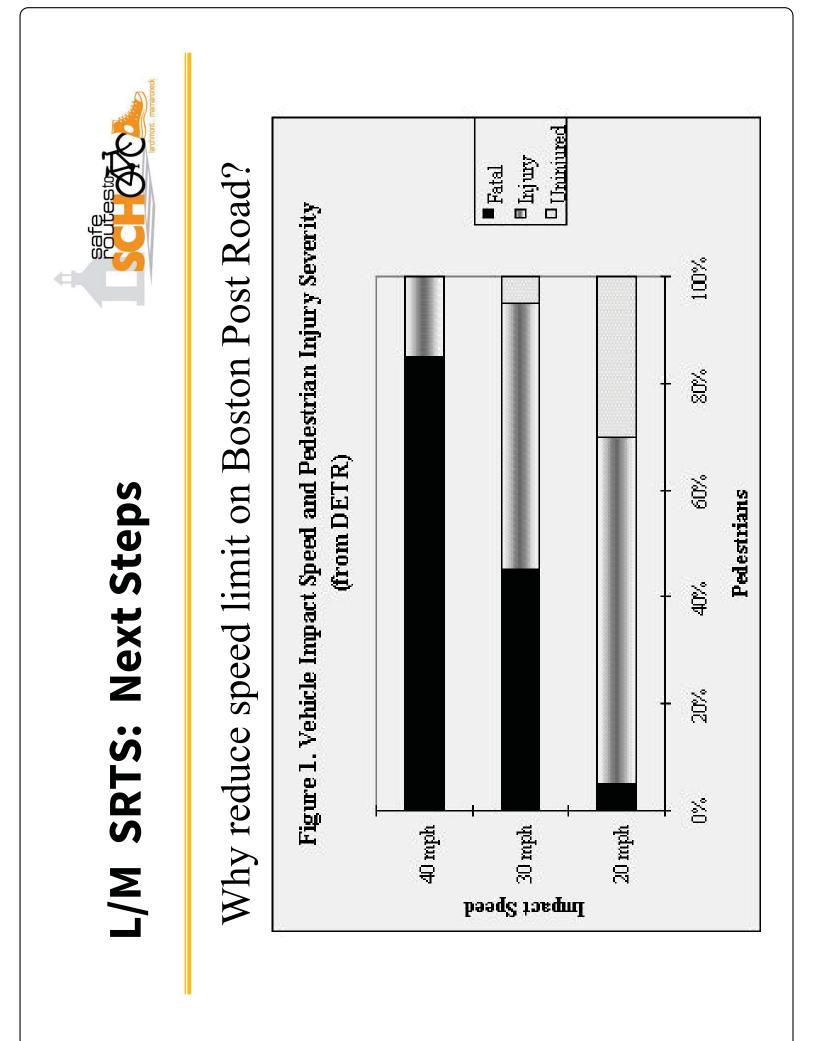


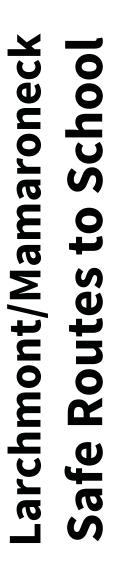
L/M SRTS: Next Steps

Education and Encouragement at middle and high school $\mathbf{>}$

level

- Driver Awareness programs >
- Traffic Calming on Boston Post Road: Speed limit reduction during school hours in vicinity of MHS $\mathbf{>}$









Appendix

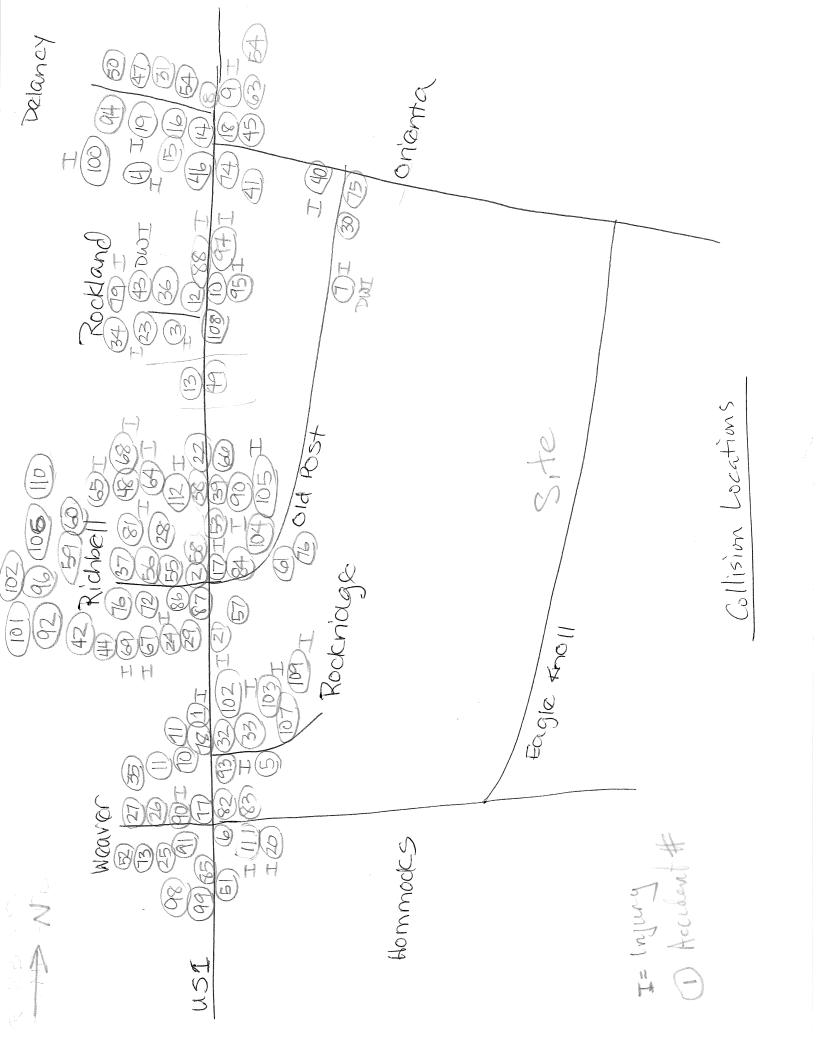
Description

Crash History

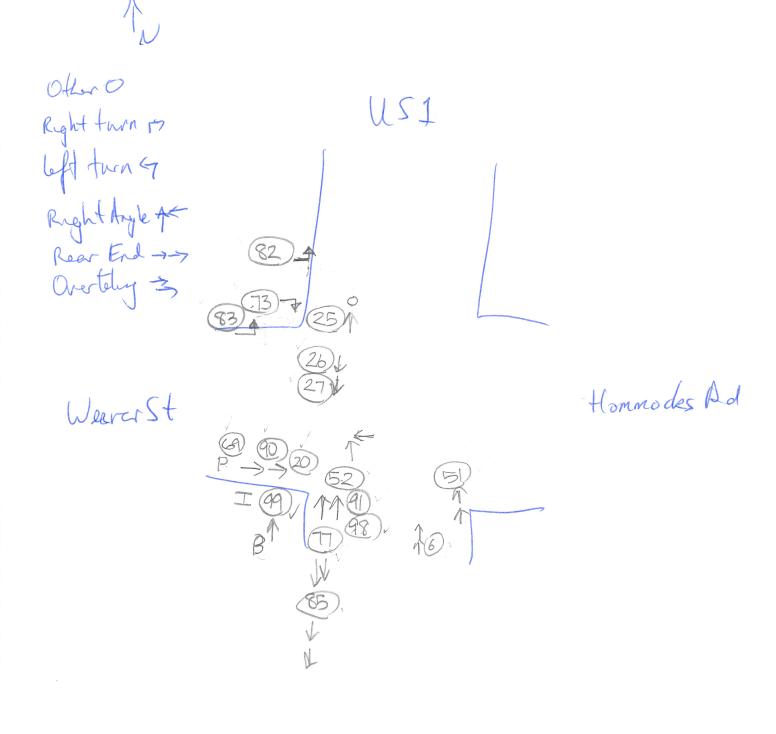
	0		ccident	Accident Severity					Manr	Manner of Collision	ollision			
Intersection	Total Accidents	Fatal	Ē	PDO	IOd	Rear End	Rt Angle	Left turn	Right turn	Over- taking	Head- on	Ped	Bike	Other
Boston Post Road & Hommocks Rd/Weaver St	27		10	15	2	∞	ъ	m	-	4			2	4
Boston Post Road & Old Boston Rd/Richbell Rd	40		21	24	2	3	6	9	1	9		9	2	7
Boston Post Road & Orienta Ave/Delancy Ave	35		15	22	9	15	9	1		6		2	1	1
Hommocks Rd & Eagle Knolls Rd	1		1											1
Orienta Ave & Fairway Lane	1						1							
Orienta Ave & East Cove Rd	3		1	2	1	1	1				1			
Old Boston Post Rd & Cooper Ave	0													

Crash Summary (2013 through 2015)

Crash Summary (2013 through 2015)	rough 2015)																						
											Apparent	Apparent Contributing Factors	Factors										
Intersection	Total Passing or Accidents Improper Lane Usage	Turning Improperly	Failure D to Yield In R.O.W.	Driver L Inattn Cr	Unsafe Follow Lane Too Change Closely	ollow Ea Too Dr. osely Dr.	Eating/ Dis Distriction Distriction Co	Disregard P Traffic SI Control	Pavemt Slippery V	Other Vehicular	Backing Unsafely	View Obstructed /Limited	Passing 4 Too 0 Closely 0	Alcohol P or Drugs [Alcohol Passenger Unsafe or Drugs Distractn Speed	Unsafe Speed	Driver Ped /Bik inexperien Error ce confusior	a –	Fell Fatigued/ asleep drowsy	atigued/ drowsy	Fatigued/ Driverless/ drowsy veh	Reaction to Uninvolv- ed Vehicle	No Factor Given
Boston Post Road & Hommocks Rd/Weaver St	27	1	m	∞		1		1	T T			1		1				2			1		12
Boston Post Road & Old Boston Rd/Richbell Rd	40	1	7	16	7	1		4		1	4	4		2			7	1				1	4
Boston Post Road & Orienta Ave/Delancy Ave	35	2	7	6	5	9			2		2	1		1		1			1	1		2	4
Hommocks Rd & Eagle Knolls Rd	1			1																			
Orienta Ave & Fairway Lane	1		1																				
Orienta Ave & East Cove Rd	3			1							1	1											
Old Boston Post Rd & Cooper Ave	0																						

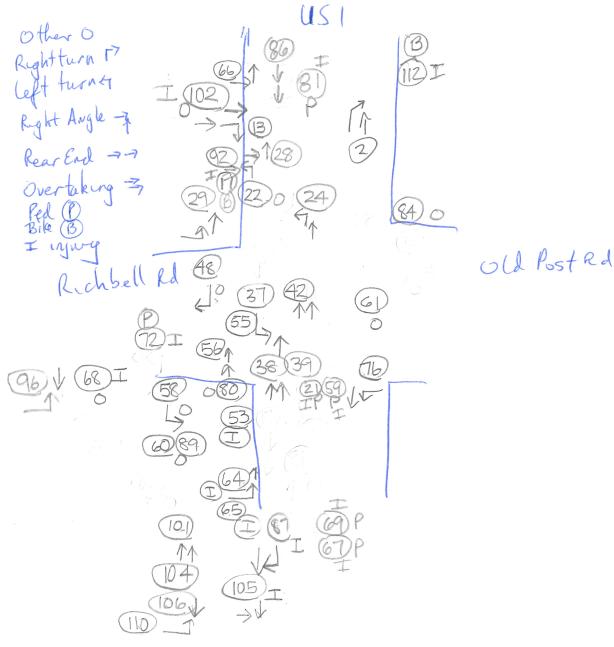


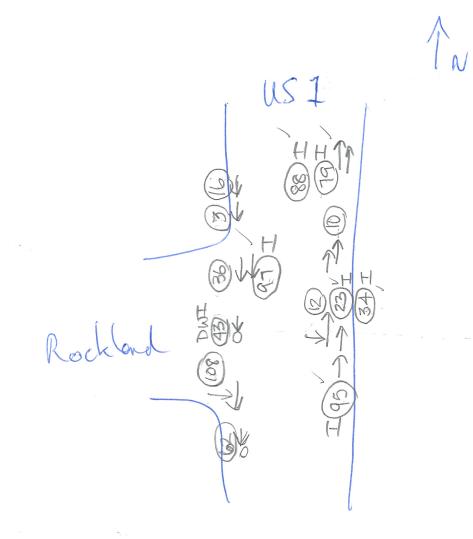
\$/14/17 Collision diagram Hommocks Rd, Weiner St, XUS 1



US 1 BH 0 1 Ē PRE elge Roch H 123355 13375 (20) (00) (II)

Richbell Rd, Old PostRd and USI TN





8/4/17 Colliscon Diagram Belancey St, Orienta Ano, US I F 4 LET other O Right turn P left turn & 8 18 Right Augle - 24 T [100] Rear End -7-> overtaking -63) M I INJURY 31 0-1(9) 54 15 50 $f \in \mathcal{E}$ 74 Ľ. SN



Appendix

Description

Synchro – Level of Service Analysis Worksheets

Existing
6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

	۶	→	\mathbf{r}	4	←	*	1	Ť	1	1	ţ	∢
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	f,		5	4Î		7	≜ †}		۲	≜ ¶≽	
Traffic Volume (vph)	72	150	51	55	143	33	83	572	134	175	493	64
Future Volume (vph)	72	150	51	55	143	33	83	572	134	175	493	64
Satd. Flow (prot)	1687	1682	0	1652	1580	0	1602	3244	0	1604	3225	0
Flt Permitted	0.443		-	0.388		-	0.188		-	0.098		-
Satd. Flow (perm)	685	1682	0	648	1580	0	313	3244	0	165	3225	0
Satd. Flow (RTOR)	000	1002	0	0.0		0	0.0	0211	0		0220	Ū
Confl. Peds. (#/hr)	150		50	50		150	32		13	13		32
Confl. Bikes (#/hr)							01		2			2
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	7%	3%	2%	2%	4%	6%	10%	5%	3%	5%	6%	2%
Shared Lane Traffic (%)	170	070	270	270	170	070	1070	070	070	070	070	270
Lane Group Flow (vph)	96	268	0	73	235	0	111	942	0	233	742	0
Turn Type	Perm	NA	U	Perm	NA	0	pm+pt	NA	U	pm+pt	NA	U
Protected Phases	I CIIII	12		I CIIII	16		5	2		1	6	
Permitted Phases	12	12		16	10		2	Z		6	0	
Total Split (s)	30.0	30.0		30.0	30.0		16.0	42.0		23.0	49.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	32.3	32.3		32.3	32.3		46.7	37.0		58.7	44.0	
Actuated g/C Ratio	0.26	0.26		0.26	0.26		0.37	0.30		0.47	0.35	
v/c Ratio	0.20	0.20		0.20	0.20		0.57	0.30		0.47	0.35	
Control Delay	58.0	51.6		0.44 54.1	50.6		39.7	68.7		75.5	37.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	58.0	51.6		54.1	50.6		39.7	68.7		75.5	37.4	
LOS	58.0 E	51.0 D			50.0 D		39.7 D			75.5 E	57.4 D	
	E	53.3		D	51.4		D	E 65.6		E	46.5	
Approach Delay		53.3 D										
Approach LOS	70			Γ4	D		40	E 398		100	D	
Queue Length 50th (ft)	73	208		54	180		49			135	264	
Queue Length 95th (ft)	112	248		87	220		69	379		176	262	_
Internal Link Dist (ft)	145	190		150	209		100	263		140	1683	
Turn Bay Length (ft)	145	40.4		150	100		180	0/0		140	1105	
Base Capacity (vph)	177	434		167	408		233	960		286	1135	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	_
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	_
Reduced v/c Ratio	0.54	0.62		0.44	0.58		0.48	0.98		0.81	0.65	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 125												
Offset: 0 (0%), Referenced	to phase 2:	NBTL, Sta	art of Gre	en								
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.98												
Intersection Signal Delay: 5	5.4				tersectior							
Intersection Capacity Utiliza	tion 62.0%			IC	CU Level o	of Service	e B					
Analysis Period (min) 15												

Analysis Period (min) 15

Splits and Phases: 6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

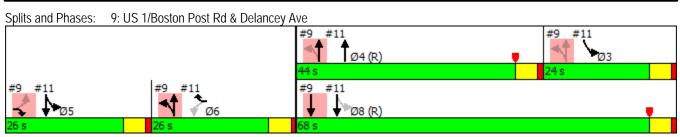
Ø2 (R)	Ø1		<u>⊿</u> _{Ø12}
42 s	23 s	30 s	30 s
	▲ Ø5		₩Ø16
49 s	16 s		30 s

Lane Group	Ø9		
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Satd. Flow (RTOR)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	9		
Permitted Phases	,		
Total Split (s)	30.0		
Total Lost Time (s)	0010		
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (ft)			
Queue Length 95th (ft)			
Internal Link Dist (ft)			
Turn Bay Length (ft)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

Lane Group EBL EBR NBL NBT SBT SBR Ø3 Ø4 Lane Configurations	
Traffic Volume (vph) 27 165 80 757 860 13 Future Volume (vph) 27 165 80 757 860 13 Satd. Flow (prot) 1736 1599 0 3487 3027 0 Flt Permitted 0.950 0.840	
Traffic Volume (vph) 27 165 80 757 860 13 Future Volume (vph) 27 165 80 757 860 13 Satd. Flow (prot) 1736 1599 0 3487 3027 0 Flt Permitted 0.950 0.840 0.840 0 0	
Satd. Flow (prot) 1736 1599 0 3487 3027 0 Flt Permitted 0.950 0.840 0 <td< td=""><td></td></td<>	
Flt Permitted 0.950 0.840	
Satd. Flow (perm) 1733 1599 0 2942 3027 0	
Satd. Flow (RTOR) 185	
Confl. Peds. (#/hr) 1 11 11	
Confl. Bikes (#/hr) 5	
Peak Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89	
Heavy Vehicles (%) 4% 1% 3% 3% 4% 8%	
Parking (#/hr) 5	
Shared Lane Traffic (%)	
Lane Group Flow (vph) 30 185 0 941 981 0	
Turn Type Perm Prot custom NA NA	
Protected Phases 5 6 6 4 8 3 4	
Permitted Phases 5 3 4 3	
Total Split (s) 26.0 26.0 26.0 68.0 24.0 44.0	
Total Lost Time (s) 5.0 5.0 5.0	
Act Effct Green (s) 17.5 17.5 87.5 63.0	
Actuated g/C Ratio 0.15 0.15 0.73 0.52	
v/c Ratio 0.12 0.47 0.38 0.62	
Control Delay 43.9 10.4 1.3 22.2	
Queue Delay 0.0 0.1 0.4 0.7	
Total Delay 43.9 10.5 1.6 22.8	
LOS D B A C	
Approach Delay 15.1 1.6 22.8	
Approach LOS B A C	
Queue Length 50th (ft) 20 0 21 271	
Queue Length 95th (ft) 47 61 18 333	
Internal Link Dist (ft) 246 90 543	
Turn Bay Length (ft) 70	
Base Capacity (vph) 303 432 2456 1589	
Starvation Cap Reductn 0 0 881 213	
Spillback Cap Reductn 0 16 0 283	
Storage Cap Reductn 0 0 0 0	
Reduced v/c Ratio 0.10 0.44 0.60 0.75	
Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 120	
Offset: 69 (58%), Referenced to phase 4:NBTL and 8:SBT, Start of Yellow	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.76	
Intersection Signal Delay: 12.7 Intersection LOS: B	
Intersection Capacity Utilization 63.3% ICU Level of Service B	
Analysis Period (min) 15	

Existing 9: US 1/Boston Post Rd & Delancey Ave

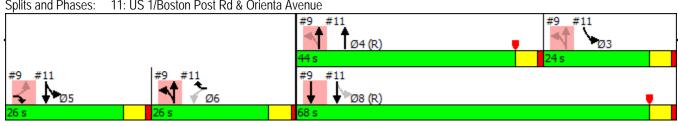
Splits and Phases: 9: US 1/Boston Post Rd & Delancey Ave



Existing 11: US 1/Boston Post Rd & Orienta Avenue

	4	•	Ť	1	1	ţ				
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	۲	1	∱ ⊅		۲	↑				
Traffic Volume (vph)	78	217	620	121	358	667				
Future Volume (vph)	78	217	620	121	358	667				
Satd. Flow (prot)	1601	1501	3387	0	1648	1757				
Flt Permitted	0.950				0.176					
Satd. Flow (perm)	1590	1501	3387	0	305	1757				
Satd. Flow (RTOR)		244								
Confl. Peds. (#/hr)	4			8	8					
Confl. Bikes (#/hr)				1						
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89				
Heavy Vehicles (%)	9%	4%	3%	11%	9%	4%				
Shared Lane Traffic (%)										
Lane Group Flow (vph)	88	244	833	0	402	749				
Turn Type	Perm	Prot	NA		custom	NA				
Protected Phases		6	4		35	58	3	5	8	
Permitted Phases	6	-			8		-	-	-	
Total Split (s)	26.0	26.0	44.0		U		24.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				2110	2010	0010	
Act Effct Green (s)	24.5	24.5	39.0		85.5	85.5				
Actuated g/C Ratio	0.20	0.20	0.32		0.71	0.71				
v/c Ratio	0.27	0.49	0.76		0.59	0.60				
Control Delay	44.5	9.0	41.6		21.7	4.3				
Queue Delay	0.0	0.0	0.0		16.0	0.4				
Total Delay	44.5	9.0	41.6		37.8	4.7				
LOS	D	A	D		D	A				
Approach Delay	18.4		41.6			16.3				
Approach LOS	В		D			В				
Queue Length 50th (ft)	58	0	303		202	55				
Queue Length 95th (ft)	110	70	374		298	53				
Internal Link Dist (ft)	450		2270		270	90				
Turn Bay Length (ft)										
Base Capacity (vph)	324	500	1100		729	1303				
Starvation Cap Reductn	0	0	0		312	180				
Spillback Cap Reductn	0	0	0		0	0				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.27	0.49	0.76		0.96	0.67				
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Reference	d to phase	4:NBTL #	and 8:SB	L. Start o	f Yellow					
Control Type: Actuated-Coo				., otari 0	. 10101					
Maximum v/c Ratio: 0.76										
Intersection Signal Delay: 25	5 7			Ir	ntersectior	10S C				
Intersection Capacity Utilization					CU Level o		B			
Analysis Period (min) 15	1011-07.070						5			

Splits and Phases: 11: US 1/Boston Post Rd & Orienta Avenue



Existing
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

	۶	-	\mathbf{F}	∢	-	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ		1	٦.	eî 👘		ሻ	††			A	
Traffic Volume (vph)	95	0	92	85	64	67	95	798	0	0	659	64
Future Volume (vph)	95	0	92	85	64	67	95	798	0	0	659	64
Satd. Flow (prot)	1736	0	1568	1678	1680	0	1646	3271	0	0	3289	0
Flt Permitted	0.604			0.950			0.175					
Satd. Flow (perm)	1092	0	1525	1651	1680	0	299	3271	0	0	3289	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	10		11	11		10	60					60
Confl. Bikes (#/hr)												4
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	4%	2%	3%	4%	3%	3%	6%	3%	2%	2%	4%	2%
Shared Lane Traffic (%)									_			
Lane Group Flow (vph)	112	0	108	100	154	0	112	939	0	0	850	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		13.0	67.0			54.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	16.7		16.7	16.7	16.7		42.2	42.2			32.1	
Actuated g/C Ratio	0.20		0.20	0.20	0.20		0.50	0.50			0.38	
v/c Ratio	0.52		0.36	0.31	0.46		0.37	0.57			0.68	_
Control Delay	48.1		41.0	39.7	42.7		18.8	18.8			28.6	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	_
Total Delay	48.1		41.0	39.7	42.7		18.8	18.8			28.6	
LOS	D		D	D	D		В	В			С	_
Approach Delay					41.5			18.8			28.6	
Approach LOS	. –				D			В			С	_
Queue Length 50th (ft)	67		62	57	91		40	228			254	
Queue Length 95th (ft)	132	100	121	113	164		78	312			338	_
Internal Link Dist (ft)		483	4.40	400	489		475	1683			2270	
Turn Bay Length (ft)	100		140	100			175	0.110			01.10	_
Base Capacity (vph)	430		601	651	662		313	2413			2140	
Starvation Cap Reductn	0		0	0	0		0	0			0	_
Spillback Cap Reductn	0		0	0	0		0	0			0	
Storage Cap Reductn	0		0	0	0		0	0			0	_
Reduced v/c Ratio	0.26		0.18	0.15	0.23		0.36	0.39			0.40	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 83.9	9											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.68												
Intersection Signal Delay: 2					tersection							
Intersection Capacity Utiliza	ition 55.8%			IC	U Level o	of Service	e B					
Analysis Period (min) 15												

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

[™] ¶ _{Ø2}	-√ Ø4	∦1 _{Ø9}
67 s	31 s	27 s
▲ Ø5 ↓ Ø6	₩Ø8	
13 s 54 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Interestion Cummon		
Intersection Summary		

	٦	\mathbf{i}	•	Ť	ţ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			र्भ	eî.	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	17	12	26	101	165	64
Future Volume (vph)	17	12	26	101	165	64
Peak Hour Factor	0.68	0.68	0.68	0.68	0.68	0.68
Hourly flow rate (vph)	25	18	38	149	243	94
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	43	187	337			
Volume Left (vph)	25	38	0			
Volume Right (vph)	18	0	94			
Hadj (s)	-0.12	0.15	-0.04			
Departure Headway (s)	4.9	4.5	4.2			
Degree Utilization, x	0.06	0.23	0.39			
Capacity (veh/h)	658	778	841			
Control Delay (s)	8.2	8.9	9.8			
Approach Delay (s)	8.2	8.9	9.8			
Approach LOS	А	А	А			
Intersection Summary						
Delay			9.4			
Level of Service			А			
Intersection Capacity Utiliz	ation		33.0%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		el el			ŧ	
Sign Control	Stop		Stop			Stop	
Traffic Volume (vph)	2	77	12	0	44	12	
Future Volume (vph)	2	77	12	0	44	12	
Peak Hour Factor	0.43	0.43	0.43	0.43	0.43	0.43	
Hourly flow rate (vph)	5	179	28	0	102	28	
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total (vph)	184	28	130				
Volume Left (vph)	5	0	102				
Volume Right (vph)	179	0	0				
Hadj (s)	-0.56	0.02	0.17				
Departure Headway (s)	3.7	4.4	4.5				
Degree Utilization, x	0.19	0.03	0.16				
Capacity (veh/h)	933	766	764				
Control Delay (s)	7.6	7.6	8.3				
Approach Delay (s)	7.6	7.6	8.3				
Approach LOS	А	А	А				
Intersection Summary							
Delay			7.9				
Level of Service			А				
Intersection Capacity Utiliza	ation		23.0%	IC	U Level o	f Service	А
Analysis Period (min)			15				

	4	•	Ť	1	1	ţ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	۲					ન		
Traffic Volume (veh/h)	6	0	0	0	0	111		
Future Volume (Veh/h)	6	0	0	0	0	111		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71		
Hourly flow rate (vph)	8	0	0	0	0	156		
Pedestrians	3		1					
Lane Width (ft)	12.0		0.0					
Walking Speed (ft/s)	4.0		4.0					
Percent Blockage	0		0					
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	160	3			3			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	160	3			3			
tC, single (s)	6.6	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.7	3.3			2.2			
p0 queue free %	99	100			100			
cM capacity (veh/h)	795	1078			1622			
Direction, Lane #	WB 1	SB 1						
Volume Total	8	156						
Volume Left	8	0						
Volume Right	0	0						
cSH	795	1622						
Volume to Capacity	0.01	0.00						
Queue Length 95th (ft)	1	0						
Control Delay (s)	9.6	0.0						
Lane LOS	А							
Approach Delay (s)	9.6	0.0						
Approach LOS	А							
Intersection Summary								
Average Delay			0.5					
Intersection Capacity Utiliz	ation		15.8%	IC	U Level o	of Service	е	
Analysis Period (min)			15	.0	5.610		-	
			15					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	ef 🕺	
Traffic Volume (veh/h)	5	1	1	117	228	2
Future Volume (Veh/h)	5	1	1	117	228	2
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	6	1	1	139	271	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	413	272	273			
vC1, stage 1 conf vol	110	272	210			
vC2, stage 2 conf vol						
vCu, unblocked vol	413	272	273			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	1.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	597	769	1296			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	7	140	273			
Volume Left	6	1	0			
Volume Right	1	0	2			
cSH	616	1296	1700			
Volume to Capacity	0.01	0.00	0.16			
Queue Length 95th (ft)	1	0	0			
Control Delay (s)	10.9	0.1	0.0			
Lane LOS	В	А				
Approach Delay (s)	10.9	0.1	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliza	ation		22.1%	IC	CU Level o	of Service
Analysis Period (min)			15	IC		
Andiysis Pendu (IIIII)			10			

No-Build
6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	et 🗧		٦	ef 👘		۲.	A		۲	A⊅	
Traffic Volume (vph)	73	152	52	56	145	33	84	584	136	177	507	65
Future Volume (vph)	73	152	52	56	145	33	84	584	136	177	507	65
Satd. Flow (prot)	1687	1682	0	1652	1580	0	1602	3247	0	1604	3225	0
Flt Permitted	0.439			0.380			0.176			0.098		
Satd. Flow (perm)	679	1682	0	635	1580	0	293	3247	0	165	3225	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	150		50	50		150	32		13	13		32
Confl. Bikes (#/hr)									2			2
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	7%	3%	2%	2%	4%	6%	10%	5%	3%	5%	6%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	97	272	0	75	237	0	112	960	0	236	763	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		16.0	42.0		23.0	49.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	32.2	32.2		32.2	32.2		46.8	37.0		58.8	44.0	
Actuated g/C Ratio	0.26	0.26		0.26	0.26		0.37	0.30		0.47	0.35	
v/c Ratio	0.56	0.63		0.46	0.58		0.53	1.00		0.87	0.67	
Control Delay	59.1	52.1		55.3	50.9		41.7	72.9		76.2	38.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	59.1	52.1		55.3	50.9		41.7	72.9		76.2	38.0	
LOS	E	D		E	D		D	E		E	D	
Approach Delay		53.9			52.0			69.6			47.0	
Approach LOS		D			D			E			D	
Queue Length 50th (ft)	74	212		56	182		49	408		138	274	
Queue Length 95th (ft)	#115	252		90	222		70	388		179	270	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	174	433		163	407		227	961		286	1135	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.56	0.63		0.46	0.58		0.49	1.00		0.83	0.67	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 125	5											
Offset: 0 (0%), Referenced		NBTL, St	art of Gre	en								
Control Type: Actuated-Cod												
Maximum v/c Ratio: 1.00												
Intersection Signal Delay: 5	57.3			In	ntersectior	LOS: E						
Intersection Capacity Utiliza					CU Level o		e B					
Analysis Period (min) 15												
# 95th percentile volume	exceeds ca	pacity, qu	ieue may	be longe	r.							
Ouquo shown is maximi				5								

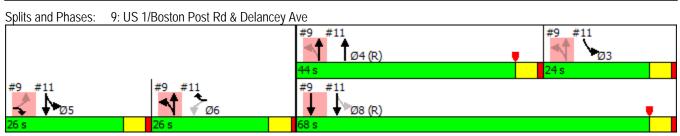
95th percentile volume exceeds capacity, queue may be longer Queue shown is maximum after two cycles.

Ø2 (R)	Ø1	∦1 ø9	<u>⊿</u> _{Ø12}
42 s	23 s	30 s	30 s
↓ Ø6	▲ Ø5		₩ Ø16
49 s	16 s		30 s

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

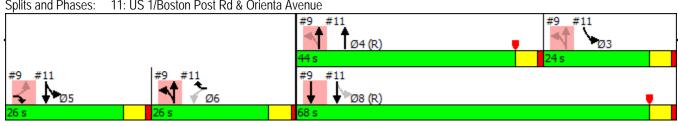
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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4		
Lane Configurations	۲	1			≜ ⊅					
Traffic Volume (vph)	27	168	81	772	877	14				
Future Volume (vph)	27	168	81	772	877	14				
Satd. Flow (prot)	1736	1599	0	3487	3027	0				
Flt Permitted	0.950			0.835						
Satd. Flow (perm)	1733	1599	0	2924	3027	0				
Satd. Flow (RTOR)		189								
Confl. Peds. (#/hr)	1		11			11				
Confl. Bikes (#/hr)						5				
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89				
Heavy Vehicles (%)	4%	1%	3%	3%	4%	8%				
Parking (#/hr)					5					
Shared Lane Traffic (%)										
Lane Group Flow (vph)	30	189	0	958	1001	0				
Turn Type	Perm		custom	NA	NA					
Protected Phases		5	6	64	8		3	4		
Permitted Phases	5		34	3						
Total Split (s)	26.0	26.0	26.0		68.0		24.0	44.0		
Total Lost Time (s)	5.0	5.0			5.0					
Act Effct Green (s)	17.8	17.8		87.2	63.0					
Actuated g/C Ratio	0.15	0.15		0.73	0.52					
v/c Ratio	0.12	0.48		0.39	0.63					
Control Delay	43.6	10.3		1.3	22.5					
Queue Delay	0.0	0.1		0.4	0.8					
Total Delay	43.6	10.4		1.7	23.3					
LOS	D	В		А	С					
Approach Delay	15.0			1.7	23.3					
Approach LOS	В			А	С					
Queue Length 50th (ft)	20	0		22	278					
Queue Length 95th (ft)	47	62		18	343					
Internal Link Dist (ft)	246			90	543					
Turn Bay Length (ft)		70								
Base Capacity (vph)	303	435		2445	1589					
Starvation Cap Reductn	0	0		860	211					
Spillback Cap Reductn	0	17		0	295					
Storage Cap Reductn	0	0		0	0					
Reduced v/c Ratio	0.10	0.45		0.60	0.77					
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120)									
Offset: 69 (58%), Reference		4:NBTL	and 8:SB	T, Start o	f Yellow					
Control Type: Actuated-Coc										
Maximum v/c Ratio: 0.77										
Intersection Signal Delay: 1	2.9			In	itersectior	n LOS: B				
Intersection Capacity Utiliza					CU Level o		С			
Analysis Period (min) 15										

No-Build 9: US 1/Boston Post Rd & Delancey Ave



No-Build 11: US 1/Boston Post Rd & Orienta Avenue

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	٦	1	∱ ⊅		٦	†				
Traffic Volume (vph)	79	220	633	123	362	682				
Future Volume (vph)	79	220	633	123	362	682				
Satd. Flow (prot)	1601	1501	3388	0	1648	1757				
Flt Permitted	0.950				0.168					
Satd. Flow (perm)	1590	1501	3388	0	291	1757				
Satd. Flow (RTOR)		247								
Confl. Peds. (#/hr)	4			8	8					
Confl. Bikes (#/hr)				1						
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89				
Heavy Vehicles (%)	9%	4%	3%	11%	9%	4%				
Shared Lane Traffic (%)	770	170	0.10	1170	,,,,	170				
Lane Group Flow (vph)	89	247	849	0	407	766				
Turn Type	Perm	Prot	NA	U	custom	NA				
Protected Phases	1 0111	6	4		3 5	58	3	5	8	
Permitted Phases	6	U	т		8	5.0	5	5	0	
Total Split (s)	26.0	26.0	44.0		0		24.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				24.0	20.0	00.0	
Act Effct Green (s)	24.2	24.2	39.0		85.8	85.8				
Actuated g/C Ratio	0.20	0.20	0.32		0.72	0.72				
v/c Ratio	0.20	0.20	0.32		0.72	0.72				
Control Delay	44.8	9.0	42.1		22.4	4.5				
Queue Delay	0.0	9.0 0.0	42.1		26.1	4.5 0.5				
Total Delay	44.8	9.0	42.1		48.4	4.9				
LOS	44.8 D	9.0 A	42.1 D		40.4 D	4.9 A				
Approach Delay	18.5	A	42.1		U	20.0				
	16.5 B		42.1 D			20.0 C				
Approach LOS	в 60	0	310		213	59				
Queue Length 50th (ft)		0				59				
Queue Length 95th (ft)	111	70	383		310					
Internal Link Dist (ft)	450		2270			90				
Turn Bay Length (ft)	220	500	1101		704	1000				
Base Capacity (vph)	320	500	1101		724	1303				
Starvation Cap Reductn	0	0	0		321	185				
Spillback Cap Reductn	0	0	0		0	0				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.28	0.49	0.77		1.01	0.69				
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Referenced		4:NBTL a	and 8:SB	F, Start o	f Yellow					
Control Type: Actuated-Coor	dinated									
Maximum v/c Ratio: 0.77										
Intersection Signal Delay: 27					tersectior					
Intersection Capacity Utilizat	ion 58.5%			IC	CU Level o	of Service	В			
Analysis Period (min) 15										



No-Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1		1	ľ	et.		ľ	<u></u>			∱ ₽	
Traffic Volume (vph)	96	0	93	86	65	68	96	813	0	0	675	65
Future Volume (vph)	96	0	93	86	65	68	96	813	0	0	675	65
Satd. Flow (prot)	1736	0	1568	1678	1680	0	1646	3271	0	0	3289	0
Flt Permitted	0.596			0.950			0.171					
Satd. Flow (perm)	1077	0	1525	1651	1680	0	292	3271	0	0	3289	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	10		11	11		10	60					60
Confl. Bikes (#/hr)												4
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	4%	2%	3%	4%	3%	3%	6%	3%	2%	2%	4%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	113	0	109	101	156	0	113	956	0	0	870	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		13.0	67.0			54.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	16.9		16.9	16.9	16.9		43.1	43.1			33.2	
Actuated g/C Ratio	0.20		0.20	0.20	0.20		0.51	0.51			0.39	
v/c Ratio	0.53		0.36	0.31	0.47		0.38	0.58			0.68	
Control Delay	49.3		41.5	40.2	43.3		18.9	18.8			28.6	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	49.3		41.5	40.2	43.3		18.9	18.8			28.6	
LOS	D		D	D	D		В	В			С	_
Approach Delay					42.1			18.8			28.6	
Approach LOS	(0			50	D		10	В			С	
Queue Length 50th (ft)	68		64	58	93		40	236			263	
Queue Length 95th (ft)	135	100	124	115	168		78	320			347	
Internal Link Dist (ft)		483	140	100	489		175	1683			2270	
Turn Bay Length (ft)	410		140	100	/ [1		175	2200			0110	
Base Capacity (vph)	418		591	640	651		309	2399			2112	
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn Storage Cap Reductn	v		0	0	0		0	Ũ			0	
Reduced v/c Ratio	0 0.27		0.18	0.16	0.24		0 0.37	0 0.40			0.41	
	0.27		0.10	0.10	0.24		0.37	0.40			0.41	
Intersection Summary												
Cycle Length: 125 Actuated Cycle Length: 85.	1											
Control Type: Semi Act-Un												
Maximum v/c Ratio: 0.68	COOL											
Intersection Signal Delay: 2	2 2			In	tersectior							
Intersection Signal Delay. 2					CU Level C		B B					
Analysis Doriod (min) 15	auon 30.3 <i>%</i>			IC.								

Analysis Period (min) 15

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

[™] ¶ _{Ø2}	<₽ Ø4	∦1 ø9
67 s	31 s	27 s
▲ Ø5 ↓ Ø6	₩ Ø8	
13 s 54 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		
intersection summary		

	٦	\mathbf{i}	•	Ť	ţ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	et 🗧	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	17	12	26	102	167	65
Future Volume (vph)	17	12	26	102	167	65
Peak Hour Factor	0.68	0.68	0.68	0.68	0.68	0.68
Hourly flow rate (vph)	25	18	38	150	246	96
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	43	188	342			
Volume Left (vph)	25	38	0			
Volume Right (vph)	18	0	96			
Hadj (s)	-0.12	0.15	-0.04			
Departure Headway (s)	4.9	4.5	4.2			
Degree Utilization, x	0.06	0.24	0.40			
Capacity (veh/h)	656	777	841			
Control Delay (s)	8.2	8.9	9.9			
Approach Delay (s)	8.2	8.9	9.9			
Approach LOS	А	А	А			
Intersection Summary						
Delay			9.4			
Level of Service			А			
Intersection Capacity Utiliza	ation		33.2%	IC	CU Level o	of Service
Analysis Period (min)			15			

	4	•	1	~	1	Ŧ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		eî.			र्स
Sign Control	Stop		Stop			Stop
Traffic Volume (vph)	2	78	12	0	45	12
Future Volume (vph)	2	78	12	0	45	12
Peak Hour Factor	0.43	0.43	0.43	0.43	0.43	0.43
Hourly flow rate (vph)	5	181	28	0	105	28
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total (vph)	186	28	133			
Volume Left (vph)	5	0	105			
Volume Right (vph)	181	0	0			
Hadj (s)	-0.56	0.02	0.17			
Departure Headway (s)	3.7	4.4	4.5			
Degree Utilization, x	0.19	0.03	0.17			
Capacity (veh/h)	930	764	763			
Control Delay (s)	7.6	7.6	8.4			
Approach Delay (s)	7.6	7.6	8.4			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.9			
Level of Service			А			
Intersection Capacity Utiliz	zation		23.1%	IC	U Level c	of Service
Analysis Period (min)			15			

	4	•	Ť	1	1	Ļ	-	
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	۲					र्स		
Traffic Volume (veh/h)	6	0	0	0	0	112		
Future Volume (Veh/h)	6	0	0	0	0	112		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71		
Hourly flow rate (vph)	8	0	0	0	0	158		
Pedestrians	3		1					
Lane Width (ft)	12.0		0.0					
Walking Speed (ft/s)	4.0		4.0					
Percent Blockage	0		0					
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	162	3			3			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	162	3			3			
tC, single (s)	6.6	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.7	3.3			2.2			
p0 queue free %	99	100			100			
cM capacity (veh/h)	793	1078			1622			
Direction, Lane #	WB 1	SB 1						
Volume Total	8	158						
Volume Left	8	0						
Volume Right	0	0						
cSH	793	1622						
Volume to Capacity	0.01	0.00						
Queue Length 95th (ft)	1	0						
Control Delay (s)	9.6	0.0						
Lane LOS	А							
Approach Delay (s)	9.6	0.0						
Approach LOS	А							
Intersection Summary								
Average Delay			0.5					
Intersection Capacity Utiliz	zation		15.9%	IC	U Level o	of Service	ڊ	
Analysis Period (min)			15					
			10					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî.	
Traffic Volume (veh/h)	5	1	1	118	231	2
Future Volume (Veh/h)	5	1	1	118	231	2
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	6	1	1	140	275	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	418	276	277			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	418	276	277			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	593	765	1292			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	7	141	277			
Volume Left	6	1	0			
Volume Right	1	0	2			
cSH	613	1292	1700			
Volume to Capacity	0.01	0.00	0.16			
Queue Length 95th (ft)	1	0	0			
Control Delay (s)	10.9	0.1	0.0			
Lane LOS	B	A	0.0			
Approach Delay (s)	10.9	0.1	0.0			
Approach LOS	B	0.1	0.0			
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilizat	tion		22.3%	10	CU Level d	of Sonvice
Analysis Period (min)	UUT			IC	O Level (I SEIVICE
Andiysis Peniou (min)			15			

Build <u>6: US 1/Boston Post Rd & Weaver Street/Hommocks Road</u>

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	el e		5	eî 🕺		7	↑ 1,		1	A1≱	
Traffic Volume (vph)	73	153	52	76	150	34	84	584	140	177	507	65
Future Volume (vph)	73	153	52	76	150	34	84	584	140	177	507	65
Satd. Flow (prot)	1687	1682	0	1652	1581	0	1602	3244	0	1604	3225	0
Flt Permitted	0.425			0.379			0.176			0.098		
Satd. Flow (perm)	660	1682	0	634	1581	0	293	3244	0	165	3225	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	150		50	50		150	32		13	13		32
Confl. Bikes (#/hr)									2			2
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	7%	3%	2%	2%	4%	6%	10%	5%	3%	5%	6%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	97	273	0	101	245	0	112	966	0	236	763	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		16.0	42.0		23.0	49.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	32.2	32.2		32.2	32.2		46.8	37.0		58.8	44.0	
Actuated g/C Ratio	0.26	0.26		0.26	0.26		0.37	0.30		0.47	0.35	
v/c Ratio	0.57	0.63		0.62	0.60		0.53	1.01		0.87	0.67	
Control Delay	60.3	52.2		64.1	51.6		41.7	74.5		76.2	38.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	60.3	52.2		64.1	51.6		41.7	74.5		76.2	38.0	
LOS	E	D		E	D		D	E		E	D	
Approach Delay		54.3			55.3			71.1			47.0	
Approach LOS		D			E			E			D	
Queue Length 50th (ft)	74	213		78	189		49	~416		138	274	
Queue Length 95th (ft)	#121	252		#134	229		70	390		179	270	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	170	433		163	407		227	960		286	1135	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.57	0.63		0.62	0.60		0.49	1.01		0.83	0.67	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 125												
Offset: 0 (0%), Referenced to	nhase 2·NRT	Start of	Green									
Control Type: Actuated-Coord			Orcen									
Maximum v/c Ratio: 1.01												
Intersection Signal Delay: 58.3	3			In	tersection	OS [,] F						
Intersection Capacity Utilization					U Level of							
Analysis Period (min) 15				10								
 Volume exceeds capacity, 	queue is the	pretically in	nfinite									
Queue shown is maximum			infinto.									
# 95th percentile volume ex			nav be lon	aer.								
Queue shown is maximum				<u>.</u>								
	5											

Ø2 (R)	Ø1	. 	 Ø12	
42 s	23 s	30 s	30 s	
₩ø6	▲ ø5		Ø16	
49 s	16 s		30 s	

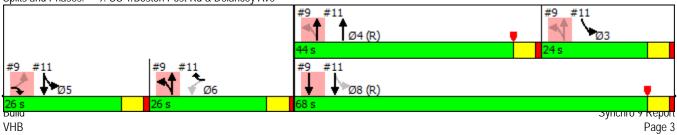
Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Build 9: US 1/Boston Post Rd & Delancey Ave

	≯	\mathbf{i}	1	1	۰.	∢			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
ane Configurations	5	1		4ħ	At≱				
affic Volume (vph)	27	169	86	787	880	14			
ture Volume (vph)	27	169	86	787	880	14			
td. Flow (prot)	1736	1599	0	3487	3027	0			
t Permitted	0.950			0.818					
td. Flow (perm)	1733	1599	0	2865	3027	0			
atd. Flow (RTOR)		190							
onfl. Peds. (#/hr)	1		11			11			
onfl. Bikes (#/hr)						5			
ak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89			
avy Vehicles (%)	4%	1%	3%	3%	4%	8%			
rking (#/hr)					5				
ared Lane Traffic (%)									
ne Group Flow (vph)	30	190	0	981	1005	0			
n Type	Perm	Prot	custom	NA	NA				
otected Phases		5	6	64	8		3	4	
mitted Phases	5		34	3					
al Split (s)	26.0	26.0	26.0		68.0		24.0	44.0	
al Lost Time (s)	5.0	5.0			5.0				
Effct Green (s)	17.8	17.8		87.2	63.0				
uated g/C Ratio	0.15	0.15		0.73	0.52				
Ratio	0.12	0.48		0.40	0.63				
ntrol Delay	43.6	10.3		1.4	22.5				
eue Delay	0.0	0.1		0.4	0.9				
tal Delay	43.6	10.4		1.8	23.4				
S	D	В		А	С				
proach Delay	14.9			1.8	23.4				
proach LOS	В			А	С				
eue Length 50th (ft)	20	0		24	281				
eue Length 95th (ft)	47	61		21	345				
ernal Link Dist (ft)	246	-		90	543				
rn Bay Length (ft)		70		a (
se Capacity (vph)	303	436		2435	1589				
arvation Cap Reductn	0	0		843	210				
illback Cap Reductn	0	18		0	298				
rage Cap Reductn	0	0		0	0				
duced v/c Ratio	0.10	0.45		0.62	0.78				
ersection Summary									
cle Length: 120									
ctuated Cycle Length: 120									
fset: 69 (58%), Referenced t		BTL and 8	3:SBT, Star	t of Yellow					
ntrol Type: Actuated-Coordi	nated								

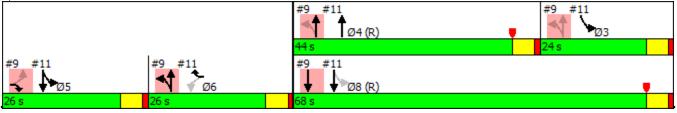
Maximum v/c Ratio: 0.77 Intersection Signal Delay: 12.9

Intersection Signal Delay. 12.7 Intersection Capacity Utilization 64.9% Analysis Period (min) 15 Intersection LOS: B ICU Level of Service C



Build 11: US 1/Boston Post Rd & Orienta Avenue

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	ኘ	1	A		٦	^				
Traffic Volume (vph)	79	239	634	123	367	682				
Future Volume (vph)	79	239	634	123	367	682				
Satd. Flow (prot)	1601	1501	3388	0	1648	1757				
Flt Permitted	0.950				0.168					
Satd. Flow (perm)	1590	1501	3388	0	291	1757				
Satd. Flow (RTOR)		269								
Confl. Peds. (#/hr)	4			8	8					
Confl. Bikes (#/hr)				1						
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89				
Heavy Vehicles (%)	9%	4%	3%	11%	9%	4%				
Shared Lane Traffic (%)										
Lane Group Flow (vph)	89	269	850	0	412	766				
Turn Type	Perm	Prot	NA	5	custom	NA				
Protected Phases		6	4		3 5	5 8	3	5	8	
Permitted Phases	6	Ŭ			8	5.0	J	Ŭ	Ŭ	
Total Split (s)	26.0	26.0	44.0		Ū		24.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				2110	2010	0010	
Act Effct Green (s)	24.2	24.2	39.0		85.8	85.8				
Actuated g/C Ratio	0.20	0.20	0.32		0.72	0.72				
//c Ratio	0.28	0.52	0.77		0.61	0.61				
Control Delay	44.8	9.0	42.2		22.6	4.4				
Queue Delay	0.0	0.0	0.0		27.9	0.5				
Total Delay	44.8	9.0	42.2		50.5	4.9				
LOS	D	A	D		D	A				
Approach Delay	17.9	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	42.2		U	20.9				
Approach LOS	B		12.2 D			C				
Queue Length 50th (ft)	60	0	311		216	57				
Queue Length 95th (ft)	111	73	384		317	54				
Internal Link Dist (ft)	450	75	2270		517	90				
Turn Bay Length (ft)	400		2210			70				
Base Capacity (vph)	320	517	1101		724	1303				
Starvation Cap Reductn	0	0	0		319	185				
Spillback Cap Reductn	0	0	0		0	0				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.28	0.52	0.77		1.02	0.69				
	0.20	0.52	0.77		1.02	0.07				
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120	o phoos A.M.	O here IT(CDT Chart	of Valley						
Offset: 69 (58%), Referenced to Control Type: Actuated-Coordir	u phase 4:NE	si L and 8:	SRI' Start	U YEIIOW						
CONTROL LYDE: ACTUATED-COORDIN	nated									
Vaximum v/c Ratio: 0.77					torooction 1	00.0				
	F0 00/				tersection I U Level of					



Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1		1	1	el el		1	^			A1⊅	
Traffic Volume (vph)	97	0	93	93	67	84	96	813	0	0	675	65
Future Volume (vph)	97	0	93	93	67	84	96	813	0	0	675	65
Satd. Flow (prot)	1736	0	1568	1678	1667	0	1646	3271	0	0	3289	0
Flt Permitted	0.549			0.950			0.170					
Satd. Flow (perm)	993	0	1525	1651	1667	0	290	3271	0	0	3289	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	10		11	11		10	60					60
Confl. Bikes (#/hr)												4
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	4%	2%	3%	4%	3%	3%	6%	3%	2%	2%	4%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	114	0	109	109	178	0	113	956	0	0	870	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		13.0	67.0			54.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	17.9		17.9	17.9	17.9		43.6	43.6			33.6	
Actuated g/C Ratio	0.21		0.21	0.21	0.21		0.50	0.50			0.39	
v/c Ratio	0.56		0.35	0.32	0.52		0.39	0.58			0.68	
Control Delay	51.1		41.1	40.2	44.0		19.3	19.3			29.1	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	51.1		41.1	40.2	44.0		19.3	19.3			29.1	
LOS	D		D	D	D		В	B			С	
Approach Delay	2		2	2	42.6		2	19.3			29.1	
Approach LOS					D			В			С	
Queue Length 50th (ft)	70		64	64	109		42	243			269	
Queue Length 95th (ft)	138		124	123	191		78	320			347	
Internal Link Dist (ft)	100	483		.20	489			1683			2270	
Turn Bay Length (ft)		100	140	100	107		175	1000			2270	
Base Capacity (vph)	377		580	627	634		304	2383			2089	
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.30		0.19	0.17	0.28		0.37	0.40			0.42	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 86.5												
Control Type: Semi Act-Uncoc	ord											
Maximum v/c Ratio: 0.68												
Intersection Signal Delay: 28.0)			Int	tersection	LOS: C						
Intersection Capacity Utilizatio	n 57.7%			IC	U Level of	Service B						
Analysis Period (min) 15												
Splits and Phases: 20: US 1	/Boston Post	Rd & Rich	nbell Rd/O	ld Boston I	Post Rd							

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

▲ ¶ _{Ø2}	× Ø4	₩ 1 _{Ø9}
67 s	31 s	27 s
↑ Ø5 ↓ Ø6	₩ Ø8	
13 s 54 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ę	eî.	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	38	12	26	102	167	70
Future Volume (vph)	38	12	26	102	167	70
Peak Hour Factor	0.68	0.68	0.68	0.68	0.68	0.68
Hourly flow rate (vph)	56	18	38	150	246	103
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	74	188	349			
Volume Left (vph)	56	38	0			
Volume Right (vph)	18	0	103			
Hadj (s)	0.02	0.15	-0.05			
Departure Headway (s)	5.1	4.6	4.3			
Degree Utilization, x	0.11	0.24	0.41			
Capacity (veh/h)	637	753	821			
Control Delay (s)	8.7	9.1	10.2			
Approach Delay (s)	8.7	9.1	10.2			
Approach LOS	А	А	В			
Intersection Summary						
Delay			9.7			
Level of Service			А			
Intersection Capacity Utiliz	ation		33.5%	IC	CU Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et 🗧			ર્સ
Sign Control	Stop		Stop			Stop
Traffic Volume (vph)	2	107	12	0	50	12
Future Volume (vph)	2	107	12	0	50	12
Peak Hour Factor	0.43	0.43	0.43	0.43	0.43	0.43
Hourly flow rate (vph)	5	249	28	0	116	28
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total (vph)	254	28	144			
Volume Left (vph)	5	0	116			
Volume Right (vph)	249	0	0			
Hadj (s)	-0.57	0.02	0.18			
Departure Headway (s)	3.8	4.6	4.6			
Degree Utilization, x	0.26	0.04	0.19			
Capacity (veh/h)	924	728	732			
Control Delay (s)	8.1	7.8	8.7			
Approach Delay (s)	8.1	7.8	8.7			
Approach LOS	А	А	А			
Intersection Summary						
Delay			8.3			
Level of Service			А			
Intersection Capacity Utiliz	zation		24.9%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	٦					ર્સ		
Traffic Volume (veh/h)	32	0	0	0	4	112		
Future Volume (Veh/h)	32	0	0	0	4	112		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71		
Hourly flow rate (vph)	45	0	0	0	6	158		
Pedestrians	3		1					
Lane Width (ft)	12.0		0.0					
Walking Speed (ft/s)	4.0		4.0					
Percent Blockage	0		0					
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	174	3			3			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	174	3			3			
tC, single (s)	6.6	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.7	3.3			2.2			
p0 queue free %	94	100			100			
cM capacity (veh/h)	778	1078			1622			
Direction, Lane #	WB 1	SB 1						
Volume Total	45	164						
Volume Left	45	6						
Volume Right	0	0						
cSH	778	1622						
Volume to Capacity	0.06	0.00						
Queue Length 95th (ft)	5	0						
Control Delay (s)	9.9	0.3						
Lane LOS	А	А						
Approach Delay (s)	9.9	0.3						
Approach LOS	А							
Intersection Summary								
Average Delay			2.4					
Intersection Capacity Utiliz	ation		16.1%	IC	U Level o	of Service	Ś	
Analysis Period (min)			15					
			10					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्च	et 🗧	
Traffic Volume (veh/h)	5	1	1	139	236	2
Future Volume (Veh/h)	5	1	1	139	236	2
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	6	1	1	165	281	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	449	282	283			
vC1, stage 1 conf vol	117	202	200			
vC2, stage 2 conf vol						
vCu, unblocked vol	449	282	283			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	569	759	1285			
Direction, Lane #	EB 1	NB 1 166	SB 1 283			
	7	100				
Volume Left	6 1		0			
Volume Right		0 1285	2			
cSH Valuma ta Canaaitu	590		1700			
Volume to Capacity	0.01	0.00	0.17			
Queue Length 95th (ft)	1	0	0			
Control Delay (s)	11.2	0.1	0.0			
Lane LOS	В	A	0.0			
Approach Delay (s)	11.2	0.1	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliz	ation		22.5%	IC	CU Level o	of Service
Analysis Period (min)			15			
J						

Existing
6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	eî 👘		۳.	ef 👘		۳.	∱ }		۳.	≜ †}	
Traffic Volume (vph)	115	80	90	53	77	34	145	688	50	60	765	105
Future Volume (vph)	115	80	90	53	77	34	145	688	50	60	765	105
Satd. Flow (prot)	1770	1605	0	1652	1652	0	1745	3413	0	1668	3311	0
Flt Permitted	0.641			0.538			0.141			0.204		
Satd. Flow (perm)	1162	1605	0	925	1652	0	258	3413	0	357	3311	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	21		10	10		21	22		11	11		22
Confl. Bikes (#/hr)	0.00	0.00	3	0.00		0.00	0.00		0.00	0.00	0.00	3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	1%	6%	2%	1%	1%	1%	2%	2%	1%	3%	2%
Shared Lane Traffic (%)	100	100	0	50	104	0	4/4	000	0	(7	0/7	
Lane Group Flow (vph)	128	189	0	59	124	0	161	820	0	67	967	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	10	12		1/	16		5	2		1	6	_
Permitted Phases	12	20.0		16	20.0		2	(2.0		6	50.0	
Total Split (s)	30.0	30.0		30.0	30.0		18.0	63.0		13.0	58.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.8	39.8		39.8	39.8 0.29		68.2	59.1		59.2 0.44	53.0	
Actuated g/C Ratio	0.29 0.38	0.29 0.40		0.29 0.22	0.29		0.50	0.43 0.55		0.44	0.39 0.75	
Control Delay	48.4	47.1		46.9	44.4		0.67 53.1	30.7		25.8	40.2	
Queue Delay	40.4	47.1		40.9	44.4 0.0		0.0	0.0		25.8	40.2	
Total Delay	48.4	47.1		46.9	44.4		53.1	30.7		25.8	40.2	
LOS	40.4 D	47.1 D		40.9 D	D		55.1 D	50.7 C		23.0 C	40.2 D	
Approach Delay	D	47.6		D	45.2		D	34.3		U	39.3	
Approach LOS		47.0 D			ч <u></u> .2			54.5 С			57.5 D	
Queue Length 50th (ft)	103	153		45	96		75	284		30	384	
Queue Length 95th (ft)	178	242		93	162		115	349		54	467	
Internal Link Dist (ft)	170	190		70	209		110	263		01	1683	
Turn Bay Length (ft)	145	170		150	207		180	200		140	1000	
Base Capacity (vph)	339	469		270	483		276	1483		237	1290	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.38	0.40		0.22	0.26		0.58	0.55		0.28	0.75	
Intersection Summary												
Cycle Length: 136												
Actuated Cycle Length: 136												
Offset: 0 (0%), Referenced	to phase 2:	NBTL, Sta	art of Gre	en								
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 3					tersectior							
Intersection Capacity Utiliza	tion 63.0%			IC	CU Level o	of Service	вB					

Intersection Capacity Utilization 63.0% Analysis Period (min) 15

J → D Ø2 (R)	Ø1	₩A _{Ø9}	_{Ø12}
63 s	13 s	30 s	30 s
Ø6	▲ ø5		₩ Ø16
58 s	18 s		30 s

Lane Group	Ø9
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Satd. Flow (RTOR)	
Confl. Peds. (#/hr)	
Confl. Bikes (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	9
Permitted Phases	
Total Split (s)	30.0
Total Lost Time (s)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

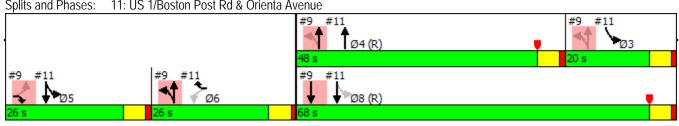
Existing 9: US 1/Boston Post Rd & Delancey Ave

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
Lane Configurations	5	1		-4↑	A				
Traffic Volume (vph)	14	55	91	815	900	24			
Future Volume (vph)	14	55	91	815	900	24			
Satd. Flow (prot)	1787	1583	0	3484	2993	0			
Flt Permitted	0.950			0.843					
Satd. Flow (perm)	1772	1583	0	2949	2993	0			
Satd. Flow (RTOR)		59							
Confl. Peds. (#/hr)	5		11			11			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93			
Heavy Vehicles (%)	1%	2%	13%	2%	5%	4%			
Parking (#/hr)					5				
Shared Lane Traffic (%)									
Lane Group Flow (vph)	15	59	0	974	994	0			
Turn Type	Perm		custom	NA	NA	-			
Protected Phases		5	6	6 4	8		3	4	
Permitted Phases	5	U	34	3	Ū		Ũ	•	
Total Split (s)	26.0	26.0	26.0	U	68.0		20.0	48.0	
Total Lost Time (s)	5.0	5.0	20.0		5.0		20.0	10.0	
Act Effct Green (s)	15.7	15.7		89.3	63.0				
Actuated g/C Ratio	0.13	0.13		0.74	0.52				
v/c Ratio	0.06	0.23		0.39	0.63				
Control Delay	43.8	13.0		1.1	22.6				
Queue Delay	0.0	0.0		0.3	0.4				
Total Delay	43.8	13.0		1.4	23.0				
LOS	D	B		A	C				
Approach Delay	19.3	D		1.4	23.0				
Approach LOS	В			A	C				
Queue Length 50th (ft)	10	0		19	277				
Queue Length 95th (ft)	30	37		19	348				
Internal Link Dist (ft)	246	07		90	543				
Turn Bay Length (ft)	210	70		70	010				
Base Capacity (vph)	310	325		2525	1571				
Starvation Cap Reductn	0	0		816	200				
Spillback Cap Reductn	0	2		0	121				
Storage Cap Reductn	0	0		0	0				
Reduced v/c Ratio	0.05	0.18		0.57	0.73				
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120)								
Offset: 69 (58%), Reference		4:NBTI	and 8-SB	T. Start of	f Yellow				
Control Type: Actuated-Coc									
Maximum v/c Ratio: 0.70	. an atou								
Intersection Signal Delay: 1	26			In	tersection	I OS' R			
Intersection Capacity Utiliza					CU Level c		C		
Analysis Period (min) 15							5		

		#9 #11	#9 #11 Ø3
		48 s	20 s
#9 #11	#9 #11	#9 #11 ↓ Ø8 (R)	
26 s	26 s	68 s	

Existing 11: US 1/Boston Post Rd & Orienta Avenue

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	٦	1	A		٦	†				
Traffic Volume (vph)	72	231	675	128	195	756				
Future Volume (vph)	72	231	675	128	195	756				
Satd. Flow (prot)	1694	1487	3461	0	1710	1757				
Flt Permitted	0.950				0.184					
Satd. Flow (perm)	1671	1487	3461	0	331	1757				
Satd. Flow (RTOR)		248								
Confl. Peds. (#/hr)	8			7	7					
Confl. Bikes (#/hr)				1						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				
Heavy Vehicles (%)	3%	5%	2%	3%	5%	4%				
Shared Lane Traffic (%)	0.10	0.10	270	0,0	0,0	170				
Lane Group Flow (vph)	77	248	864	0	210	813				
Turn Type	Perm	Prot	NA	Ŭ	custom	NA				
Protected Phases	1 0111	6	4		3 5	5 8	3	5	8	
Permitted Phases	6	Ū			8	00	0	0	U	
Total Split (s)	26.0	26.0	48.0		U		20.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				20.0	20.0	00.0	
Act Effct Green (s)	26.3	26.3	43.0		83.7	83.7				
Actuated g/C Ratio	0.22	0.22	0.36		0.70	0.70				
v/c Ratio	0.22	0.48	0.70		0.33	0.66				
Control Delay	42.1	8.6	36.6		10.8	7.1				
Queue Delay	0.0	0.0	0.0		3.9	0.8				
Total Delay	42.1	8.6	36.6		14.6	7.9				
LOS	42.1 D	A	50.0 D		В	7.7 A				
Approach Delay	16.5	Л	36.6		D	9.3				
Approach LOS	B		50.0 D			7.5 A				
Queue Length 50th (ft)	49	0	298		51	157				
Queue Length 95th (ft)	99	74	373		113	212				
Internal Link Dist (ft)	450	/4	2270		115	90				
Turn Bay Length (ft)	400		2270			70				
Base Capacity (vph)	366	519	1240		716	1303				
Starvation Cap Reductn	0	0	0		418	220				
Spillback Cap Reductin	0	0	0		410	0				
Storage Cap Reductin	0	0	0		0	0				
Reduced v/c Ratio	0.21	0.48	0.70		0.70	0.75				
	0.21	0.40	0.70		0.70	0.75				
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Referenced		4:NBTL a	and 8:SB	T, Start o	f Yellow					
Control Type: Actuated-Coor	dinated									
Maximum v/c Ratio: 0.70										
Intersection Signal Delay: 21	.0			Ir	ntersectior	LOS: C				
Intersection Capacity Utilizat Analysis Period (min) 15				10	CU Level o	of Service	А			



Existing
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ		1	ሻ	4		ሻ	††			A⊅	
Traffic Volume (vph)	97	0	90	106	40	55	77	780	0	0	839	95
Future Volume (vph)	97	0	90	106	40	55	77	780	0	0	839	95
Satd. Flow (prot)	1787	0	1599	1728	1668	0	1728	3303	0	0	3288	0
Flt Permitted	0.690			0.950			0.142					
Satd. Flow (perm)	1271	0	1557	1703	1668	0	257	3303	0	0	3288	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	17		10	10		17	26					26
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	1%	0%	1%	1%	3%	1%	1%	2%	2%	2%	4%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	107	0	99	116	104	0	85	857	0	0	1026	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		12.0	67.0			55.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	14.3		14.3	14.3	14.3		44.4	44.4			34.9	
Actuated g/C Ratio	0.18		0.18	0.18	0.18		0.57	0.57			0.45	
v/c Ratio	0.46		0.35	0.37	0.34		0.28	0.46			0.70	
Control Delay	43.9		39.8	39.8	39.3		13.8	13.2			24.0	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	43.9		39.8	39.8	39.3		13.8	13.2			24.0	
LOS	D		D	D	D		В	В			С	
Approach Delay					39.6			13.3			24.0	
Approach LOS					D			В			С	
Queue Length 50th (ft)	36		33	39	34		10	68			154	
Queue Length 95th (ft)	135		122	139	127		61	285			443	
Internal Link Dist (ft)		483			489			1683			2270	
Turn Bay Length (ft)	140			100			175					
Base Capacity (vph)	518		635	694	680		307	2668			2326	
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.21		0.16	0.17	0.15		0.28	0.32			0.44	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 78.	2											
Control Type: Semi Act-Une	coord											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 2					tersectior							
Intersection Capacity Utiliza	ation 55.8%			IC	CU Level of	of Service	вB					
Analysis Period (min) 15												

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd



Lane Group	Ø9
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Satd. Flow (RTOR)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	9
Permitted Phases	
Total Split (s)	27.0
Total Lost Time (s)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî 🗍	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	19	12	5	79	115	13
Future Volume (vph)	19	12	5	79	115	13
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	23	14	6	95	139	16
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	37	101	155			
Volume Left (vph)	23	6	0			
Volume Right (vph)	14	0	16			
Hadj (s)	-0.09	0.03	0.09			
Departure Headway (s)	4.4	4.2	4.2			
Degree Utilization, x	0.04	0.12	0.18			
Capacity (veh/h)	776	841	848			
Control Delay (s)	7.6	7.7	8.1			
Approach Delay (s)	7.6	7.7	8.1			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.9			
Level of Service			А			
Intersection Capacity Utiliz	ation		18.6%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		el 🕴			Ę	
Sign Control	Stop		Stop			Stop	
Traffic Volume (vph)	0	9	10	1	22	14	
Future Volume (vph)	0	9	10	1	22	14	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	
Hourly flow rate (vph)	0	12	13	1	28	18	
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total (vph)	12	14	46				
Volume Left (vph)	0	0	28				
Volume Right (vph)	12	1	0				
Hadj (s)	-0.58	-0.03	0.14				
Departure Headway (s)	3.4	3.9	4.1				
Degree Utilization, x	0.01	0.02	0.05				
Capacity (veh/h)	1020	906	876				
Control Delay (s)	6.5	7.0	7.3				
Approach Delay (s)	6.5	7.0	7.3				
Approach LOS	А	А	А				
Intersection Summary							
Delay			7.1				
Level of Service			А				
Intersection Capacity Utiliza	ation		18.9%	IC	U Level o	f Service	А
Analysis Period (min)			15				

$\epsilon \sim 1 \epsilon \star$	ŧ.
Movement WBL WBR NBT NBR SBL	SBT
Lane Configurations	र्स
Traffic Volume (veh/h) 5 0 0 2	120
Future Volume (Veh/h) 5 0 0 2	120
Sign Control Stop Free	Free
Grade 0% 0%	0%
Peak Hour Factor 0.91 0.91 0.91 0.91 0.91	0.91
Hourly flow rate (vph) 5 0 0 2	132
Pedestrians 2 3	
Lane Width (ft) 12.0 0.0	
Walking Speed (ft/s) 4.0 4.0	
Percent Blockage 0 0	
Right turn flare (veh)	
	None
Median storage veh)	
Upstream signal (ft) 1147	
pX, platoon unblocked	
vC, conflicting volume 141 2 2	
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol 141 2 2	
tC, single (s) 6.4 6.2 4.1	
tC, 2 stage (s)	
tF (s) 3.5 3.3 2.2	
p0 queue free % 99 100 100	
cM capacity (veh/h) 852 1083 1624	
Direction, Lane # WB 1 SB 1	
Volume Total 5 134	
Volume Left 5 2	
Volume Right 0 0	
cSH 852 1624	
Volume to Capacity 0.01 0.00	
Queue Length 95th (ft) 0 0	
Control Delay (s) 9.3 0.1	
Lane LOS A A	
Approach Delay (s) 9.3 0.1	
Approach LOS A	
Intersection Summary	
Average Delay 0.4	
Intersection Capacity Utilization 16.4% ICU Level of	Service
Analysis Period (min) 15	

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	4Î	
Traffic Volume (veh/h)	0	1	0	98	127	3
Future Volume (Veh/h)	0	1	0	98	127	3
Sign Control	Stop			Free	Free	-
Grade	1%			0%	0%	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	0	1	0	114	148	3
Pedestrians		·	Ū	2	1	Ū
Lane Width (ft)				10.0	10.0	
Walking Speed (ft/s)				4.0	4.0	
Percent Blockage				4.0 0	۰.۳ 0	
Right turn flare (veh)				U	U	
Median type				None	None	
Median storage veh)				NONC	NOTIC	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	264	152	151			
vC1, stage 1 conf vol	204	152	131			
vC2, stage 2 conf vol						
vCu, unblocked vol	264	152	151			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	726	896	1436			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	1	114	151			
Volume Left	0	0	0			
Volume Right	1	0	3			
cSH	896	1436	1700			
Volume to Capacity	0.00	0.00	0.09			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	9.0	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	9.0	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	tion		17.5%	10	CU Level d	of Service
Analysis Period (min)	-		15			
			10			

No-Build
6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	4Î		5	4Î		7	At≱		5	≜ †⊅	
Traffic Volume (vph)	116	81	91	54	78	34	147	704	51	61	783	106
Future Volume (vph)	116	81	91	54	78	34	147	704	51	61	783	106
Satd. Flow (prot)	1770	1605	0	1652	1652	0	1745	3413	0	1668	3311	0
Flt Permitted	0.640			0.534			0.132			0.195		
Satd. Flow (perm)	1160	1605	0	919	1652	0	241	3413	0	341	3311	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	21		10	10		21	22		11	11		22
Confl. Bikes (#/hr)			3									3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	1%	6%	2%	1%	1%	1%	2%	2%	1%	3%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	129	191	0	60	125	0	163	839	0	68	988	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		18.0	63.0		13.0	58.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.7	39.7		39.7	39.7		68.3	59.1		59.3	53.0	
Actuated g/C Ratio	0.29	0.29		0.29	0.29		0.50	0.43		0.44	0.39	
v/c Ratio	0.38	0.41		0.22	0.26		0.69	0.57		0.32	0.77	
Control Delay	48.7	47.3		47.1	44.6		56.2	30.9		26.4	40.9	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	48.7	47.3		47.1	44.6		56.2	30.9		26.4	40.9	
LOS	D	D		D	D		E	С		С	D	
Approach Delay		47.9			45.4			35.1			40.0	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	104	155		46	97		76	293		30	396	
Queue Length 95th (ft)	179	244		94	164		118	360		55	482	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	338	468		268	481		269	1483		231	1290	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.38	0.41		0.22	0.26		0.61	0.57		0.29	0.77	
Intersection Summary												
Cycle Length: 136												
Actuated Cycle Length: 136												
Offset: 0 (0%), Referenced t		NBTL Sta	art of Gre	en								
Control Type: Actuated-Coo												
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 3	9.4			In	tersectior	LOS: D						
Intersection Capacity Utiliza					CU Level o							
Analysis Daried (win) 15						2 3. 1.0	_					

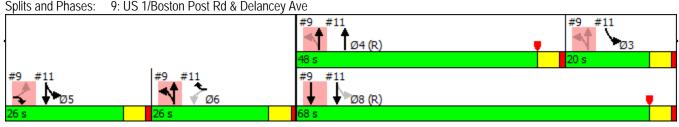
Intersection Capacity Utilization 63.8% Analysis Period (min) 15

Ø2 (R)		_ _{Ø1}	₩k _{Ø9}	<u>⊿</u> _{Ø12}
63 s		13 s	30 s	30 s
Ø6	1	Ø5		₩Ø16
58 s	18 s			30 s

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

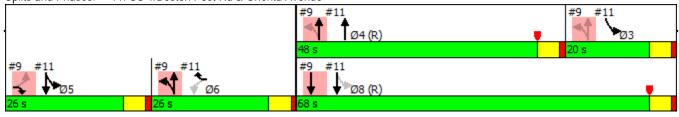
No-Build 9: US 1/Boston Post Rd & Delancey Ave

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
Lane Configurations	5	1		- 4 ↑	≜ †⊅				
Traffic Volume (vph)	14	57	93	831	919	25			
Future Volume (vph)	14	57	93	831	919	25			
Satd. Flow (prot)	1787	1583	0	3484	2993	0			
Flt Permitted	0.950			0.836					
Satd. Flow (perm)	1772	1583	0	2925	2993	0			
Satd. Flow (RTOR)		61							
Confl. Peds. (#/hr)	5		11			11			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93			
Heavy Vehicles (%)	1%	2%	13%	2%	5%	4%			
Parking (#/hr)					5				
Shared Lane Traffic (%)									
Lane Group Flow (vph)	15	61	0	994	1015	0			
Turn Type	Perm	Prot	custom	NA	NA				
Protected Phases		5	6	64	8		3	4	
Permitted Phases	5		34	3					
Total Split (s)	26.0	26.0	26.0		68.0		20.0	48.0	
Total Lost Time (s)	5.0	5.0			5.0				
Act Effct Green (s)	15.8	15.8		89.2	63.0				
Actuated g/C Ratio	0.13	0.13		0.74	0.52				
v/c Ratio	0.06	0.23		0.39	0.65				
Control Delay	43.6	12.8		1.2	22.9				
Queue Delay	0.0	0.0		0.3	0.5				
Total Delay	43.6	12.8		1.5	23.4				
LOS	D	В		А	С				
Approach Delay	18.9			1.5	23.4				
Approach LOS	В			А	С				
Queue Length 50th (ft)	10	0		20	286				
Queue Length 95th (ft)	30	38		19	358				
Internal Link Dist (ft)	246			90	543				
Turn Bay Length (ft)		70							
Base Capacity (vph)	310	327		2518	1571				
Starvation Cap Reductn	0	0		805	198				
Spillback Cap Reductn	0	2		0	125				
Storage Cap Reductn	0	0		0	0				
Reduced v/c Ratio	0.05	0.19		0.58	0.74				
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 69 (58%), Reference		4:NBTL	and 8:SB	T, Start of	f Yellow				
Control Type: Actuated-Cool	rdinated								
Maximum v/c Ratio: 0.71									
Intersection Signal Delay: 12					tersection				
Intersection Capacity Utilizat	tion 67.7%			IC	CU Level o	of Service	С		
Analysis Period (min) 15									



No-Build 11: US 1/Boston Post Rd & Orienta Avenue

	4	•	1	*	×	Ŧ					
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8		
Lane Configurations	ľ	1	A		۲	•					
Traffic Volume (vph)	73	234	690	130	197	774					
Future Volume (vph)	73	234	690	130	197	774					
Satd. Flow (prot)	1694	1487	3461	0	1710	1757					
Flt Permitted	0.950				0.176						
Satd. Flow (perm)	1671	1487	3461	0	317	1757					
Satd. Flow (RTOR)		252									
Confl. Peds. (#/hr)	8			7	7						
Confl. Bikes (#/hr)				1							
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93					
Heavy Vehicles (%)	3%	5%	2%	3%	5%	4%					
Shared Lane Traffic (%)	0,0	0,0	270	0,0	0,0	170					
Lane Group Flow (vph)	78	252	882	0	212	832					
Turn Type	Perm	Prot	NA	Ū	custom	NA					
Protected Phases	1 0111	6	4		3 5	5 8	3	5	8		
Permitted Phases	6	U	Т		8	5.0	5	0	U		
Total Split (s)	26.0	26.0	48.0		0		20.0	26.0	68.0		
Total Lost Time (s)	5.0	5.0	5.0				20.0	20.0	00.0		
Act Effct Green (s)	26.2	26.2	43.0		83.8	83.8					
Actuated g/C Ratio	0.22	0.22	0.36		0.70	0.70					
v/c Ratio	0.22	0.22	0.30		0.70	0.70					
Control Delay	42.2	8.6	37.0		11.2	7.4					
Queue Delay	42.2	0.0	0.0		4.5	0.9					
Total Delay	42.2	8.6	37.0		15.8	8.3					
LOS	42.2 D	0.0 A	57.0 D		15.8 B	0.3 A					
	16.6	A	37.0		D	9.8					
Approach Delay	10.0 B		37.0 D			9.0 A					
Approach LOS	50	0			EE						
Queue Length 50th (ft)		0	307		55	167					
Queue Length 95th (ft)	100	75	383		119	226					
Internal Link Dist (ft)	450		2270			90					
Turn Bay Length (ft)	0/1	501	1040		744	1000					
Base Capacity (vph)	364	521	1240		711	1303					
Starvation Cap Reductn	0	0	0		421	219					
Spillback Cap Reductn	0	0	0		0	0					
Storage Cap Reductn	0	0	0		0	0					
Reduced v/c Ratio	0.21	0.48	0.71		0.73	0.77					
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 69 (58%), Referenced	l to phase	4:NBTL a	and 8:SB	T, Start o	f Yellow						
Control Type: Actuated-Coord											
Maximum v/c Ratio: 0.71											
Intersection Signal Delay: 21.	5			In	itersectior	LOS: C					
Intersection Signal Delay: 21. Intersection Capacity Utilization					itersectior CU Level o		A				

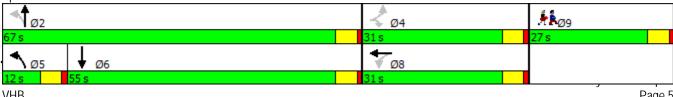


No-Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۳		1	ሻ	et 🗧		٦	<u></u>			≜ ⊅	
Traffic Volume (vph)	98	0	91	107	41	56	78	797	0	0	858	96
Future Volume (vph)	98	0	91	107	41	56	78	797	0	0	858	96
Satd. Flow (prot)	1787	0	1599	1728	1668	0	1728	3303	0	0	3288	0
Flt Permitted	0.688			0.950			0.137					
Satd. Flow (perm)	1268	0	1557	1703	1668	0	248	3303	0	0	3288	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	17		10	10		17	26					26
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	1%	0%	1%	1%	3%	1%	1%	2%	2%	2%	4%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	108	0	100	118	107	0	86	876	0	0	1048	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		12.0	67.0			55.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	14.5		14.5	14.5	14.5		45.0	45.0			35.5	
Actuated g/C Ratio	0.18		0.18	0.18	0.18		0.57	0.57			0.45	
v/c Ratio	0.46		0.35	0.38	0.35		0.29	0.47			0.71	
Control Delay	44.2		40.1	40.1	39.7		14.0	13.3			24.3	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	44.2		40.1	40.1	39.7		14.0	13.3			24.3	
LOS	D		D	D	D		В	В			С	
Approach Delay					39.9			13.4			24.3	
Approach LOS					D			В			С	
Queue Length 50th (ft)	37		34	40	36		10	71			159	_
Queue Length 95th (ft)	136	100	123	141	130		63	294			458	
Internal Link Dist (ft)	1.10	483		100	489		475	1683			2270	
Turn Bay Length (ft)	140		(07	100	(70		175	2/55			0007	
Base Capacity (vph)	511		627	686	672		301	2655			2307	_
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	_
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.21		0.16	0.17	0.16		0.29	0.33			0.45	
Intersection Summary												
Cycle Length: 125												_
Actuated Cycle Length: 79												
Control Type: Semi Act-Uno	coord											
Maximum v/c Ratio: 0.71	2.0											
Intersection Signal Delay: 2					tersection							
Intersection Capacity Utiliza	alion 56.4%			IC	CU Level o	DI Service	βB					

Analysis Period (min) 15

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd



Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	19	12	5	80	116	13
Future Volume (vph)	19	12	5	80	116	13
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	23	14	6	96	140	16
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	37	102	156			
Volume Left (vph)	23	6	0			
Volume Right (vph)	14	0	16			
Hadj (s)	-0.09	0.03	0.09			
Departure Headway (s)	4.4	4.2	4.2			
Degree Utilization, x	0.04	0.12	0.18			
Capacity (veh/h)	775	841	848			
Control Delay (s)	7.6	7.7	8.1			
Approach Delay (s)	7.6	7.7	8.1			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.9			
Level of Service			А			
Intersection Capacity Utiliz	ation		18.6%	IC	U Level c	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT						
Lane Configurations	Y		el 🕴			ধ						
Sign Control	Stop		Stop			Stop						
Traffic Volume (vph)	0	9	10	1	22	14						
Future Volume (vph)	0	9	10	1	22	14						
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78						
Hourly flow rate (vph)	0	12	13	1	28	18						
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total (vph)	12	14	46									
Volume Left (vph)	0	0	28									
Volume Right (vph)	12	1	0									
Hadj (s)	-0.58	-0.03	0.14									
Departure Headway (s)	3.4	3.9	4.1									
Degree Utilization, x	0.01	0.02	0.05									
Capacity (veh/h)	1020	906	876									
Control Delay (s)	6.5	7.0	7.3									
Approach Delay (s)	6.5	7.0	7.3									
Approach LOS	А	А	А									
Intersection Summary												
Delay			7.1									
Level of Service			А									
Intersection Capacity Utiliza	ation		18.9%	IC	U Level o	f Service		А	А	А	А	А
Analysis Period (min)			15									

	4	•	Ť	1	1	ţ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	۲					र्स		
Traffic Volume (veh/h)	5	0	0	0	2	122		
Future Volume (Veh/h)	5	0	0	0	2	122		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91		
Hourly flow rate (vph)	5	0	0	0	2	134		
Pedestrians	2		3					
Lane Width (ft)	12.0		0.0					
Walking Speed (ft/s)	4.0		4.0					
Percent Blockage	0		0					
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	143	2			2			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	143	2			2			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	99	100			100			
cM capacity (veh/h)	850	1083			1624			
Direction, Lane #	WB 1	SB 1						
Volume Total	5	136						
Volume Left	5	2						
Volume Right	0	0						
cSH	850	1624						
Volume to Capacity	0.01	0.00						
Queue Length 95th (ft)	0	0						
Control Delay (s)	9.3	0.1						
Lane LOS	А	А						
Approach Delay (s)	9.3	0.1						
Approach LOS	А							
Intersection Summary								
Average Delay			0.4					
Intersection Capacity Utiliz	zation		16.5%	IC	U Level o	of Service	ę	
Analysis Period (min)			15					
			10					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî.	
Traffic Volume (veh/h)	0	1	0	99	129	3
Future Volume (Veh/h)	0	1	0	99	129	3
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	0	1	0	115	150	3
Pedestrians				2	1	
Lane Width (ft)				10.0	10.0	
Walking Speed (ft/s)				4.0	4.0	
Percent Blockage				0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	268	154	153			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	268	154	153			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	723	894	1434			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	<u> </u>	115	153			
Volume Left	0	0	0			
Volume Right	1	0	3			
cSH	894	1434	1700			
Volume to Capacity	0.00	0.00	0.09			
Queue Length 95th (ft)	0.00	0.00	0.09			
Control Delay (s)	9.0	0.0	0.0			
Lane LOS	9.0 A	0.0	0.0			
Approach Delay (s)	9.0	0.0	0.0			
Approach LOS	9.0 A	0.0	0.0			
	A					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilizat	tion		17.6%	IC	CU Level o	of Service
Analysis Period (min)			15			

Build 6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	eî 👘		<u>۲</u>	4Î		<u>۲</u>	∱ ⊅		۳.	∱ ⊅	
Traffic Volume (vph)	116	86	91	63	80	35	147	704	70	62	783	106
Future Volume (vph)	116	86	91	63	80	35	147	704	70	62	783	106
Satd. Flow (prot)	1770	1610	0	1652	1652	0	1745	3397	0	1668	3311	0
Flt Permitted	0.635	1/10	0	0.525	1/50	0	0.132	0007	0	0.185	0011	0
Satd. Flow (perm)	1151	1610	0	903	1652	0	241	3397	0	324	3311	0
Satd. Flow (RTOR)	21		10	10		01	22		11	11		22
Confl. Peds. (#/hr)	21		10 3	10		21	22		11	11		22
Confl. Bikes (#/hr) Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	3 0.90
Heavy Vehicles (%)	2%	0.90 1%	0.90 6%	2%	0.90	0.90	0.90	2%	2%	0.90	3%	2%
Shared Lane Traffic (%)	Ζ/0	1 /0	070	Z /0	1 70	1 /0	1 /0	Z /0	2 /0	1 /0	370	Z /0
Lane Group Flow (vph)	129	197	0	70	128	0	163	860	0	69	988	0
Turn Type	Perm	NA	U	Perm	NA	0	pm+pt	NA	0	pm+pt	NA	0
Protected Phases	T CITI	12		I CIIII	16		5	2		рштрі 1	6	
Permitted Phases	12	12		16	10		2	2		6	U	
Total Split (s)	30.0	30.0		30.0	30.0		18.0	63.0		13.0	58.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.7	39.7		39.7	39.7		68.3	59.1		59.3	53.0	
Actuated g/C Ratio	0.29	0.29		0.29	0.29		0.50	0.43		0.44	0.39	
v/c Ratio	0.39	0.42		0.27	0.27		0.69	0.58		0.34	0.77	
Control Delay	48.8	47.6		47.9	44.7		56.2	31.4		27.4	40.9	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	48.8	47.6		47.9	44.7		56.2	31.4		27.4	40.9	
LOS	D	D		D	D		E	С		С	D	
Approach Delay		48.1			45.8			35.3			40.0	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	104	160		54	99		76	303		31	396	
Queue Length 95th (ft)	180	251		108	168		118	371		55	482	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	335	469		263	481		269	1476		224	1290	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.39	0.42		0.27	0.27		0.61	0.58		0.31	0.77	
Intersection Summary												
Cycle Length: 136												
Actuated Cycle Length: 136												_
Offset: 0 (0%), Referenced		NBIL, Sta	art of Gre	en								
Control Type: Actuated-Coo	ainated											
Maximum v/c Ratio: 0.77	0.4				torecetter							
Intersection Signal Delay: 3					tersection							
Intersection Capacity Utiliza	1011 04.2%			IC	CU Level of	DI SELVICE	eC					

Analysis Period (min) 15

J √ Ø2 (R)		Ø1	₩A _{Ø9}	 Ø12
63 s		13 s	30 s	30 s
↓ Ø6	1	Ø5		₩ Ø16
58 s	18 s			30 s

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Build 9: US 1/Boston Post Rd & Delancey Ave

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
Lane Configurations	۲	1		- 4 ↑	A				
Traffic Volume (vph)	14	62	96	838	934	25			
Future Volume (vph)	14	62	96	838	934	25			
Satd. Flow (prot)	1787	1583	0	3483	2993	0			
Flt Permitted	0.950			0.825					
Satd. Flow (perm)	1772	1583	0	2886	2993	0			
Satd. Flow (RTOR)		67							
Confl. Peds. (#/hr)	5		11			11			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93			
Heavy Vehicles (%)	1%	2%	13%	2%	5%	4%			
Parking (#/hr)					5				
Shared Lane Traffic (%)									
Lane Group Flow (vph)	15	67	0	1004	1031	0			
Turn Type	Perm		custom	NA	NA				
Protected Phases		5	6	64	8		3	4	
Permitted Phases	5		34	3					
Total Split (s)	26.0	26.0	26.0		68.0		20.0	48.0	
Total Lost Time (s)	5.0	5.0			5.0				
Act Effct Green (s)	16.0	16.0		89.0	63.0				
Actuated g/C Ratio	0.13	0.13		0.74	0.52				
v/c Ratio	0.06	0.25		0.40	0.66				
Control Delay	43.4	12.5		1.2	23.2				
Queue Delay	0.0	0.0		0.3	0.5				
Total Delay	43.4	12.5		1.5	23.7				
LOS	D	В		А	С				
Approach Delay	18.1			1.5	23.7				
Approach LOS	В			А	С				
Queue Length 50th (ft)	10	0		20	293				
Queue Length 95th (ft)	30	40		20	367				
Internal Link Dist (ft)	246			90	543				
Turn Bay Length (ft)		70							
Base Capacity (vph)	310	332		2507	1571				
Starvation Cap Reductn	0	0		795	196				
Spillback Cap Reductn	0	2		0	121				
Storage Cap Reductn	0	0		0	0				
Reduced v/c Ratio	0.05	0.20		0.59	0.75				
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 69 (58%), Reference		4:NBTL	and 8:SB	T, Start of	f Yellow				
Control Type: Actuated-Coo									
Maximum v/c Ratio: 0.72									
Intersection Signal Delay: 1	3.0			In	tersectior	LOS: B			
Intersection Capacity Utiliza					CU Level o		С		
Analysis Period (min) 15									
5									

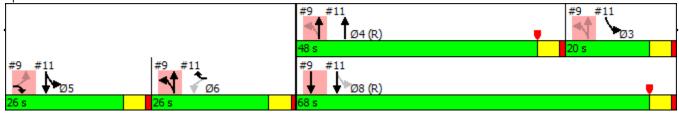
Splits and Phases: 9: US 1/Boston Post Rd & Delancey Ave

		#9 #11	#9 #11 • Ø3
		48 s	20 s
#9 #11	#9 #11	#9 #11 ↓ Ø8 (R)	
26 s	26 s	68 s	

Build <u>11: US 1/Boston Post Rd & Orienta Avenue</u>

	4	•	1	*	1	Ļ				
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	۲	1	At≱		۲	•				
Traffic Volume (vph)	73	243	691	134	217	774				
Future Volume (vph)	73	243	691	134	217	774				
Satd. Flow (prot)	1694	1487	3460	0	1710	1757				
Flt Permitted	0.950				0.174					
Satd. Flow (perm)	1671	1487	3460	0	313	1757				
Satd. Flow (RTOR)		261								
Confl. Peds. (#/hr)	8			7	7					
Confl. Bikes (#/hr)				1						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				
Heavy Vehicles (%)	3%	5%	2%	3%	5%	4%				
Shared Lane Traffic (%)										
ane Group Flow (vph)	78	261	887	0	233	832				
Furn Type	Perm	Prot	NA	-	custom	NA				
Protected Phases		6	4		3 5	58	3	5	8	
Permitted Phases	6	Ŭ			8	50				
Total Split (s)	26.0	26.0	48.0		Ŭ		20.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				2010	2010	0010	
Act Effct Green (s)	26.0	26.0	43.0		84.0	84.0				
Actuated g/C Ratio	0.22	0.22	0.36		0.70	0.70				
//c Ratio	0.22	0.50	0.72		0.37	0.68				
Control Delay	42.5	8.7	37.2		12.7	7.2				
Queue Delay	0.0	0.0	0.0		5.7	0.9				
Total Delay	42.5	8.7	37.2		18.4	8.0				
LOS	D	A	D		B	A				
Approach Delay	16.5	7.	37.2		D	10.3				
Approach LOS	B		D			В				
Queue Length 50th (ft)	50	0	310		70	156				
Queue Length 95th (ft)	100	76	385		141	211				
Internal Link Dist (ft)	450	70	2270		171	90				
Turn Bay Length (ft)	730		2270			70				
Base Capacity (vph)	361	526	1239		709	1303				
Starvation Cap Reductn	0	0	0		411	216				
Spillback Cap Reductn	0	0	0		0	210				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.22	0.50	0.72		0.78	0.77				
	0.22	0.50	0.72		0.70	0.77				
ntersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120	d to phase				fValler					
Offset: 69 (58%), Reference		4:INBTE &	ana 9:28	i, Start O	reliow					
Control Type: Actuated-Coor	unated									
Maximum v/c Ratio: 0.72				,	tore anti-					
ntersection Signal Delay: 21					itersection		٨			
ntersection Capacity Utilizat	uon 53.1%			10	U Level o	of Service	A			
Analysis Period (min) 15										

Splits and Phases: 11: US 1/Boston Post Rd & Orienta Avenue



Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۳		1	ሻ	et 🗧		٦	<u></u>			≜ ⊅	
Traffic Volume (vph)	100	0	92	110	42	64	78	797	0	0	858	96
Future Volume (vph)	100	0	92	110	42	64	78	797	0	0	858	96
Satd. Flow (prot)	1787	0	1599	1728	1660	0	1728	3303	0	0	3288	0
Flt Permitted	0.682			0.950			0.136					
Satd. Flow (perm)	1257	0	1557	1703	1660	0	246	3303	0	0	3288	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	17		10	10		17	26					26
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	1%	0%	1%	1%	3%	1%	1%	2%	2%	2%	4%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	110	0	101	121	116	0	86	876	0	0	1048	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		12.0	67.0			55.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	15.0		15.0	15.0	15.0		45.3	45.3			35.8	
Actuated g/C Ratio	0.19		0.19	0.19	0.19		0.57	0.57			0.45	
v/c Ratio	0.47		0.35	0.38	0.37		0.29	0.47			0.71	
Control Delay	44.1		39.8	39.9	39.9		14.3	13.6			24.6	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	44.1		39.8	39.9	39.9		14.3	13.6			24.6	
LOS	D		D	D	D		В	В			С	
Approach Delay					39.9			13.7			24.6	
Approach LOS					D			В			С	
Queue Length 50th (ft)	38		34	41	39		11	73			162	
Queue Length 95th (ft)	138	100	124	144	139		63	298			465	
Internal Link Dist (ft)	1.10	483		100	489		475	1683			2270	
Turn Bay Length (ft)	140		(00	100	(/)		175	0/41			0000	
Base Capacity (vph)	503		622	681	663		299	2641			2293	_
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	_
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.22		0.16	0.18	0.17		0.29	0.33			0.46	
Intersection Summary												
Cycle Length: 125	7											_
Actuated Cycle Length: 79.												
Control Type: Semi Act-Uno	coord											
Maximum v/c Ratio: 0.71					1							
Intersection Signal Delay: 2					tersection							
Intersection Capacity Utiliza	alion 56.6%			IC	CU Level o	DI Service	βB					

Analysis Period (min) 15

Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd



Lane Group	Ø9		
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Satd. Flow (RTOR)			
Confl. Peds. (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	9		
Permitted Phases			
Total Split (s)	27.0		
Total Lost Time (s)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (ft)			
Queue Length 95th (ft)			
Internal Link Dist (ft)			
Turn Bay Length (ft)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

	٦	\mathbf{r}	1	1	Ŧ	<
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ŧ	el el	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	29	12	5	80	116	37
Future Volume (vph)	29	12	5	80	116	37
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	35	14	6	96	140	45
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	49	102	185			
Volume Left (vph)	35	6	0			
Volume Right (vph)	14	0	45			
Hadj (s)	-0.01	0.03	-0.01			
Departure Headway (s)	4.5	4.2	4.1			
Degree Utilization, x	0.06	0.12	0.21			
Capacity (veh/h)	741	827	861			
Control Delay (s)	7.8	7.8	8.2			
Approach Delay (s)	7.8	7.8	8.2			
Approach LOS	А	А	А			
Intersection Summary						
Delay			8.0			
Level of Service			А			
Intersection Capacity Utiliz	zation		18.7%	IC	U Level c	f Service
Analysis Period (min)			15			

	4	•	Ť	~	1	Ŧ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۲		eî 🗧			र्स
Sign Control	Stop		Stop			Stop
Traffic Volume (vph)	0	23	10	1	48	14
Future Volume (vph)	0	23	10	1	48	14
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78
Hourly flow rate (vph)	0	29	13	1	62	18
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total (vph)	29	14	80			
Volume Left (vph)	0	0	62			
Volume Right (vph)	29	1	0			
Hadj (s)	-0.58	-0.03	0.17			
Departure Headway (s)	3.5	4.0	4.1			
Degree Utilization, x	0.03	0.02	0.09			
Capacity (veh/h)	990	876	859			
Control Delay (s)	6.6	7.1	7.6			
Approach Delay (s)	6.6	7.1	7.6			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.3			
Level of Service			А			
Intersection Capacity Utiliz	zation		20.4%	IC	U Level c	of Service
Analysis Period (min)			15			

	4	•	1	1	1	Ļ	•	
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	٦					र्स		
Traffic Volume (veh/h)	17	0	0	0	21	122		
Future Volume (Veh/h)	17	0	0	0	21	122		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91		
Hourly flow rate (vph)	19	0	0	0	23	134		
Pedestrians	2		3					
Lane Width (ft)	12.0		0.0					
Walking Speed (ft/s)	4.0		4.0					
Percent Blockage	0		0					
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	185	2			2			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	185	2			2			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	98	100			99			
cM capacity (veh/h)	794	1083			1624			
Direction, Lane #	WB 1	SB 1						
Volume Total	19	157						
Volume Left	19	23						
Volume Right	0	0						
cSH	794	1624						
Volume to Capacity	0.02	0.01						
Queue Length 95th (ft)	2	1						
Control Delay (s)	9.6	1.2						
Lane LOS	A	A						
Approach Delay (s)	9.6	1.2						
Approach LOS	A							
Intersection Summary								
Average Delay			2.1					
Intersection Capacity Utiliz	ration		17.6%	IC		of Service	2	
Analysis Period (min)	adon		17.078	10	U LEVEL			
			10					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî.	
Traffic Volume (veh/h)	0	1	0	109	152	3
Future Volume (Veh/h)	0	1	0	109	152	3
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Hourly flow rate (vph)	0	1	0	127	177	3
Pedestrians				2	1	
Lane Width (ft)				10.0	10.0	
Walking Speed (ft/s)				4.0	4.0	
Percent Blockage				0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	306	180	180			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	306	180	180			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	687	864	1402			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	1	127	180			
Volume Left	0	0	0			
Volume Right	1	0	3			
cSH	864	1402	1700			
Volume to Capacity	0.00	0.00	0.11			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	9.2	0.0	0.0			
Lane LOS	A	010	010			
Approach Delay (s)	9.2	0.0	0.0			
Approach LOS	A	010	010			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	ration		18.8%	10	CU Level o	f Service
Analysis Period (min)			10.0 %	IC		JEIVICE
Analysis Fellou (IIIII)			10			

Existing 6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

	٦	-	\mathbf{r}	1	-	*	1	1	1	5	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1.		ሻ	4		ኘ	≜1 ≱		ሻ	≜ 1≽	
Traffic Volume (vph)	148	60	120	40	60	45	147	781	60	46	714	145
Future Volume (vph)	148	60	120	40	60	45	147	781	60	46	714	145
Satd. Flow (prot)	1787	1590	0	1668	1630	0	1745	3439	0	1668	3330	0
Flt Permitted	0.671			0.550			0.149			0.156		
Satd. Flow (perm)	1254	1590	0	952	1630	0	272	3439	0	272	3330	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	5		14	14		5	31		24	24		31
Confl. Bikes (#/hr)									2			5
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	1%	1%	2%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	154	188	0	42	110	0	153	877	0	48	895	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		18.0	57.0		12.0	51.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.2	39.2		39.2	39.2		61.8	52.9		52.0	46.0	
Actuated g/C Ratio	0.30	0.30		0.30	0.30		0.48	0.41		0.40	0.36	
v/c Ratio	0.40	0.39		0.15	0.22		0.60	0.62		0.28	0.75	
Control Delay	45.4	43.8		43.0	41.1		47.5	32.8		27.1	41.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	45.4	43.8		43.0	41.1		47.5	32.8		27.1	41.4	
LOS	D	D		D	D		D	С		С	D	
Approach Delay		44.5			41.6			34.9			40.7	
Approach LOS		D			D			С			D	
Queue Length 50th (ft)	118	143		30	79		70	305		21	344	
Queue Length 95th (ft)	198	227		66	139		111	377		42	425	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	381	483		289	495		283	1410		187	1187	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.40	0.39		0.15	0.22		0.54	0.62		0.26	0.75	
Intersection Summary												
Cycle Length: 129												
Actuated Cycle Length: 129												
Offset: 0 (0%), Referenced to		L, Start of	Green									
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.75	_											
Intersection Signal Delay: 38.9					tersection							
Intersection Capacity Utilization	on 64.4%			IC	U Level of	Service C						
Analysis Period (min) 15												

Ø2 (R)		Ø1	∦1 ø9	<u>↓</u> _{Ø12}
57 s		12 s	30 s	30 s
↓ ø ₆	1	Ø5		₩ Ø16
51 s	18 s			30 s

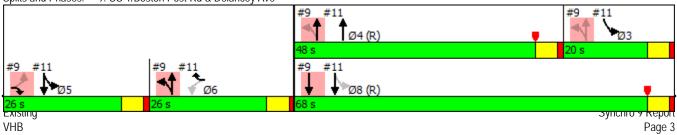
Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Existing 9: US 1/Boston Post Rd & Delancey Ave

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
Lane Configurations	5	1		-a†	A				
Traffic Volume (vph)	18	63	42	951	822	25			
Future Volume (vph)	18	63	42	951	822	25			
Satd. Flow (prot)	1787	1599	0	3567	3051	0			
Flt Permitted	0.950			0.955					
Satd. Flow (perm)	1750	1599	0	3412	3051	0			
Satd. Flow (RTOR)		66							
Confl. Peds. (#/hr)	12		16						
Confl. Bikes (#/hr)						7			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95			
Heavy Vehicles (%)	1%	1%	1%	1%	3%	4%			
Parking (#/hr)					5				
Shared Lane Traffic (%)					-				
Lane Group Flow (vph)	19	66	0	1045	891	0			
Turn Type	Perm	Prot	custom	NA	NA	Ū			
Protected Phases	T CITI	5	6	6 4	8		3	4	
Permitted Phases	5	U	34	3	U		U	•	
Total Split (s)	26.0	26.0	26.0	5	68.0		20.0	48.0	
Total Lost Time (s)	5.0	5.0	20.0		5.0		20.0	10.0	
Act Effct Green (s)	14.4	14.4		90.6	63.0				
Actuated g/C Ratio	0.12	0.12		0.76	0.52				
v/c Ratio	0.09	0.27		0.39	0.56				
Control Delay	45.4	13.1		0.8	20.8				
Queue Delay	0.0	0.0		0.4	0.1				
Total Delay	45.4	13.1		1.3	20.9				
LOS	+3.+ D	B		A	20.7 C				
Approach Delay	20.4	U		1.3	20.9				
Approach LOS	C			A	C				
Queue Length 50th (ft)	13	0		12	234				
Queue Length 95th (ft)	35	40		12	295				
Internal Link Dist (ft)	246	40		90	543				
Turn Bay Length (ft)	210	70		70	010				
Base Capacity (vph)	306	334		2674	1601				
Starvation Cap Reductn	0	0		1020	0				
Spillback Cap Reductn	0	1		0	108				
Storage Cap Reductin	0	0		0	0				
Reduced v/c Ratio	0.06	0.20		0.63	0.60				
	0.00	0.20		0.00	0.00				
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120	ta ultar. A ME								
Offset: 69 (58%), Referenced		SIL and 8	s:SBT, Star	t of Yellow					
Control Type: Actuated-Coord	inated								
Maximum v/c Ratio: 0.80	7					00.5			
Intersection Signal Delay: 10.					tersection I				
Intersection Capacity Utilization	on 66.9%			IC	U Level of	Service C			

Intersection Capacity Utilization 66.9% Analysis Period (min) 15 ICU Level of Service C

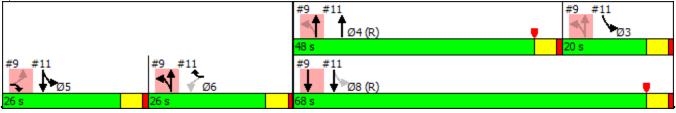
Splits and Phases: 9: US 1/Boston Post Rd & Delancey Ave



Existing 11: US 1/Boston Post Rd & Orienta Avenue

✓	*	1	1	1	Ŧ					
WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8		
			129							
	1516	3512	0		1809					
		16								
4			11	11						
			3							
0.95	0.95	0.95	0.95	0.95	0.95					
53	163	1018	0	200	732					
			-							
						3	5	8		
6	Ŭ			8	5.0	Ŭ	U	J.		
	26.0	48.0		Ŭ		20.0	26.0	68.0		
						2010	2010	0010		
				82.4	82.4					
	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			Ŭ						
	0			71						
	57			171						
400		2270			70					
201	171	1268		660	12/1					
0.11	0.01	0.00		0.07	0.01					
nhaso 1.ME	STL and S	SRT Stort	of Vellow	1						
aicu										
			In	tersection I	05.0					
				11013566110111	_UJ. U					
53.8%				CU Level of	Sonvico A					
	WBL WBL 50 50 1711 0.950 1699 4 0.95 2% 53 Perm 6 26.0 5.0 27.6 0.23 0.14 40.1 0.0 40.1 D 16.3 B 33 74 450 391 0 0 0.14 vectors 0.14 0.95 0.23 0.14 40.1 0.95 0.23 0.14 40.1 0.0 0.0 0.14 1.00 0.0 0.0 0.14 1.00 0.0 0.0 0.0 0.14 0.0 0.0 0.0 0.14 0.0 0.0 0.0 0.0 0.14 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	50 155 50 155 50 155 1711 1516 0.950 1699 1516 163 4 0.95 0.95 2% 3% 53 163 Perm Prot 6 6 6 26.0 26.0 5.0 5.0 27.6 27.6 0.23 0.23 0.14 0.34 40.1 8.5 0.0 0.0 40.1 8.5 D A 16.3 B 33 0 74 59 450 391 474 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	50         155         838           50         155         838           50         155         838           1711         1516         3512           0.950         163         16           1699         1516         3512           163         16         4           0.95         0.95         0.95           2%         3%         1%           53         163         1018           Perm         Prot         NA           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           6         4           16         3           0.23         0.23         0.36           0.14         0.34         0.80           40.0         B         D           16.3	50         155         838         129           50         155         838         129           1711         1516         3512         0           0.950         163         16         11           3         163         16         11           3         0.95         0.95         0.95         0.95           2%         3%         1%         1%         1%           53         163         1018         0         0           Perm         Prot         NA         6         4           6         2         2         3.0.36         0         0           0.23         0.23         0.36         0         0         0           0.14         0.34         0.80         40.1         8.5         40.0           0.14         0.34         0.80         40.1         8.5         40.0           0         0         0         0         0         0           16.3         40.0         2270         391         474         1268           0         0         0         0         0         0           0         0	50         155         838         129         190           50         155         838         129         190           1711         1516         3512         0         1710           0.950         0.119         163         16         11           163         16         11         11         11           3         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%           53         163         1018         0         200           Perm         Prot         NA         custom         6           6         4         3.5         6         8           26.0         26.0         48.0         5.0         5.0           5.0         5.0         5.0         5.0         5.0           27.6         27.6         43.0         82.4         0.35           40.1         8.5         40.0         16.9         0.0           0.1         8.5         40.0         20.1         14.9           0.3         0         365         71         74           16.3         40.0 <t< td=""><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         163         16         11         11           4         11         11         11         3           0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         3.5         5.8           6         8         26.0         26.0         48.0           5.0         5.0         5.0         5.9         2.7.6         27.6         43.0         82.4         82.4           0.23         0.23         0.36         0.69         0.69         0.59           0.0         0.0         0         3.2         0.4           40.1         8.5         40.0         8.7</td><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         163         16         1809           163         16         11         11         1           3         0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         5.8         3           6         8         20.0         5.0         5.0           5.0         5.0         5.0         20.0         5.0         5.9           40.1         8.5         40.0         16.9         5.2         0.0         0.0         0.59           40.1         8.5         40.0         20.1         5.6         0         A         16.3         40.0         <td< td=""><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         214         1809           0.95         0.119         1         11         11         1           4         11         11         1         3         3           0.95         0.95         0.95         0.95         0.95         296           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         26.0         20.0         26.0           5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0           27.6         27.6         43.0         82.4         82.4         82.4         0.23         0.4         0.1         8.5         40.0         16.9         5.</td><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         163         16           4         11         11         3         0           4         11         11         1         3           0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         68.0           5.0         5.0         5.0         5.0         5.9           27.6         27.6         43.0         82.4         82.4           0.23         0.36         0.69         0.4         40.1         8.5           0.0         0.0</td><td>S0         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         0         0         0           163         16         1         1         1         1           4         11         11         3         3         0         0         95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%         1%         5%         1%           53         163         1018         0         200         732         7         7         7         7         7         8         3         5         8         3         5         8         5         8         5         8         3         5         8         6         6         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10</td></td<></td></t<>	50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         163         16         11         11           4         11         11         11         3           0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         3.5         5.8           6         8         26.0         26.0         48.0           5.0         5.0         5.0         5.9         2.7.6         27.6         43.0         82.4         82.4           0.23         0.23         0.36         0.69         0.69         0.59           0.0         0.0         0         3.2         0.4           40.1         8.5         40.0         8.7	50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         163         16         1809           163         16         11         11         1           3         0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         5.8         3           6         8         20.0         5.0         5.0           5.0         5.0         5.0         20.0         5.0         5.9           40.1         8.5         40.0         16.9         5.2         0.0         0.0         0.59           40.1         8.5         40.0         20.1         5.6         0         A         16.3         40.0 <td< td=""><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         214         1809           0.95         0.119         1         11         11         1           4         11         11         1         3         3           0.95         0.95         0.95         0.95         0.95         296           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         26.0         20.0         26.0           5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0           27.6         27.6         43.0         82.4         82.4         82.4         0.23         0.4         0.1         8.5         40.0         16.9         5.</td><td>50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         163         16           4         11         11         3         0           4         11         11         1         3           0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         68.0           5.0         5.0         5.0         5.0         5.9           27.6         27.6         43.0         82.4         82.4           0.23         0.36         0.69         0.4         40.1         8.5           0.0         0.0</td><td>S0         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         0         0         0           163         16         1         1         1         1           4         11         11         3         3         0         0         95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%         1%         5%         1%           53         163         1018         0         200         732         7         7         7         7         7         8         3         5         8         3         5         8         5         8         5         8         3         5         8         6         6         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10</td></td<>	50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         214         1809           0.95         0.119         1         11         11         1           4         11         11         1         3         3           0.95         0.95         0.95         0.95         0.95         296           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         26.0         20.0         26.0           5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0           27.6         27.6         43.0         82.4         82.4         82.4         0.23         0.4         0.1         8.5         40.0         16.9         5.	50         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         163         16           4         11         11         3         0           4         11         11         1         3           0.95         0.95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%           53         163         1018         0         200         732           Perm         Prot         NA         custom         NA           6         4         35         58         3         5           6         8         20.0         26.0         68.0           5.0         5.0         5.0         5.0         5.9           27.6         27.6         43.0         82.4         82.4           0.23         0.36         0.69         0.4         40.1         8.5           0.0         0.0	S0         155         838         129         190         695           50         155         838         129         190         695           1711         1516         3512         0         1710         1809           0.950         0.119         0         0         0         0           163         16         1         1         1         1           4         11         11         3         3         0         0         95         0.95         0.95         0.95           2%         3%         1%         1%         5%         1%         1%         5%         1%           53         163         1018         0         200         732         7         7         7         7         7         8         3         5         8         3         5         8         5         8         5         8         3         5         8         6         6         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10

Splits and Phases: 11: US 1/Boston Post Rd & Orienta Avenue



Existing
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦		1	۲	eî.		۲	<b>†</b> †			At≱	
Traffic Volume (vph)	115	0	120	105	55	70	85	820	0	0	712	101
Future Volume (vph)	115	0	120	105	55	70	85	820	0	0	712	101
Satd. Flow (prot)	1787	0	1568	1728	1667	0	1728	3336	0	0	3369	0
Flt Permitted	0.673			0.950			0.176					
Satd. Flow (perm)	1252	0	1540	1718	1667	0	319	3336	0	0	3369	0
Satd. Flow (RTOR)			126		46						14	
Confl. Peds. (#/hr)	10		4	4		10	19					19
Confl. Bikes (#/hr)									5			5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	2%	3%	1%	4%	2%	1%	1%	2%	2%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	121	0	126	111	132	0	89	863	0	0	855	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		14.0	67.0			53.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	15.7		15.7	15.7	15.7		40.0	40.0			29.3	
Actuated g/C Ratio	0.21		0.21	0.21	0.21		0.53	0.53			0.39	
v/c Ratio	0.46		0.30	0.31	0.34		0.26	0.49			0.65	
Control Delay	40.8		9.6	35.7	26.2		14.6	14.8			24.7	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	40.8		9.6	35.7	26.2		14.6	14.8			24.7	
LOS	D		A	D	C		В	В			C	
Approach Delay	2			5	30.6		2	14.7			24.7	
Approach LOS					С			В			С	
Queue Length 50th (ft)	38		0	34	26		11	71			125	
Queue Length 95th (ft)	148		51	131	118		68	302			365	
Internal Link Dist (ft)	110	483	01	101	489		00	1683			2270	
Turn Bay Length (ft)		100	140	100	107		175	1005			2210	
Base Capacity (vph)	545		741	747	751		381	2716			2401	
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.22		0.17	0.15	0.18		0.23	0.32			0.36	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 75.4												
Control Type: Semi Act-Uncoo	rd											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay: 21.2				Ini	tersection	0S: C						
Intersection Capacity Utilizatio					U Level of							
Analysis Period (min) 15				10	2 20101 01							
Splits and Phases: 20: US 1	/Boston Post	Rd & Rich	bell Rd/O	ld Boston I	Post Rd							

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67 s		31 s	27 s
<b>Ø</b> 5	▼ Ø6	<b>₩</b> Ø8	
14 s	53 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ŧ	el el	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	13	7	6	70	77	19
Future Volume (vph)	13	7	6	70	77	19
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	15	8	7	79	87	21
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	23	86	108			
Volume Left (vph)	15	7	0			
Volume Right (vph)	8	0	21			
Hadj (s)	-0.06	0.03	-0.10			
Departure Headway (s)	4.2	4.1	3.9			
Degree Utilization, x	0.03	0.10	0.12			
Capacity (veh/h)	807	861	902			
Control Delay (s)	7.4	7.5	7.5			
Approach Delay (s)	7.4	7.5	7.5			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.5			
Level of Service			А			
Intersection Capacity Utilization			19.9%	IC	U Level of S	Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT								
Lane Configurations	W.		4			÷								
Sign Control	Stop		Stop			Stop								
Traffic Volume (vph)	0	14	11	0	38	13								
Future Volume (vph)	0	14	11	0	38	13								
Peak Hour Factor	0.70	0.70	0.70	0.70	0.70	0.70								
Hourly flow rate (vph)	0	20	16	0	54	19								
Direction, Lane #	WB 1	NB 1	SB 1											
Volume Total (vph)	20	16	73											
Volume Left (vph)	0	0	54											
Volume Right (vph)	20	0	0											
Hadj (s)	-0.58	0.02	0.16											
Departure Headway (s)	3.5	4.0	4.1											
Degree Utilization, x	0.02	0.02	0.08											
Capacity (veh/h)	995	874	865											
Control Delay (s)	6.6	7.1	7.5											
Approach Delay (s)	6.6	7.1	7.5											
Approach LOS	А	А	А											
Intersection Summary														
Delay			7.3											
Level of Service			А											
Intersection Capacity Utilization			19.5%	ICI	U Level of S	ervice		A	А	А	А	А	А	А
Analysis Period (min)			15											

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	5					4	
Traffic Volume (veh/h)	6	0	0	0	2	•• 98	
Future Volume (Veh/h)	6	0	0	0	2	98	
Sign Control	Stop	0	Free	U	2	Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Hourly flow rate (vph)	0.00	0.00	0.00	0.00	2	111	
Pedestrians	19	0	0	0	Z	111	
Lane Width (ft)	19						
Walking Speed (ft/s)	4.0						
Percent Blockage	2						
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (ft)			1147				
pX, platoon unblocked							
vC, conflicting volume	134	19			19		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	134	19			19		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	99	100			100		
cM capacity (veh/h)	847	1042			1579		
					,		
Direction, Lane #	WB 1	SB 1					
Volume Total	7	113					
Volume Left	7	2					
Volume Right	0	0					
cSH	847	1579					
Volume to Capacity	0.01	0.00					
Queue Length 95th (ft)	1	0					
Control Delay (s)	9.3	0.1					
Lane LOS	A	A					
Approach Delay (s)	9.3	0.1					
Approach LOS	A	0.1					
••							
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilization			15.3%	IC	U Level of Se	ervice	
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥			નુ	f,		
Traffic Volume (veh/h)	4	2	0	83	94	3	
Future Volume (Veh/h)	4	2	0	83	94	3	
Sign Control	Stop			Free	Free		
Grade	1%			0%	0%		
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	
Hourly flow rate (vph)	4	2	0	93	106	3	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	200	108	109				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	200	108	109				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	99	100	100				
cM capacity (veh/h)	790	949	1488				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	6	93	109				
Volume Left	4	0	0				
Volume Right	2	0	3				
cSH	837	1488	1700				
Volume to Capacity	0.01	0.00	0.06				
Queue Length 95th (ft)	1	0	0				
Control Delay (s)	9.3	0.0	0.0				
Lane LOS	А						
Approach Delay (s)	9.3	0.0	0.0				
Approach LOS	А						
Intersection Summary							
Average Delay			0.3				
Intersection Capacity Utilization			15.1%	IC	U Level of S	Service	
Analysis Period (min)			15				

No-Build
6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	14		<u>۲</u>	ef 👘		<u>۲</u>	<b>≜</b> 1≽		- <b>T</b>	<b>≜</b> 1≽	
Traffic Volume (vph)	150	61	122	41	61	46	149	799	61	47	731	147
Future Volume (vph)	150	61	122	41	61	46	149	799	61	47	731	147
Satd. Flow (prot)	1787	1590	0	1668	1630	0	1745	3439	0	1668	3331	0
Flt Permitted	0.668			0.545			0.141			0.147		
Satd. Flow (perm)	1249	1590	0	943	1630	0	257	3439	0	256	3331	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	5		14	14		5	31		24	24		31
Confl. Bikes (#/hr)									2			5
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	1%	1%	2%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	156	191	0	43	112	0	155	896	0	49	914	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		18.0	57.0		12.0	51.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.2	39.2		39.2	39.2		61.8	52.9		52.0	46.0	
Actuated g/C Ratio	0.30	0.30		0.30	0.30		0.48	0.41		0.40	0.36	
v/c Ratio	0.41	0.40		0.15	0.23		0.63	0.64		0.29	0.77	
Control Delay	45.7	43.9		43.1	41.1		49.8	33.1		28.2	42.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	45.7	43.9		43.1	41.1		49.8	33.1		28.2	42.1	
LOS	D	D		D	D		D	С		С	D	
Approach Delay		44.7			41.7			35.6			41.4	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	120	146		30	81		71	314		21	354	
Queue Length 95th (ft)	201	231		68	141		113	386		43	436	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	379	483		286	495		277	1410		181	1187	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.41	0.40		0.15	0.23		0.56	0.64		0.27	0.77	
Intersection Summary												
Cycle Length: 129												
Actuated Cycle Length: 129												
Offset: 0 (0%), Referenced to	phase 2:NBT	L, Start of	Green									
Control Type: Actuated-Coord												
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 39.4	4			Int	ersection I	LOS: D						
Intersection Capacity Utilizatio					U Level of							
Analysis Period (min) 15												

Ø2 (R)		Ø1	<b>∦1</b> ø9	<u>↓</u> _{Ø12}
57 s		12 s	30 s	30 s
<b>↓</b> ø ₆	•	Ø5		<b>₩</b> Ø16
51 s	18 s			30 s

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

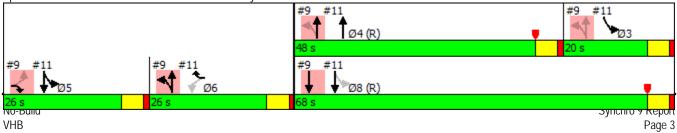
#### No-Build 9: US 1/Boston Post Rd & Delancey Ave

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4	
Lane Configurations	7	1		-a†	A				
Traffic Volume (vph)	18	65	44	970	840	25			
Future Volume (vph)	18	65	44	970	840	25			
Satd. Flow (prot)	1787	1599	0	3567	3051	0			
Flt Permitted	0.950			0.955					
Satd. Flow (perm)	1750	1599	0	3412	3051	0			
Satd. Flow (RTOR)		68							
Confl. Peds. (#/hr)	12		16						
Confl. Bikes (#/hr)						7			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95			
Heavy Vehicles (%)	1%	1%	1%	1%	3%	4%			
Parking (#/hr)					5				
Shared Lane Traffic (%)									
Lane Group Flow (vph)	19	68	0	1067	910	0			
Turn Type	Perm	Prot	custom	NA	NA				
Protected Phases		5	6	64	8		3	4	
Permitted Phases	5		34	3					
Total Split (s)	26.0	26.0	26.0		68.0		20.0	48.0	
Total Lost Time (s)	5.0	5.0			5.0				
Act Effct Green (s)	14.5	14.5		90.5	63.0				
Actuated g/C Ratio	0.12	0.12		0.75	0.52				
v/c Ratio	0.09	0.27		0.40	0.57				
Control Delay	45.2	13.0		0.9	21.0				
Queue Delay	0.0	0.0		0.5	0.2				
Total Delay	45.2	13.0		1.3	21.2				
LOS	D	В		А	С				
Approach Delay	20.1			1.3	21.2				
Approach LOS	С			А	С				
Queue Length 50th (ft)	13	0		12	242				
Queue Length 95th (ft)	35	40		13	304				
Internal Link Dist (ft)	246			90	543				
Turn Bay Length (ft)		70							
Base Capacity (vph)	306	335		2669	1601				
Starvation Cap Reductn	0	0		1011	0				
Spillback Cap Reductn	0	2		0	137				
Storage Cap Reductn	0	0		0	0				
Reduced v/c Ratio	0.06	0.20		0.64	0.62				
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 69 (58%), Referenced		BTL and 8	3:SBT, Star	t of Yellow					
Control Type: Actuated-Coordi	inated								
Maximum v/c Ratio: 0.82									
Intersection Signal Delay: 10.9	)			Int	tersection l	_OS: B			
Interception Concelly 141111-	- 17 00/			10	111	Comilar C			

Intersection Signal Delay: 10.9 Intersection Capacity Utilization 67.9% Analysis Period (min) 15

ICU Level of Service C

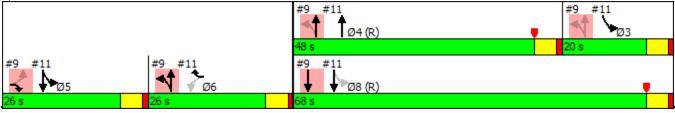
Splits and Phases: 9: US 1/Boston Post Rd & Delancey Ave



#### No-Build 11: US 1/Boston Post Rd & Orienta Avenue

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
Lane Configurations	5	1	A		٦	•				_
Traffic Volume (vph)	51	157	857	131	192	713				
Future Volume (vph)	51	157	857	131	192	713				
Satd. Flow (prot)	1711	1516	3513	0	1710	1809				
Flt Permitted	0.950				0.111					
Satd. Flow (perm)	1699	1516	3513	0	200	1809				
Satd. Flow (RTOR)		165	16							
Confl. Peds. (#/hr)	4			11	11					
Confl. Bikes (#/hr)				3						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95				
Heavy Vehicles (%)	2%	3%	1%	1%	5%	1%				
Shared Lane Traffic (%)	270	070	170	170	070	170				
Lane Group Flow (vph)	54	165	1040	0	202	751				
Turn Type	Perm	Prot	NA	U	custom	NA				
Protected Phases	- I UIII	6	4		3 5	58	3	5	8	
Permitted Phases	6	U	7		8	50	J	5	0	
Total Split (s)	26.0	26.0	48.0		U		20.0	26.0	68.0	
Total Lost Time (s)	5.0	5.0	5.0				20.0	20.0	00.0	
Act Effct Green (s)	27.5	27.5	43.0		82.5	82.5				
	0.23	0.23	0.36		02.5	02.5				
Actuated g/C Ratio	0.23	0.23	0.36		0.89	0.69				
	40.3	8.5	40.8		17.6	5.5				
Control Delay										
Queue Delay	0.0	0.0	0.0		2.5	0.4				
Total Delay	40.3	8.5	40.8		20.1	5.9				
LOS	D	А	D		С	A				
Approach Delay	16.3		40.8			8.9				
Approach LOS	В	0	D		75	A				
Queue Length 50th (ft)	33	0	376		75	115				
Queue Length 95th (ft)	74	60	465		148	150				
Internal Link Dist (ft)	450		2270			90				
Turn Bay Length (ft)										
Base Capacity (vph)	388	474	1269		664	1341				
Starvation Cap Reductn	0	0	0		343	205				
Spillback Cap Reductn	0	0	0		0	0				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.14	0.35	0.82		0.63	0.66				
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Referenced t	o phase 4:NE	BTL and 8:	SBT, Start	of Yellov	V					
Control Type: Actuated-Coordi										
Maximum v/c Ratio: 0.82										
Intersection Signal Delay: 24.7				Ir	ntersection I	OS: C				
Intersection Capacity Utilizatior	1 54.5%			IC	CU Level of	Service A				

Splits and Phases: 11: US 1/Boston Post Rd & Orienta Avenue



No-Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ		1	<u> </u>	eî 👘		<u>۲</u>	<b>*</b> *			At≱	
Traffic Volume (vph)	116	0	122	106	56	71	86	838	0	0	729	102
Future Volume (vph)	116	0	122	106	56	71	86	838	0	0	729	102
Satd. Flow (prot)	1787	0	1568	1728	1667	0	1728	3336	0	0	3373	0
Flt Permitted	0.669			0.950			0.171					
Satd. Flow (perm)	1244	0	1540	1718	1667	0	310	3336	0	0	3373	0
Satd. Flow (RTOR)			128		46						14	
Confl. Peds. (#/hr)	10		4	4		10	19					19
Confl. Bikes (#/hr)									5			5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	2%	3%	1%	4%	2%	1%	1%	2%	2%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	122	0	128	112	134	0	91	882	0	0	874	0
Turn Type	Perm		Perm	Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases	4		4	8			2					
Total Split (s)	31.0		31.0	31.0	31.0		14.0	67.0			53.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0		5.0	5.0			5.0	
Act Effct Green (s)	15.9		15.9	15.9	15.9		40.9	40.9			30.2	
Actuated g/C Ratio	0.21		0.21	0.21	0.21		0.54	0.54			0.40	
v/c Ratio	0.47		0.30	0.31	0.35		0.27	0.49			0.65	
Control Delay	41.6		9.6	36.2	26.7		14.6	14.8			24.7	
Queue Delay	0.0		0.0	0.0	0.0		0.0	0.0			0.0	
Total Delay	41.6		9.6	36.2	26.7		14.6	14.8			24.7	
LOS	D		А	D	С		В	В			С	
Approach Delay					31.1			14.8			24.7	
Approach LOS					С			В			С	
Queue Length 50th (ft)	39		0	35	27		12	75			130	
Queue Length 95th (ft)	151		52	133	120		69	311			375	
Internal Link Dist (ft)		483			489			1683			2270	
Turn Bay Length (ft)			140	100			175					
Base Capacity (vph)	532		731	734	739		375	2709			2370	
Starvation Cap Reductn	0		0	0	0		0	0			0	
Spillback Cap Reductn	0		0	0	0		0	0			0	
Storage Cap Reductn	0		0	0	0		0	0			0	
Reduced v/c Ratio	0.23		0.18	0.15	0.18		0.24	0.33			0.37	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 76.4												
Control Type: Semi Act-Unco	ora											
Maximum v/c Ratio: 0.65	2					00.0						
Intersection Signal Delay: 21.					ersection							
Intersection Capacity Utilization Analysis Period (min) 15	DN 59.4%			IC	U Level of	Service B	i					
Splits and Phases: 20: US	1/Boston Post	Rd & Rich	bell Rd/O	ld Boston I	Post Rd							

↑ Ø2 Ø2 Ø2 Ø Ø Ø Ø Ø Ø Ø Ø Ø Ø Ø Ø Ø Ø		✓ Ø4	
67 s		31 s	27 s
<b>▲</b> Ø5	↓ Ø6	<b>₩</b> Ø8	
14 s 53	3 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

	≯	$\mathbf{r}$	1	1	.↓	∢
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ŧ	el el	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	13	7	6	71	78	19
Future Volume (vph)	13	7	6	71	78	19
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	15	8	7	80	88	21
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	23	87	109			
Volume Left (vph)	15	7	0			
Volume Right (vph)	8	0	21			
Hadj (s)	-0.06	0.03	-0.10			
Departure Headway (s)	4.2	4.1	3.9			
Degree Utilization, x	0.03	0.10	0.12			
Capacity (veh/h)	806	861	902			
Control Delay (s)	7.4	7.5	7.5			
Approach Delay (s)	7.4	7.5	7.5			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.5			
Level of Service			А			
Intersection Capacity Utilization	1		19.9%	IC	U Level of	Service
Analysis Period (min)			15			

	1	•	1	1	1	†						
Movement	WBL	WBR	NBT	NBR	SBL	SBT						
Lane Configurations	¥.		f,			र्स						
Sign Control	Stop		Stop			Stop						
Traffic Volume (vph)	0	14	11	0	38	13						
Future Volume (vph)	0	14	11	0	38	13						
Peak Hour Factor	0.70	0.70	0.70	0.70	0.70	0.70						
Hourly flow rate (vph)	0	20	16	0	54	19						
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total (vph)	20	16	73									
Volume Left (vph)	0	0	54									
Volume Right (vph)	20	0	0									
Hadj (s)	-0.58	0.02	0.16									
Departure Headway (s)	3.5	4.0	4.1									
Degree Utilization, x	0.02	0.02	0.08									
Capacity (veh/h)	995	874	865									
Control Delay (s)	6.6	7.1	7.5									
Approach Delay (s)	6.6	7.1	7.5									
Approach LOS	А	А	А									
Intersection Summary												
Delay			7.3									
Level of Service			А									
Intersection Capacity Utilization			19.5%	ICI	U Level of Se	ervice		А	А	А	А	А
Analysis Period (min)			15									

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	-	•	Ť	1	¥ .	.↓	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations		WDR	NDT	NDK	JDL		
Traffic Volume (veh/h)	<b>1</b> 6	0	0	0	2	<b>4</b> 99	
Future Volume (Veh/h)	6	0	0	0	2	99 99	
		0		0	Z		
Sign Control	Stop		Free			Free	
Grade	0%	0.00	0%	0.00	0.00	0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Hourly flow rate (vph)	7	0	0	0	2	113	
Pedestrians	19						
Lane Width (ft)	12.0						
Walking Speed (ft/s)	4.0						
Percent Blockage	2						
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (ft)			1147				
pX, platoon unblocked							
vC, conflicting volume	136	19			19		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	136	19			19		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	99	100			100		
cM capacity (veh/h)	845	1042			1579		
					1377		
Direction, Lane #	WB 1	SB 1					
Volume Total	7	115					
Volume Left	7	2					
Volume Right	0	0					
cSH	845	1579					
Volume to Capacity	0.01	0.00					
Queue Length 95th (ft)	1	0					
Control Delay (s)	9.3	0.1					
Lane LOS	A	А					
Approach Delay (s)	9.3	0.1					
Approach LOS	A						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilization			15.3%	IC	U Level of S	ervice	
Analysis Period (min)			15				

MovementEBLEBRNBLNBTSBTSBRLane ConfigurationsY11Traffic Volume (veh/h)42084953Future Volume (veh/h)42084953Sign ControlStopFreeFreeGrade1%0%0%Peak Hour Factor0.890.890.890.890.890.890.89
Lane ConfigurationsYAPTraffic Volume (veh/h)42084953Future Volume (Veh/h)42084953Sign ControlStopFreeFreeGrade1%0%0%0%
Lane ConfigurationsYImage: Configuration of the second seco
Traffic Volume (veh/h)         4         2         0         84         95         3           Future Volume (Veh/h)         4         2         0         84         95         3           Sign Control         Stop         Free         Free           Grade         1%         0%         0%
Future Volume (Veh/h)         4         2         0         84         95         3           Sign Control         Stop         Free         Free           Grade         1%         0%         0%
Sign ControlStopFreeFreeGrade1%0%0%
Grade 1% 0% 0%
Hourly flow rate (vph) 4 2 0 94 107 3
Pedestrians
Lane Width (ft)
Walking Speed (ft/s)
Percent Blockage
Right turn flare (veh)
Median type None None
Median storage veh)
Upstream signal (ft)
pX, platoon unblocked
vC, conflicting volume 202 108 110
vC1, stage 1 conf vol
vC2, stage 2 conf vol
vCu, unblocked vol 202 108 110
tC, single (s) 6.4 6.2 4.1
tC, 2 stage (s)
tF (s)     3.5     3.3     2.2       p0 queue free %     99     100     100
cM capacity (veh/h) 788 948 1486
Direction, Lane # EB 1 NB 1 SB 1
Volume Total 6 94 110
Volume Left 4 0 0
Volume Right 2 0 3
cSH 835 1486 1700
Volume to Capacity 0.01 0.00 0.06
Queue Length 95th (ft) 1 0 0
Control Delay (s) 9.3 0.0 0.0
Lane LOS A
Approach Delay (s) 9.3 0.0 0.0
Approach LOS A
Intersection Summary
Average Delay 0.3
Intersection Capacity Utilization 15.2% ICU Level of Service
Analysis Period (min) 15

Build 6: US 1/Boston Post Rd & Weaver Street/Hommocks Road

	≯	+	*	4	Ļ	•	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	4Î		1	et		1	A		1	A⊅	
Traffic Volume (vph)	150	64	122	52	64	46	149	799	74	48	731	147
Future Volume (vph)	150	64	122	52	64	46	149	799	74	48	731	147
Satd. Flow (prot)	1787	1594	0	1668	1632	0	1745	3429	0	1668	3331	0
Flt Permitted	0.663			0.541			0.141			0.141		
Satd. Flow (perm)	1239	1594	0	937	1632	0	257	3429	0	246	3331	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	5		14	14		5	31		24	24		31
Confl. Bikes (#/hr)									2			5
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	1%	1%	2%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	156	194	0	54	115	0	155	909	0	50	914	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		12			16		5	2		1	6	
Permitted Phases	12			16			2			6		
Total Split (s)	30.0	30.0		30.0	30.0		18.0	57.0		12.0	51.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Act Effct Green (s)	39.2	39.2		39.2	39.2		61.8	52.9		52.0	46.0	
Actuated g/C Ratio	0.30	0.30		0.30	0.30		0.48	0.41		0.40	0.36	
v/c Ratio	0.41	0.40		0.19	0.23		0.63	0.65		0.30	0.77	
Control Delay	45.8	44.0		43.5	41.2		49.8	33.4		29.2	42.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	45.8	44.0		43.5	41.2		49.8	33.4		29.2	42.1	
LOS	D	D		D	D		D	С		С	D	
Approach Delay		44.8			41.9			35.8			41.4	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	120	149		39	83		71	321		22	354	
Queue Length 95th (ft)	202	235		81	144		113	395		44	436	
Internal Link Dist (ft)		190			209			263			1683	
Turn Bay Length (ft)	145			150			180			140		
Base Capacity (vph)	376	484		284	496		277	1406		178	1187	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.41	0.40		0.19	0.23		0.56	0.65		0.28	0.77	
Intersection Summary												
Cycle Length: 129												
Actuated Cycle Length: 129												
Offset: 0 (0%), Referenced t		NBTL, Sta	art of Gre	en								
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 39					tersectior							
Intersection Capacity Utiliza	tion 65.4%			IC	CU Level of	of Service	еC					
Analysis Period (min) 15												

		Ø1	₩ <b>1</b> 299	A _{Ø12}
57 s		12 s	30 s	30 s
<b>₩</b> Ø6	1	Ø5		₩ Ø16
51 s	18 s			30 s

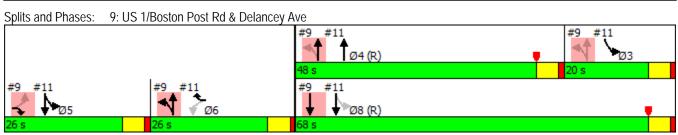
Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	30.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

# Build 9: US 1/Boston Post Rd & Delancey Ave

	٨	*	•	1	ţ	1				
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	Ø4		
Lane Configurations	۲.	1			<b>≜</b> ⊅					
Traffic Volume (vph)	18	68	46	978	850	25				
Future Volume (vph)	18	68	46	978	850	25				
Satd. Flow (prot)	1787	1599	0	3567	3051	0				
Flt Permitted	0.950			0.955						
Satd. Flow (perm)	1750	1599	0	3411	3051	0				
Satd. Flow (RTOR)		72								
Confl. Peds. (#/hr)	12		16							
Confl. Bikes (#/hr)						7				
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95				
Heavy Vehicles (%)	1%	1%	1%	1%	3%	4%				
Parking (#/hr)					5					
Shared Lane Traffic (%)										
Lane Group Flow (vph)	19	72	0	1077	921	0				
Turn Type	Perm		custom	NA	NA					
Protected Phases		5	6	64	8		3	4		
Permitted Phases	5		34	3						
Total Split (s)	26.0	26.0	26.0		68.0		20.0	48.0		
Total Lost Time (s)	5.0	5.0			5.0					
Act Effct Green (s)	14.8	14.8		90.2	63.0					
Actuated g/C Ratio	0.12	0.12		0.75	0.52					
v/c Ratio	0.09	0.28		0.40	0.58					
Control Delay	45.1	12.7		0.9	21.2					
Queue Delay	0.0	0.0		0.5	0.2					
Total Delay	45.1	12.8		1.4	21.4					
LOS	D	В		А	С					
Approach Delay	19.5			1.4	21.4					
Approach LOS	В			А	С					
Queue Length 50th (ft)	13	0		13	245					
Queue Length 95th (ft)	35	41		14	310					
Internal Link Dist (ft)	246			90	543					
Turn Bay Length (ft)		70								
Base Capacity (vph)	306	339		2663	1601					
Starvation Cap Reductn	0	0		997	0					
Spillback Cap Reductn	0	3		0	174					
Storage Cap Reductn	0	0		0	0					
Reduced v/c Ratio	0.06	0.21		0.65	0.65					
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Reference		4:NBTI	and 8:SB	T, Start o	f Yellow					
Control Type: Actuated-Coc			2.1.0 0100	., ctart 0						
Maximum v/c Ratio: 0.82										
Intersection Signal Delay: 1	1.0			In	itersectior	LOS: B				
Intersection Capacity Utiliza					CU Level o		С			
Analysis Period (min) 15							-			

# Build 9: US 1/Boston Post Rd & Delancey Ave

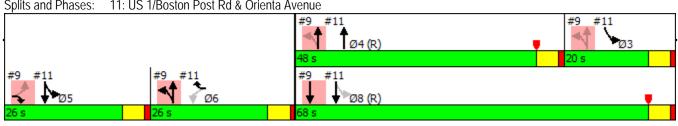
Splits and Phases: 9: US 1/Boston Post Rd & Delancey Ave



# Build 11: US 1/Boston Post Rd & Orienta Avenue

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ane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø3	Ø5	Ø8	
ane Configurations	ľ	1	A⊅		ľ	•				
Traffic Volume (vph)	51	168	857	133	206	713				
uture Volume (vph)	51	168	857	133	206	713				
Satd. Flow (prot)	1711	1516	3512	0	1710	1809				
-It Permitted	0.950				0.110					
Satd. Flow (perm)	1699	1516	3512	0	198	1809				
Satd. Flow (RTOR)		177	16							
Confl. Peds. (#/hr)	4			11	11					
Confl. Bikes (#/hr)				3						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95				
Heavy Vehicles (%)	2%	3%	1%	1%	5%	1%				
Shared Lane Traffic (%)	270	0.10	170	170	0,0					
ane Group Flow (vph)	54	177	1042	0	217	751				
Furn Type	Perm	Prot	NA		custom	NA				
Protected Phases	1 0111	6	4		3 5	58	3	5	8	
Permitted Phases	6	U	т		8	5.0	5	5	U	
Fotal Split (s)	26.0	26.0	48.0		0		20.0	26.0	68.0	
Fotal Lost Time (s)	5.0	5.0	5.0				20.0	20.0	00.0	
Act Effct Green (s)	27.2	27.2	43.0		82.8	82.8				
Actuated g/C Ratio	0.23	0.23	0.36		02.0	02.0				
//c Ratio	0.23	0.23	0.30		0.38	0.60				
Control Delay	40.5	8.4	41.0		18.7	5.4				
Queue Delay	40.5	0.4	41.0 0.0		2.7	0.4				
Fotal Delay	40.5	8.4	41.0		2.7	5.8				
_OS	40.5 D	0.4 A	41.0 D		21.4 C	0.0 A				
	15.9	A	41.0		C	9.3				
Approach Delay			41.0 D			9.3 A				
Approach LOS	B	0	377		07					
Queue Length 50th (ft)	33	0			87	108				
Queue Length 95th (ft)	75	62	466		164	140				
nternal Link Dist (ft)	450		2270			90				
Furn Bay Length (ft)	0.05	404	10/0		( ( 0	40.44				
Base Capacity (vph)	385	481	1268		663	1341				
Starvation Cap Reductn	0	0	0		333	204				
Spillback Cap Reductn	0	0	0		0	0				
Storage Cap Reductn	0	0	0		0	0				
Reduced v/c Ratio	0.14	0.37	0.82		0.66	0.66				
ntersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 69 (58%), Reference		4:NBTL a	and 8:SB	T, Start o	f Yellow					
Control Type: Actuated-Coor	rdinated									
Vaximum v/c Ratio: 0.82										
ntersection Signal Delay: 24	1.7			In	itersectior	LOS: C				
ntersection Capacity Utilizat	ion 55.3%			IC	CU Level o	of Service	В			

Splits and Phases: 11: US 1/Boston Post Rd & Orienta Avenue



Build
20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

Lane GroupEBLEBTEBRWBLWBTWBRNBLNBTNBRSBLSBTSBRLane ConfigurationsIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
Traffic Volume (vph)118012211057808683800729102Future Volume (vph)118012211057808683800729102Satd. Flow (prot)178701568172816590172833360033730Fit Permitted0.6460.9500.171033360033730Satd. Flow (perm)12020154017181659031033360033730Satd. Flow (RTOR)12851140191414101919Confl. Bikes (#/hr)104410191919Confl. Bikes (#/hr)1%2%0.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.95
Traffic Volume (vph)118012211057808683800729102Future Volume (vph)118012211057808683800729102Satd. Flow (prot)178701568172816590172833360033730Flt Permitted0.6460.9500.1710178703373033730Satd. Flow (perm)12020154017181659031033360033730Satd. Flow (RTOR)128511410191414191919Confl. Peds. (#/hr)104410191919Confl. Bikes (#/hr)102%0.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.950.9
Satd. Flow (prot)       1787       0       1568       1728       1659       0       1728       3336       0       0       3373       0         Flt Permitted       0.646       0.950       0.171       0       1787       0       1568       1728       1659       0       1728       3336       0       0       3373       0         Flt Permitted       0.646       0.950       0.171       0       1787       0       3373       0         Satd. Flow (perm)       1202       0       1540       1718       1659       0       310       3336       0       0       3373       0         Satd. Flow (perm)       1202       0       1540       1718       1659       0       310       3336       0       0       3373       0         Satd. Flow (RTOR)       128       51       128       51       14       14       14       14       14       14       14       14       14       16       14       16       16       16       16       16       16       16       16       16       16       16       16       17       17       17       17       17       16       17
Fit Permitted       0.646       0.950       0.171         Satd. Flow (perm)       1202       0       1540       1718       1659       0       310       3336       0       0       3373       0         Satd. Flow (perm)       1202       0       1540       1718       1659       0       310       3336       0       0       3373       0         Satd. Flow (RTOR)       128       51       14       14       10       19       14         Confl. Peds. (#/hr)       10       4       4       10       19       19       19         Confl. Bikes (#/hr)       5       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.9
Satd. Flow (perm)       1202       0       1540       1718       1659       0       310       3336       0       0       3373       0         Satd. Flow (RTOR)       128       51       114       14       14       14       19       14         Confl. Peds. (#/hr)       10       4       4       10       19       19       19         Confl. Bikes (#/hr)       5       5       5       5       5       5       5         Peak Hour Factor       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95 <t< td=""></t<>
Satd. Flow (RTOR)       128       51       14         Confl. Peds. (#/hr)       10       4       4       10       19       19         Confl. Bikes (#/hr)       10       4       4       10       19       5       5         Peak Hour Factor       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95
Confl. Peds. (#/hr)       10       4       4       10       19       19         Confl. Bikes (#/hr)       5       5       5       5       5       5         Peak Hour Factor       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95
Confl. Bikes (#/hr)       5         Peak Hour Factor       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95       0.95
Peak Hour Factor         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95         0.95
Heavy Vehicles (%)         1%         2%         3%         1%         4%         2%         1%         1%         2%         2%         1%         1%           Shared Lane Traffic (%)         Lane Group Flow (vph)         124         0         128         116         144         0         91         882         0         0         874         0
Shared Lane Traffic (%)           Lane Group Flow (vph)         124         0         128         116         144         0         91         882         0         0         874         0
Lane Group Flow (vph)         124         0         128         116         144         0         91         882         0         0         874         0
Turn TypePermPermNApm+ptNANA
Protected Phases 8 5 2 6
Permitted Phases 4 4 8 2
Total Split (s)         31.0         31.0         31.0         14.0         67.0         53.0
Total Lost Time (s)         5.0         5.0         5.0         5.0         5.0           Att Effet Council (c)         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         16.2         <
Act Effct Green (s)         16.3         16.3         16.3         41.0         30.4           Act Effct Green (s)         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21         0.21
Actuated g/C Ratio         0.21         0.21         0.21         0.53         0.53         0.39           v/a Datia         0.20         0.22         0.27         0.27         0.53         0.49
v/c Ratio         0.49         0.30         0.32         0.37         0.27         0.50         0.65           Control Delay         42.2         9.6         36.2         26.5         14.8         15.0         24.9
Control Delay         42.2         9.6         36.2         26.5         14.8         15.0         24.9           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0
Total Delay         42.2         9.6         36.2         26.5         14.8         15.0         24.9
Itilia Delay         42.2         9.0         50.2         20.5         14.6         15.0         24.7           LOS         D         A         D         C         B         B         C
Approach Delay         30.8         15.0         24.9
Approach LOS C B C
Queue Length 50th (ft)         40         0         36         29         12         77         132
Queue Length 95th (ft)         155         52         137         69         311         375
Internal Link Dist (ft) 483 489 1683 2270
Turn Bay Length (ft)         140         100         175
Base Capacity (vph) 509 726 729 733 373 2705 2354
Starvation Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0
Spillback Cap Reductn 0 0 0 0 0 0 0 0
Storage Cap Reductn 0 0 0 0 0 0 0
Reduced v/c Ratio         0.24         0.18         0.16         0.20         0.24         0.33         0.37
Intersection Summary
Cycle Length: 125
Actuated Cycle Length: 77
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.65
Intersection Signal Delay: 21.5 Intersection LOS: C
Intersection Capacity Utilization 60.1% ICU Level of Service B
Analysis Period (min) 15

#### Splits and Phases: 20: US 1/Boston Post Rd & Richbell Rd/Old Boston Post Rd

<b>↑</b> _{Ø2}	√ ø4	
67 s	31 s	27 s
<b>▲</b> Ø5 <b>↓</b> Ø6	<b>₩</b> Ø8	
14 s 53 s	31 s	

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Total Split (s)	27.0	
Total Lost Time (s)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ę	ę.	
Sign Control	Stop			Stop	Stop	
Traffic Volume (vph)	25	7	6	71	78	35
Future Volume (vph)	25	7	6	71	78	35
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	28	8	7	80	88	39
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total (vph)	36	87	127			
Volume Left (vph)	28	7	0			
Volume Right (vph)	8	0	39			
Hadj (s)	0.04	0.03	-0.17			
Departure Headway (s)	4.4	4.1	3.9			
Degree Utilization, x	0.04	0.10	0.14			
Capacity (veh/h)	779	848	907			
Control Delay (s)	7.6	7.6	7.5			
Approach Delay (s)	7.6	7.6	7.5			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.6			
Level of Service			А			
Intersection Capacity Utilization	ation		19.9%	IC	U Level c	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Υ		eî 🗧			4
Sign Control	Stop		Stop			Stop
Traffic Volume (vph)	0	30	11	0	56	13
Future Volume (vph)	0	30	11	0	56	13
Peak Hour Factor	0.70	0.70	0.70	0.70	0.70	0.70
Hourly flow rate (vph)	0	43	16	0	80	19
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total (vph)	43	16	99			
Volume Left (vph)	0	0	80			
Volume Right (vph)	43	0	0			
Hadj (s)	-0.58	0.02	0.18			
Departure Headway (s)	3.6	4.1	4.2			
Degree Utilization, x	0.04	0.02	0.11			
Capacity (veh/h)	973	854	849			
Control Delay (s)	6.7	7.2	7.7			
Approach Delay (s)	6.7	7.2	7.7			
Approach LOS	А	А	А			
Intersection Summary						
Delay			7.4			
Level of Service			А			
Intersection Capacity Utiliz	zation		20.5%	IC	U Level c	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	٦					र्स		
Traffic Volume (veh/h)	20	0	0	0	15	99		
Future Volume (Veh/h)	20	0	0	0	15	99		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88		
Hourly flow rate (vph)	23	0	0	0	17	113		
Pedestrians	19							
Lane Width (ft)	12.0							
Walking Speed (ft/s)	4.0							
Percent Blockage	2							
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (ft)			1147					
pX, platoon unblocked								
vC, conflicting volume	166	19			19			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	166	19			19			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	97	100			99			
cM capacity (veh/h)	805	1042			1579			
Direction, Lane #	WB 1	SB 1						
Volume Total	23	130						
Volume Left	23	17						
Volume Right	0	0						
cSH	805	1579						
Volume to Capacity	0.03	0.01						
Queue Length 95th (ft)	2	1						
Control Delay (s)	9.6	1.0						
Lane LOS	A	A						
Approach Delay (s)	9.6	1.0						
Approach LOS	A							
Intersection Summary								
Average Delay			2.3					
Intersection Capacity Utiliza	ation		16.0%	IC	Ulevelo	of Service	ڊ	
Analysis Period (min)			10.070	10				
			10					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî.	
Traffic Volume (veh/h)	4	2	0	96	111	3
Future Volume (Veh/h)	4	2	0	96	111	3
Sign Control	Stop			Free	Free	
Grade	1%			0%	0%	
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	4	2	0	108	125	3
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)					110110	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	234	126	128			
vC1, stage 1 conf vol	201	120	120			
vC2, stage 2 conf vol						
vCu, unblocked vol	234	126	128			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.11	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	756	926	1464			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	6	108	128			
Volume Left	4	0	0			
Volume Right	2	0	3			
cSH	805	1464	1700			
Volume to Capacity	0.01	0.00	0.08			
Queue Length 95th (ft)	1	0	0			
Control Delay (s)	9.5	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	9.5	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliz	zation		16.0%	IC	CU Level c	of Service
Analysis Period (min)			15			
			10			



# Appendix

Description

Count Sheets

#### Study Name BOSTON POST RD AT HOMMOCKS RD (297650/298442/298569) Start Date 03-10-2016 Start Time 7:00 AM

Vehic	les																
		Southbou Southb				Westbo Westb				Northbo Northb			Eastbound St. Eastbound				
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Riaht	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	
7:00 AM	16	56	Trigrit 14	0-10111 0	 14	16	rigiii 1	0-14111 0	 15	111	17	0-1uiii   0	18	 19	Tigrit 11	0-1u11 0	
7:15 AM	22	64	9	0	9	10	4	0	6	129	13	0	25	35	6	0	
7:30 AM	69	78	14	0	17	37	14	0	13	125	41	0	12	53	4	0	
7:45 AM	61	158	16	0	20	55	7	0	19	171	71	0	13	58	10	0	
8:00 AM	38	137	16	0	14	39	. 9	0	26	128	16	0	19	32	18	0	
8:15 AM	7	98	18	0	4	12	3	0	21	127	6	0	28	7	16	0	
8:30 AM	5	100	20	0	3	7	3	0	25	168	7	0	41	12	23	0	
8:45 AM	8	135	28	0	10	6	1	0	14	149	8	0	31	8	24	0	
9:00 AM	9	117	19	0	1	8	4	0	19	121	2	0	28	6	17	0	
9:15 AM	4	93	24	0	4	2	3	0	22	156	8	0	24	4	26	0	
9:30 AM	7	97	14	0	5	6	0	0	23	123	9	0	25	11	25	0	
9:45 AM	11	129	16	0	9	6	8	0	22	122	5	0	24	11	20	0	
10:00 AM	10	117	21	0	4	7	5	0	22	131	4	0	19	6	17	0	
2:00 PM	6	142	31	0	5	10	2	0	27	146	10	0	40	11	19	0	
2:15 PM	15	127	26	0	7	11	7	0	35	173	9	0	22	8	12	0	
2:30 PM	16	153	31	0	5	6	6	0	30	140	11	0	23	19	12	0	
2:45 PM	26	173	18	0	5	13	3	0	40	149	11	0	28	27	19	0	
3:00 PM	35	162	24	0	22	24	17	0	25	156	17	0	19	13	16	0	
3:15 PM	26	158	26	0	23	17	14	0	11	163	17	0	35	32	21	0	
3:30 PM	18	126	18	0	13	21	5	0	27	144	22	0	29	18	18	0	
3:45 PM	8	157	24	0	20	19	9	0	32	141	15	0	33	27	26	0	
4:00 PM	22	165	31	0	5	16	8	0	28	155	14	0	27	30	17	0	
4:15 PM	15	202	21	0	12	21	14	0	38	167	14	0	33	13	27	0	
4:30 PM	12	241	29	0	12	17	3	0	47	205	4	0	19	9	16	0	
4:45 PM	9	198	32	0	11	13	5	0	27	161	10	0	20	24	9	0	
5:00 PM	18	219	19	0	11	19	3	0	57	169	8	0	25	12	22	0	
5:15 PM	9	213	34	0	20	11	7	0	29	159	9	0	36	10	15	0	
5:30 PM	7	181	27	0	10	14	3	0	33	178	14	0	35	11	16	0	
5:45 PM	10	206	34	0	0	9	4	0	29	142	16	0	31	13	16	0	
6:00 PM	8	152	29	0	9	12	10	0	33	156	5	0	33	6	21	0	
6:15 PM	7	139	19	0	4	10	2		29	141	7	0	35	9	20	0	
6:30 PM	12	129	22	0	12	7	3	0	30	142	6	0	28	6	15	0	
6:45 PM	20	140	19	0	10	6	1	0	23	130	23	0	25	18	8	0	
7:00 PM	19	101	24	0	11	17	7	0	26	104	24	0	30	29	14	0	

#### Study Name BOSTON POST RD AT HOMMOCKS RD (297650/298442/298569) Start Date 03-10-2016 Start Time 7:00 AM Site Code

Trucks

TUCK	Southbound St. Westbound St.									1.01		Eastbound St.				
		Southbo				Westbo				Northbo	ound St.		Eastbound St. Eastbound			
												. <b>T</b>				
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	<u> </u>	U-Turn	Left	Thru	<b>U</b>	U-Turn
7:00 AM 7:15 AM	0	0	0		0	0 0	0 0		0	6 4	0	0	1	0 0	1 0	0
	1	3	1	0	0				0		0	0	0			0
7:30 AM	0	4 7	0		0	0	0		1	1	0	0	0	0	0	0
7:45 AM	0	-	0	-	0	0	0	-	1	3	2	0	0	0	0	0
8:00 AM	0	6	0		0	0	0		0	8	1	0	0	3	0	0
8:15 AM	1	4	0		0	0	0		3	4	0	0	1	0	1	0
8:30 AM	1	3	0		0	0	0		0	4	0	0	0	1	1	0
8:45 AM	0	3	2		0	0	0		1	3	0	0	1	0	0	0
9:00 AM	0	6	0	-	0	0	0	-	0	5	0	0	2	1	0	0
9:15 AM	0	5	0		0	0	0		1	7	0	0	1	0	1	0
9:30 AM	0	3	0	0	0	1	0		0	2	2	0	0	1	1	0
9:45 AM	0	5	1	0	0	2	1	0	1	5	0	0	1	2	0	0
10:00 AM	1	6	1	0	2	0	1	~	1	3	0	0	0	0	0	0
2:00 PM	0	4	•	°,	0	0	0	-	0	2	0	0	1	•	1	0
2:15 PM	0	4	0		1	0	0		1	3	0	0	0	0	0	0
2:30 PM	0	2	0	-	0	0	0		0	5	1	0	0	0	1	0
2:45 PM	0	4	0		0	1	0		1	3	1	0	0	0	2	0
3:00 PM	0	2	0	0	0	0	0	-	0	3	0	0	1	0	1	0
3:15 PM	0	4	3	-	0	0	0	-	0	2	0	0	0	0	1	0
3:30 PM	0	1	1	0	0	0	1	0	2	1	1	0	0	0	0	0
3:45 PM	0	4	0	-	1	0	0	-	0	3	1	0	0	0	1	0
4:00 PM	0	3	0		0	0	0		1	4	0	0	0	0	0	0
4:15 PM	0	11	0		0	0	0		0	2	0	0	1	0	0	0
4:30 PM	0	6	1	0	0	0	0	-	0	5	0	0	0	0	1	0
4:45 PM	0	9	0		0	0	0		0	0	0	0	0	0	0	0
5:00 PM	0	4	1	0	0	1	0		0	2	0	0	0	1	0	0
5:15 PM	0	2	1	0	0	0	0		0	3	0	0	0	0	0	0
5:30 PM	0	2	0	-	0	0	0		0	0	0	0	0	0	0	0
5:45 PM	0	1	0	-	0	0	0		0	2	0	0	0	0	1	0
6:00 PM	0	0	0	-	0	0	0		1	0	0	0	0	0	0	0
6:15 PM	0	2	0	-	0	0	0		0	0	0	0	0	0	0	0
6:30 PM	0	0	0	-	0	0	0		0	0	0	0	0	0	0	0
6:45 PM	1	0	0	-	0	0	0		0	1	0	0	0	1	0	0
7:00 PM	0	0	0	0	1	2	0	0	0	1	1	0	0	0	1	0

#### Study Name BOSTON POST RD AT HOMMOCKS RD (297650/298442/298569) Start Date 03-10-2016 Start Time 7:00 AM Site Code

Buses

Duse	Southbound St.															
	S					Westbo				Northbo				Eastbo		
		Southb	-			West				North	bound			Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	1	1	0	0	0	0	0	0	2	0	0	3	0	0	0
7:15 AM	0	1	0	0	0	0	0	0	0	3	0	0	1	0	2	0
7:30 AM	3	1	1	0	0	1	1	0	0	2	0	0	0	1	0	0
7:45 AM	3	3	0	0	1	2	0	0	0	3	1	0	1	0	0	0
8:00 AM	1	2	0	0	0	3	1	0	3	0	0	0	1	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	3	0	0	2	0	0	0
8:30 AM	0	1	3	0	0	0	0	0	0	1	0	0	0	0	0	0
8:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:45 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0
2:30 PM	0	1	0	0	0	0	0	0	0	4	0	0	0	4	0	0
2:45 PM	0	3	0	0	0	1	0	0	0	1	1	0	0	1	0	0
3:00 PM	1	3	0	0	1	0	3	0	0	0	1	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0	3	1	0	1	0	1	0
3:30 PM	0	1	0	0	0	1	0	0	0	1	0	0	1	0	2	0
3:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0
4:00 PM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	1	0
4:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	1	0	1	0
4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	3	0	0	0	0	0	0	0	0	0	0	1	0	0	0
5:00 PM	0	3	0	0	0	0	0	0	0	0	0	0	1	0	0	0
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

# Study Name BOSTON POST RD AT HOMMOCKS RD (297650/298442/298569) Start Date 03-10-2016 Start Time 7:00 AM Site Code

# Pedestrians

T cuestitans	Southbo Southb		Westbo Westb		Northbo Northb		Eastbou Eastbo	
Start Time	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW
7:00 AM	0	1	0	1	0	1	1	1
7:15 AM	0	2	2	1	1	0	2	1
7:30 AM	1	52	3	0	22	1	0	10
7:45 AM	1	84	5	0	15	0	2	14
8:00 AM	0	8	0	4	4	0	0	1
8:15 AM	3	1	1	0	4	4	2	3
8:30 AM	0	1	1	1	1	0	1	5
8:45 AM	1	0	1	0	0	0	2	0
9:00 AM	0	2	1	1	2	1	0	3
9:15 AM	0	0	1	2	1	2	0	2
9:30 AM	1	4	0	1	1	0	1	1
9:45 AM	1	0	0	0	0	1	0	2
10:00 AM	2	0	2	0	1	1	5	0
2:00 PM	1	0	0	0	0	0	3	1
2:15 PM	1	0	2	0	0	0	1	1
2:30 PM	4	5	1	1	1	0	8	4
2:45 PM	2	16	1	1	0	1	24	3
3:00 PM	84	6	1	62	0	63	55	5
3:15 PM	16	4	1	8	0	15	16	0
3:30 PM	7	7	1	4	1	5	7	2
3:45 PM	5	4	1	2	4	0	2	3
4:00 PM	6	2	1	5	1	1	3	4
4:15 PM	2	0	0	0	2	1	6	2
4:30 PM	1	1	1	1	0	1	1	1
4:45 PM	1	1	4	1	4	5	2	4
5:00 PM	0	3	0	0	1	1	3	3
5:15 PM	1	5	0	5	0	0	3	1
5:30 PM	0	1	1	0	0	0	2	2
5:45 PM	0	2	0	0	0	0	0	0
6:00 PM	2	0	1	0	0	0	2	1
6:15 PM	0	0	0	0	0	1	0	1
6:30 PM	0	0	1	0	1	0	0	2
6:45 PM	0	1	1	1	0	1	3	1
7:00 PM	0	0	0	1	0	0	1	1

#### Study Name BOSTON POST RD AT HOMMOCKS RD (297650/298442/298569) Start Date 03-10-2016 Start Time 7:00 AM

## **Bicycles on Road**

,		Southbo Southl				Westbo Westb				Northbo Northl				Eastbou Eastbo		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
7:45 AM	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0
8:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0
9:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	2	0	0	0	0	0	0	0	2	1	0	0	0	0	0
2:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:45 PM	0	3	0	0	0	0	0	0	0	0	1	0	0	0	0	0
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
6:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

All Vehic	les															
		Rou	te 1			Westbo	und St.			Rou	te 1			Eastbo	und St.	
		South	bound			West	bound			North	bound			Eastb	ound	
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	16	171	37	0	19	12	9	0	24	174	22	0	46	14	23	0
11:15 AM	21	196	45	0	14	27	14	0	31	172	25	0	35	27	14	0
11:30 AM	13	183	37	0	12	12	15	0	30	181	13	0	44	28	19	0
11:45 AM	9	186	32	0	11	9	9	0	27	187	11	0	31	18	26	0
12:00 PM	17	153	42	0	13	17	16	0	43	168	13	0	32	22	31	0
12:15 PM	13	172	36	0	8	16	8	0	34	191	25	0	40	9	25	0
12:30 PM	8	183	32	0	7	14	9	0	42	215	8	0	43	7	32	0
12:45 PM	12	156	51	0	25	13	12	0	34	181	10	0	33	13	18	0
1:00 PM	13	171	42	0	20	15	13	0	39	181	13	0	21	8	27	0
1:15 PM	14	168	38	0	15	11	5	0	31	207	9	0	43	18	22	0
1:30 PM	14	154	42	0	11	18	15	0	43	145	7	0	41	15	23	0
1:45 PM	16	168	26	0	13	9	9	0	37	170	12	0	43	6	28	0

Truck	s															
	S	Southbou Southb				Westbo Westb					ound St. bound			Eastbo Eastb	und St. ound	
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	2	0	0	1	0	0	0	0	4	0	0	0	0	0	0
11:15 AM	0	1	0	0	0	0	0	0	1	4	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0
11:45 AM	0	1	0	0	0	0	0	0	0	3	0	0	0	0	0	0
12:00 PM	0	1	0	0	0	0	0	0	0	2	0	0	0	0	0	0
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	1	0	0	0	0	0	0	0	2	0	0	0	0	2	0
12:45 PM	0	2	1	0	0	0	0	0	1	3	0	0	2	0	2	0
1:00 PM	1	2	0	0	0	0	0	0	0	2	1	0	0	0	0	0
1:15 PM	0	1	0	0	0	1	0	0	0	3	0	0	0	0	0	0
1:30 PM	0	6	1	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	1	1	0	0	0	0	0	0	2	0	0	3	1	0	0

Buses	5															
		Southbo South				Westbo Westb				Northbo Northl	ound St. bound			Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Pedestrians

	Rout Southb		Hommo Westb		Rout Northb		Weave Eastbo	
Start Time	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW
11:00 AM	2	0	0	1	0	1	4	3
11:15 AM	0	3	1	2	0	0	3	5
11:30 AM	0	1	1	2	1	0	1	7
11:45 AM	0	0	5	1	7	0	5	0
12:00 PM	3	0	1	3	2	0	6	1
12:15 PM	2	0	2	1	2	0	12	2
12:30 PM	0	0	4	7	1	2	2	3
12:45 PM	3	1	1	2	2	0	2	3
1:00 PM	3	2	5	3	2	1	7	3
1:15 PM	0	0	1	0	0	0	3	3
1:30 PM	3	2	0	0	1	2	1	5
1:45 PM	3	1	1	1	1	1	4	1

#### **Bicycles on Road**

		Southbo South	ound St. bound				ound St. Dound				ound St. bound			Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	6	0	0	0	0	0	0	0	1	0	0	0	0	0	0
11:15 AM	0	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	31	0	0	0	0	0	0	0	0	0	0	0	2	1	0
11:45 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
12:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	1	0	0	0	0	0	0	2	0	0	0	0	0	0	0
12:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0

#### **All Vehicles**

[		Southbo Southb				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	60	7	0	16	6	10	0	11	122	0	0	17	0	13	0
7:15 AM	0	126	11	0	13	14	10	0	14	138	0	0	20	0	14	0
7:30 AM	0	207	19	0	25	13	22	0	35	197	0	0	24	0	29	0
7:45 AM	0	207	18	0	20	14	8	0	35	217	0	0	27	0	22	0
8:00 AM	0	111	13	0	19	11	17	0	15	186	0	0	21	0	20	0
8:15 AM	0	104	14	0	19	26	18	0	10	168	0	0	21	0	19	0
8:30 AM	0	126	14	0	16	27	12	0	10	170	0	0	27	0	12	0
8:45 AM	0	145	11	0	22	18	14	0	9	167	0	0	27	0	18	0
9:00 AM	0	135	24	0	10	11	10	0	11	142	0	0	22	0	15	0
9:15 AM	0	111	21	0	19	5	16	0	14	163	0	0	21	0	15	0
9:30 AM	0	124	20	0	14	11	18	0	11	129	0	0	23	0	22	0
9:45 AM	0	156	19	0	15	11	17	0	9	144	0	0	17	0	17	0
10:00 AM	0	118	14	0	14	12	9	0	7	151	0	0	15	0	16	0
2:00 PM	0	130	18	0	15	8	12	0	14	149	0	0	16	0	25	0
2:15 PM	0	145	21	0	17	11	6	0	14	188	0	0	20	0	16	0
2:30 PM	0	197	37	0	30	11	12	0	18	156	0	0	14	0	25	0
2:45 PM	0	178	17	0	35	15	7	0	22	161	0	0	17	0	25	0
3:00 PM	0	157	16	0	25	11	15	0	38	204	0	0	17	0	30	0
3:15 PM	0	143	14	0	22	11	11	0	15	224	0	0	24	0	24	0
3:30 PM	0	172	22	0	16	12	17	0	25	184	0	0	18	0	24	0
3:45 PM	0	145	21	0	32	17	15	0	11	185	0	0	21	0	23	0
4:00 PM	0	211	16	0	20	7	8	0	24	171	0	0	27	0	24	0
4:15 PM	0	229	21	0	30	10	12	0	22	201	0	0	22	0	19	0
4:30 PM	0	234	35	0	23	5	18	0	18	203	0	0	25	0	20	0
4:45 PM	0	253	22	0	17	16	6	0	19	165	0	0	8	0	28	0
5:00 PM	0	214	26	0	24	13	9	0	16	155	0	0	21	0	32	0
5:15 PM	0	172	15	0	25	12	10	0	25	183	0	0	9	0	22	0
5:30 PM	0	187	22	0	25	10	12	0	21	188	0	0	21	0	28	0
5:45 PM	0	184	26	0	33	7	8	0	18	186	0	0	22	0	27	0
6:00 PM	0	155	21	0	24	7	10	0	19	169	0	0	17	0	29	0
6:15 PM	0	154	28	0	9	17	13	0	16	157	0	0	23	0	27	0
6:30 PM	0	159	23	0	16	11	10	0	17	152	0	0	17	0	21	0
6:45 PM	0	122	14	0	17	13	13	0	17	145	0	0	26	0	20	0
7:00 PM	0	122	14	0	11	10	9	0	16	156	0	0	12	0	22	0

#### Trucks

	:	Southbo South	ound St. bound			Westbo Westb				Northbo Northl				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	2	1	0	0	0	1	0	0	4	0	0	1	0	0	0
7:15 AM	0	3	0	0	0	0	0	0	0	4	0	0	0	0	0	0
7:30 AM	0	9	0	0	0	0	0	0	0	1	0	0	0	0	0	0
7:45 AM	0	3	0	0	1	0	0	0	2	3	0	0	0	0	1	0
8:00 AM	0	4	0	0	0	0	1	0	0	4	0	0	1	0	0	0
8:15 AM	0	1	0	0	0	0	0	0	1	5	0	0	0	0	0	0
8:30 AM	0	4	0	0	0	0	0	0	0	1	0	0	0	0	0	0
8:45 AM	0	7	0	0	0	0	0	0	0	5	0	0	0	0	0	0
9:00 AM	0	3	0	0	1	0	0	0	0	9	0	0	0	0	0	0
9:15 AM	0	1	0	0	0	0	1	0	0	7	0	0	0	0	0	0
9:30 AM	0	5	0	0	0	0	0	0	1	5	0	0	0	0	1	0
9:45 AM	0	8	0	0	0	0	0	0	0	6	0	0	0	0	0	0
10:00 AM	0	5	0		1	0	0	0	0	4	0	0	0	0	0	0
2:00 PM	0	3	0	0	0	0	0	0	0	3	0	0	1	0	0	0
2:15 PM	0	2	0	0	0	1	0	0	0	3	0	0	0	0	0	0
2:30 PM	0	1	0	0	0	1	1	0	0	4	0	0	0	0	0	0
2:45 PM	0	2	1	0	0	0	0	0	0	6	0	0	0	0	1	0
3:00 PM	0	8	0		1	0	0	0	1	3	0	0	0	0	0	0
3:15 PM	0	3	0	-	0	0	1	0	0	2	0	0	1	0	0	0
3:30 PM	0	3	0	-	0	0	0	0	0	1	0	0	0	0	0	0
3:45 PM	0	3	0	-	0	0	0	0	0	2	0	0	0	0	0	0
4:00 PM	0	10	2	0	0	0	0	0	0	4	0	0	0	0	0	0
4:15 PM	0	7	0		0	0	0	0	0	3	0	0	0	0	1	0
4:30 PM	0	8	1	0	0	0	0	0	0	4	0	0	0	0	0	0
4:45 PM	0	6	0	-	1	1	0	0	0	2	0	0	0	0	0	0
5:00 PM	0	4	1	0	0	0	1	0	0	2	0	0	0	0	0	0
5:15 PM	0	1	0	-	0	0	0	0	1	2	0	0	0	0	0	0
5:30 PM	0	0	0	-	0	0	0	0	0	1	0	0	0	0	0	0
5:45 PM	0	1	0	-	0	0	0	0	0	1	0	0	0	0	0	0
6:00 PM	0	2	0	•	0	0	0	0	0	0	0	0	0	0	0	0
6:15 PM	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Buses	s															
		Southbo				Westbo					ound St.			Eastbo		
		South				West				North				Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	1	0	0	1	0	0	0	1	3	0	0	0	0	0	0
7:15 AM	0	2	1	0	0	0	0	0	0	4	0	0	0	0	0	0
7:30 AM	0	4	0	0	2	0	0	0	1	3	0	0	1	0	2	0
7:45 AM	0	1	0	0	0	1	0	0	1	3	0	0	1	0	0	0
8:00 AM	0	2	1	0	0	1	0	0	1	1	0	0	0	0	0	0
8:15 AM	0	3	0	0	0	0	1	0	0	4	0	0	1	0	0	0
8:30 AM	0	2	0	0	0	2	0	0	0	2	0	0	0	0	0	0
8:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
9:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	1	0	0	0	0	0		0	0	0	0	0	0	0	0
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
2:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
2:30 PM	0	2	0	0	1	0	1	0	0	1	0	0	1	0	0	0
2:45 PM	0	2	0	-	0	0	0	0	0	5	0	0	0	0	1	0
3:00 PM	0	3	0	-	1	0	1	0	1	4	0	0	1	0	0	0
3:15 PM	0	1	0	-	0	1	0	0	0	1	0	0	1	0	0	0
3:30 PM	0	0	0	0	0	0	1	0	0	5	0	0	0	0	0	0
3:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
4:00 PM	0	2	1	0	0	1	0	0	0	1	0	0	0	0	0	0
4:15 PM	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	3	0	0	0	0	0	0	0	1	0	0	0	0	0	0
5:00 PM	0	2	0	0	0	0	0	0	1	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
6:30 PM	0	0	0	-	0	0	0	0	1	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0

Pedes	trians							
	Southbo		Westbo		Northbo		Eastbou	
	Southb		West		Northb	1	Eastbo	
Start Time			Peds CCW		Peds CCW	Peds CW	Peds CCW	Peds CW
7:00 AM	0	1	2	2	0	1	2	1
7:15 AM	0	0	0	2	1	0	1	1
7:30 AM	0	2		11	4	1	2	2
7:45 AM	0	2		2	1	0	4	9
8:00 AM	3	0	0	3	0	0	16	4
8:15 AM	2	1	0	3	0	5	21	2
8:30 AM	19	3	1	0	2	1	7	13
8:45 AM	1	4		2	1	0	2	1
9:00 AM	2	1	1	1	5	1	0	3
9:15 AM 9:30 AM	1	2	1	2 0	1	3	0	2
9:30 AM 9:45 AM	0	0 1	2	0	1 0	0 0	0 6	1 2
9.45 AM 10:00 AM	0	2		2	0	0	5	3
2:00 PM	4	2		1	2	0	4	2
2:00 PM	4	2	2	0	2	0	4	2
2:30 PM	7	5	2	5	3	1	69	4
2:45 PM	1	4	1	5	1	4	20	4
3:00 PM	10	12	14	11	1	4	20	24
3:15 PM	10	0	41	2	6	2	17	17
3:30 PM	4	1	4	0	2	4	7	2
3:45 PM	0	10	10	0	- 1	2	0	6
4:00 PM	4	0	5	3	0	1	3	5
4:15 PM	2	0	6	0	3	2	1	0
4:30 PM	0	1	3	0	1	0	6	5
4:45 PM	0	4	0	0	1	2	6	0
5:00 PM	1	1	0	0	0	5	3	1
5:15 PM	0	1	1	0	1	3	2	4
5:30 PM	0	0	3	0	3	0	2	1
5:45 PM	2	2	4	0	0	1	3	0
6:00 PM	0	1	0	1	0	3	0	4
6:15 PM	1	1	0	3	0	2	1	1
6:30 PM	0	1	0	2	2	3	1	2
6:45 PM	1	1	1	0	0	0	0	0
7:00 PM	1	2	3	2	1	2	4	3

#### Bicycles

2.090		Southbo Southl				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
9:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
2:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 PM	0	2	0	0	0	1	0	0	0	0	0	0	0	0	0	0
3:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
6:00 PM	0	1	0	0	0	0	0	0	0	2	0	0	0	0	0	0
6:15 PM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

#### **Total Vehicles**

		Route 1 Southbound				ld Bosto Westt		Rd		Rou Northl				Richb Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	224	20	0	24	7	12	0	18	170	0	0	20	0	32	0
11:15 AM	0	202	14	0	20	11	13	0	26	183	0	0	26	0	32	0
11:30 AM	0	193	15	0	22	14	20	0	21	193	0	0	19	1	29	0
11:45 AM	0	170	29	0	29	13	14	0	21	210	0	0	27	0	28	0
12:00 PM	0	173	28	0	21	12	22	0	21	171	0	0	35	0	27	0
12:15 PM	0	194	24	0	21	10	19	0	20	213	0	0	26	0	34	0
12:30 PM	0	155	17	0	30	16	11	0	20	206	0	0	26	0	29	0
12:45 PM	0	179	27	0	21	16	19	0	24	206	0	0	24	0	35	0
1:00 PM	0	166	18	0	21	10	15	0	21	190	0	0	21	0	26	0
1:15 PM	0	169	19	0	24	18	12	0	18	205	0	0	25	0	36	0
1:30 PM	0	204	18	0	18	17	19	0	27	181	0	0	24	0	34	0
1:45 PM	0	164	17	0	21	14	15	0	28	160	0	0	14	0	27	0

#### Trucks

		Southbo South	ound St. bound			Westbo Westb				Northbo North	ound St. bound			Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
11:15 AM	0	1	0	0	0	0	0	0	0	6	0	0	1	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0	0
11:45 AM	0	2	0	0	0	0	0	0	0	2	0	0	0	0	0	0
12:00 PM	0	1	1	0	0	1	1	0	0	1	0	0	0	0	1	0
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	1	0	0	0	1	0	0	0	2	0	0	0	0	0	0
12:45 PM	0	2	0	0	1	0	0	0	0	3	0	0	0	0	0	0
1:00 PM	0	2	0	0	0	0	0	0	0	2	0	0	0	0	0	0
1:15 PM	0	1	1	0	0	0	0	0	0	2	0	0	0	0	1	0
1:30 PM	0	5	0	0	0	0	0	0	0	0	0	0	1	0	2	0
1:45 PM	0	1	0	0	0	0	0	0	0	4	0	0	0	0	0	0

Buses	5															
		Southbo South	ound St. bound			Westbo Westb	ound St. bound			Northbo Northl				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Pedestrians

	Rout Southb		Old Bostor Westb		Rout Northb		Richbe Eastbe	
Start Time	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW
11:00 AM	0	2	1	1	0	0	1	4
11:15 AM	0	4	1	3	4	1	4	9
11:30 AM	0	0	3	5	1	0	1	7
11:45 AM	4	1	3	3	1	0	2	1
12:00 PM	0	0	2	0	0	0	2	2
12:15 PM	3	1	1	3	2	0	4	1
12:30 PM	1	0	2	4	0	1	6	1
12:45 PM	2	0	0	1	0	4	6	0
1:00 PM	0	2	1	5	0	3	7	8
1:15 PM	5	0	2	0	4	1	3	8
1:30 PM	0	2	0	3	0	4	1	1
1:45 PM	0	0	0	0	0	0	3	5

Bicycles on	Road															
		Southbo South				Westbo Westb				Northbo North				Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	10	0	0	0	0	0	0	0	1	0	0	0	0	0	0
11:15 AM	0	42	0	0	0	0	0	0	0	1	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
11:45 AM	0	0	0	0	0	0	1	0	0	2	0	0	0	0	0	0
12:00 PM	0	3	0	0	0	0	0	0	0	1	0	0	0	0	0	0
12:15 PM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0
12:30 PM	0	1	1	0	0	0	0	0	0	1	0	0	0	0	0	0
12:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
1:30 PM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0

#### **All Vehicles**

		Southbound St. Southbound				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	16	52	2	0	9	12	17	0	5	113	11	0	1	3	5	0
7:15 AM	26	121	1	0	15	7	19	0	0	131	18	0	1	8	9	0
7:30 AM	61	182	1	0	13	15	50	0	6	143	20	0	4	13	32	0
7:45 AM	86	161	2	0	28	19	33	0	9	119	32	0	8	43	27	0
8:00 AM	75	112	7	0	16	19	46	0	5	164	29	0	4	25	10	0
8:15 AM	43	99	3	0	21	5	30	0	2	142	30	0	11	12	3	0
8:30 AM	81	121	2	0	25	8	40	0	1	144	34	0	5	16	5	0
8:45 AM	79	112	2	0	16	8	37	0	3	130	28	0	9	8	9	0
9:00 AM	52	129	3	0	16	3	30	0	4	123	27	0	5	7	8	0
9:15 AM	32	120	2	0	9	4	21	0	3	142	18	0	4	1	7	0
9:30 AM	21	104	3	0	8	2	18	0	4	130	23	0	3	8	5	0
9:45 AM	28	143	5	0	7	2	25	0	4	129	27	0	3	2	9	0
10:00 AM	21	107	0	0	14	0	20	0	6	135	25	0	2	3	3	0
2:00 PM	39	133	6	0	16	5	16	0	7	132	9	0	3	5	7	0
2:15 PM	37	141	3	0	20	3	17	0	7	142	14	0	3	8	6	0
2:30 PM	25	125	5	0	14	4	31	0	6	137	25	0	9	6	9	0
2:45 PM	41	140	4	0	17	7	22	0	6	144	27	0	2	7	14	0
3:00 PM	47	148	3	0	15	16	32	0	11	178	33	0	3	7	8	0
3:15 PM	40	125	4	0	8	10	31	0	13	169	36	0	4	1	6	0
3:30 PM	42	135	3	0	13	14	33	0	4	145	28	0	1	5	4	0
3:45 PM	55	150	7	0	13	17	33	0	2	159	27	0	5	6	1	0
4:00 PM	38	159	6	0	21	22	46	0	4	155	27	0	1	10	6	0
4:15 PM	32	224	6	0	16	22	36	0	4	167	36	0	4	3	2	0
4:30 PM	41	201	5	0	20	15	30	0	5	169	28	0	4	3	13	0
4:45 PM	44	221	6	0	12	13	32	0	1	148	24	0	4	14	11	0
5:00 PM	37	216	1	0	9	19	38	0	3	149	32	0	1	4	3	0
5:15 PM	42	185	3	0	10	6	32	0	2	154	29	0	3	6	5	0
5:30 PM	31	182	4	0	8	5	18	0	2	172	20	0	2	5	1	0
5:45 PM	45	204	2	0	15	6	30	0	5	150	30	0	4	5	3	0
6:00 PM	42	145	5	0	7	7	26	0	11	140	17	0	2	10	8	0
6:15 PM	45	157	3	0	13	3	26	0	1	173	26	0	2	9	9	0
6:30 PM	45	146	4	0	8	2	21	0	2	148	19	0	2	12	4	0
6:45 PM	26	142	2	0	7	6	18	0	2	122	40	0	2	7	5	0
7:00 PM	36	116	3	0	11	1	21	0	5	133	33	0	5	10	3	0

#### Trucks

	-	Southbo Southl				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	1	2	0	0	0	0	0	0	0	6	0	0	0	0	0	0
7:15 AM	2	4	0	0	1	0	0	0	0	5	0	0	0	0	0	0
7:30 AM	4	6	0	0	1	0	1	0	0	1	0	0	0	0	0	0
7:45 AM	3	2	0	0	0	0	1	0	0	3	0	0	0	0	0	0
8:00 AM	1	3	1	0	0	0	0	0	0	6	1	0	0	0	0	0
8:15 AM	3	2	0	0	1	0	0	0	0	6	0	0	0	0	0	0
8:30 AM	0	2	0	0	0	0	0	0	0	3	0	0	0	0	0	0
8:45 AM	1	6	0	0	0	0	1	0	0	5	0	0	0	0	0	0
9:00 AM	2	4	0	0	0	0	0	0	1	1	3	0	0	0	0	0
9:15 AM	2	4	0	0	1	0	1	0	0	8	0	0	0	0	0	0
9:30 AM	3	4	1	0	1	1	1	0	0	3	1	0	0	0	0	0
9:45 AM	2	7	0	0	0	0	1	0	0	5	1	0	0	0	0	0
10:00 AM	0	5	0	0	2	0	1	0	0	4	3	0	0	0	0	0
2:00 PM	3	3	0	0	0	1	0	0	0	3	0	0	0	0	0	0
2:15 PM	0	2	0	0	0	0	3	0	1	4	0	0	0	0	0	0
2:30 PM	0	3	0	0	1	0	1	0	0	4	0	0	0	0	0	0
2:45 PM	1	2	0	0	0	0	1	0	0	6	0	0	0	0	1	0
3:00 PM	0	2	0	0	2	0	0	0	0	1	1	0	0	0	0	0
3:15 PM	1	2	0	0	0	0	1	0	0	1	2	0	0	0	0	0
3:30 PM	0	1	0	0	3	0	0	0	0	1	0	0	0	0	0	0
3:45 PM	0	6	0	0	0	0	1	0	0	3	0	0	0	0	0	0
4:00 PM	0	3	0	0	0	0	0	0	1	2	2	0	0	0	0	0
4:15 PM	0	16	0	0	0	0	0	0	0	2	0	0	0	0	0	0
4:30 PM	1	7	0	0	0	0	0	0	0	7	0	0	0	0	0	0
4:45 PM	1	8	0	0	1	0	6	0	0	0	0	0	0	0	0	0
5:00 PM	1	5	0	0	0	0	0	0	0	3	0	0	0	0	0	0
5:15 PM	0	2	0	0	0	0	0	0	0	2	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
5:45 PM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
6:15 PM	1	1	0	0	0	0	3	0	0	0	0	0	0	1	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0

#### Buses

Dusta		Southbo Southb				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	0	0	0	2	1	2	0	0	1	4	0	0	1	0	0
7:15 AM	2	1	0	0	1	0	2	0	0	0	3	0	0	0	0	0
7:30 AM	6	2	0	0	1	0	5	0	0	0	4	0	0	0	0	0
7:45 AM	4	5	0	0	1	1	2	0	0	1	2	0	1	1	0	0
8:00 AM	2	2	0	0	1	1	4	0	0	0	1	0	0	0	0	0
8:15 AM	3	0	0	0	2	0	3	0	0	2	4	0	0	0	0	0
8:30 AM	2	0	0	0	3	0	1	0	0	1	1	0	0	0	0	0
8:45 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 PM	1	3	0	0	0	0	0	0	0	2	0	0	1	0	0	0
2:45 PM	1	2	0	0	0	0	0	0	0	2	4	0	0	0	0	0
3:00 PM	3	4	0	0	1	1	0	0	0	5	1	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	1	0	0	3	1	0	0	0	0	0
3:30 PM	3	1	0	0	1	0	1	0	0	1	4	0	0	1	1	0
3:45 PM	4	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
4:00 PM	0	1	0	0	2	3	2	0	0	0	2	0	0	0	0	0
4:15 PM	1	0	1	0	0	3	1	0	0	0	0	0	0	0	0	0
4:30 PM	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
4:45 PM	0	3	0	0	1	0	0	0	0	0	1	0	0	0	0	0
5:00 PM	0	2	0	0	0	2	4	0	0	0	0	0	0	0	0	0
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
5:45 PM	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Pedestrians

Pedes								
	Southbo		Westbou		Northbo		Eastbou	
	Southb		Westb		Northb	-	Eastbo	
Start Time	Peds CCW	Peds CW						
7:00 AM	1	0	1	0	0	1	1	1
7:15 AM	0	0	0	0	1	0	9	0
7:30 AM	0	0	1	4	0	1	1	1
7:45 AM	0	0	1	0	2	0	3	2
8:00 AM	1	0	1	1	0	0	0	2
8:15 AM	0	0	0	0	1	0	2	0
8:30 AM	0	1	4	0	0	0	1	0
8:45 AM	0	1	0	1	0	2	1	1
9:00 AM	2	0	2	1	1	0	0	0
9:15 AM	0	0	0	1	1	0	0	2
9:30 AM	0	0	1	0	0	0	0	1
9:45 AM	0	1	1	1	0	0	0	1
10:00 AM	1	0	0	0	0	0	3	1
2:00 PM	2	0	0	0	0	0	0	1
2:15 PM	0	0	0	0	0	2	0	2
2:30 PM	0	2	0	0	3	0	0	7
2:45 PM	1	3	0	1	7	1	0	8
3:00 PM	0	1	1	0	4	0	0	0
3:15 PM	1	6	19	2	4	0	1	6
3:30 PM	3	0	7	1	1	0	0	1
3:45 PM	1	0	0	3	0	3	2	1
4:00 PM	0	0	0	1	2	1	0	0
4:15 PM	2	1	0	1	0	0	5	1
4:30 PM	1	0	2	0	2	0	1	1
4:45 PM	1	0	1	1	0	1	0	1
5:00 PM	0	1	0	0	0	0	1	0
5:15 PM	1	0	0	0	1	1	1	0
5:30 PM	0	3	0	1	0	1	0	3
5:45 PM	0	1	0	1	0	0	0	0
6:00 PM	1	0	0	0	0	0	0	0
6:15 PM	0	0	0	2	0	0	1	1
6:30 PM	0	0	0	0	0	0	2	0
6:45 PM	0	0	0	0	0	0	0	1
7:00 PM	0	0	0	0	0	0	1	2

#### **Bicycles on Road**

-		Southbo South				Westbo Westb				Northbo Northb				Eastbou Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 AM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0
9:45 AM	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

#### **All Vehicles**

		Rou South				Orient Westl				Rou Northl	ite 1 bound			Delano Eastb	,	
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	42	171	9	0	17	4	28	0	3	157	35	0	6	1	14	0
11:15 AM	45	194	5	0	9	5	31	0	8	166	31	0	6	4	12	0
11:30 AM	31	176	6	0	7	3	37	0	4	181	25	0	7	2	3	0
11:45 AM	29	147	6	0	12	3	42	0	7	208	29	0	7	3	8	0
12:00 PM	49	160	7	0	8	4	35	0	6	184	32	0	4	3	6	0
12:15 PM	38	147	6	0	17	9	35	0	7	197	24	0	3	2	13	0
12:30 PM	55	167	6	0	9	2	22	0	3	216	34	0	4	7	11	0
12:45 PM	42	172	7	0	6	3	31	0	3	184	35	0	8	4	9	0
1:00 PM	34	146	5	0	17	5	32	0	6	196	29	0	7	4	13	0
1:15 PM	32	153	4	0	8	10	23	0	5	179	25	0	6	7	8	0
1:30 PM	38	164	7	0	11	7	20	0	3	171	28	0	4	1	8	0
1:45 PM	34	183	3	0	9	2	25	0	5	163	19	0	10	10	7	0

# Study Name BOSTON POST RD AT DELANCEY AV/ORIENTA AVE (297632 / 298451) Start Date 03-12-2016 Start Time 11:00 AM Site Code

Trucks

		Southbo Southb				Westbo Westb				Northbo Northl	ound St. bound			Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	1	0	0	0	0	0	0	1	1	0	0	0	0	0	0
11:15 AM	1	0	0	0	0	0	1	0	0	3	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	1	0	0	4	0	0	1	1	0	0
11:45 AM	2	2	0	0	0	0	2	0	0	1	1	0	0	0	0	0
12:00 PM	1	3	1	0	0	0	1	0	0	2	0	0	0	0	0	0
12:15 PM	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	3	1	0	0	1	0	1	0	0	2	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0
1:00 PM	0	3	0	0	0	0	0	0	0	3	0	0	0	0	0	0
1:15 PM	0	3	0	0	0	0	0	0	0	2	0	0	0	0	0	0
1:30 PM	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0

# Study Name BOSTON POST RD AT DELANCEY AV/ORIENTA AVE (297632 / 298451) Start Date 03-12-2016 Start Time 11:00 AM Site Code

Buses

		Southbo Southl				Westbo Westb					ound St. bound			Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

# Study Name BOSTON POST RD AT DELANCEY AV/ORIENTA AVE (297632 / 298451 Start Date 03-12-2016 Start Time 11:00 AM Site Code

	Southbo Southb		Westbou Westb		Northbo Northb		Eastbou Eastbo	
Start Time	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW	Peds CCW	Peds CW
11:00 AM	0	0	0	1	0	0	2	5
11:15 AM	3	4	2	1	0	0	2	3
11:30 AM	0	0	0	1	0	0	1	0
11:45 AM	1	4	0	2	0	1	3	8
12:00 PM	0	0	1	0	1	0	2	0
12:15 PM	0	2	3	2	0	0	1	0
12:30 PM	5	0	3	0	2	0	0	2
12:45 PM	0	2	0	0	0	0	7	0
1:00 PM	0	0	3	2	2	0	2	1
1:15 PM	0	0	1	2	1	0	0	2
1:30 PM	0	1	1	1	0	0	1	2
1:45 PM	0	3	0	1	0	2	2	1

#### Pedestrians

## Study Name BOSTON POST RD AT DELANCEY AV/ORIENTA AVE (297632 / 298451) Start Date 03-12-2016 Start Time 11:00 AM Site Code

<b>Bicycles on</b>	Road															
		Southbo South				Westbo Westb				Northbo Northl				Eastbo Eastb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
11:00 AM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	44	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	3	0	0	0	0	0	0	0	1	0	0	0	0	0	0
12:15 PM	0	2	1	0	0	0	0	0	0	1	0	0	0	0	0	0
12:30 PM	0	2	0	0	0	0	0	0	0	1	0	0	0	0	0	0
12:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:30 PM	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0
1:45 PM	0	0	0	0	1	0	0	0	0	1	1	0	0	0	0	0

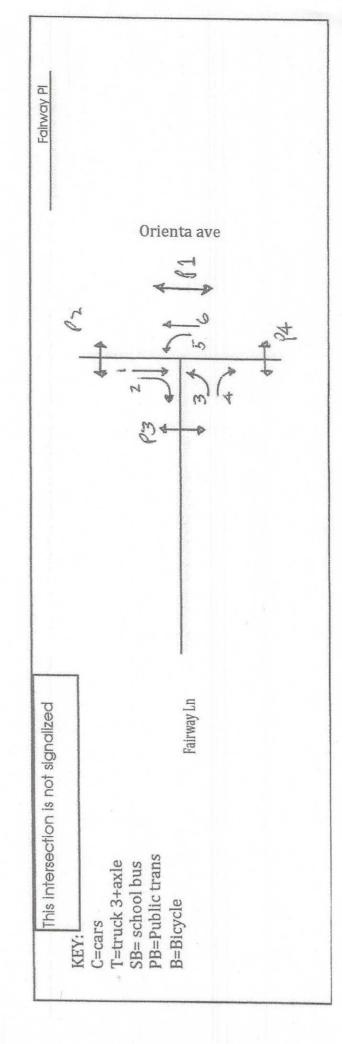
I HALL ST ONIE. WIAY 914-302-6326 fax-914-629-6815 cell TRAFFIC DATA COLLECTIONS ۲ **d** 4 N M 1 1 í Old Boston Post rd 1 1 ١ N 0, 0 1 ł d 1 AN 1 ١ í 1 N A 1 1 1 M 1 1 E 3 m ۱ ١ 1 1 ۱ ۱ 1 i 1 ł d a J 1 1 1 1 1 1 ı ( ١ i 1 1 3 s m ( l ļ 1 1 -1 ! 1 1 ł P4 M ſ ł С ( 1 1 U 1 m 1 , 1 1 1 1 1 ŀ . Field Tech: Weather: Cooper ave N 1 t 1 **d m** 1 ş ſ N DAY# 1 0 1 ) ) ( 1 N s m 5 5 62 I Į ( I 1 1 1 I 1 5 M 26 σ m N 0 N M C N 5 Mamaroneck Project 24 m ٢ 1 1 1 ١ 1 1 ٢ 1 L a a Į 1 1 ŧ ţ ſ ť Date: 3 /15/ 2016 Tuesday Location: Mamaroneck ny 1 I l 1 I ١ ŝ ( l 1 1 l l 1 -) 1 ŧ 1 This intersection is not signalized ١ 1 ł 1 ł ١ r C 7-7:15p Old Post In TIME 7:30 7:45 8:00 8:15 8:30 9:15 8:45 9:00 **Job Title:** PB=Public trans T=truck 3+axle SB= school bus B=Bicycle C=cars KEY:

Weather	Field Tech	DAY# 1
le: Mamaroneck Project	Location: Mamaroneck ny	Date: 3 /15/ 2016 Tuesday
Job Title: 1	Locatic	Date: 3

ch:  $\overline{\mathcal{A}}$ 

TRAFFIC DATA COLLECTIONS 914-302-6326 fax-914-629-6815 cell

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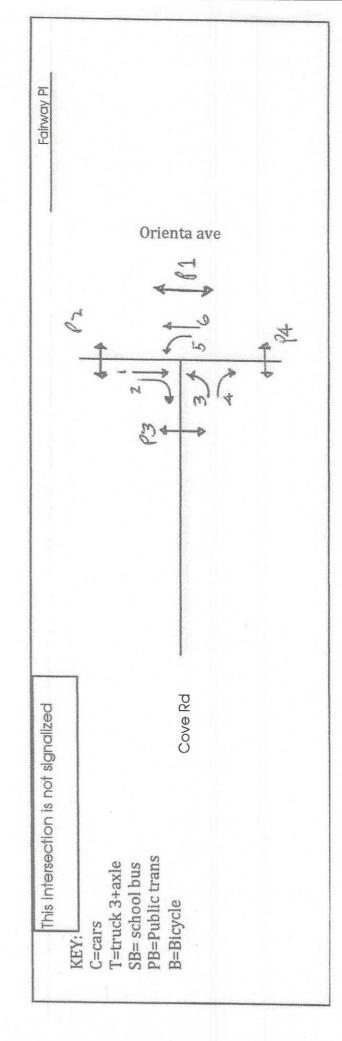
Job Title: Mamaroneck Project Weath Location: Mamaroneck ny Field T Date: 3 /15/ 2016 Tuesday DAY# 1

Weather : Field Tech:

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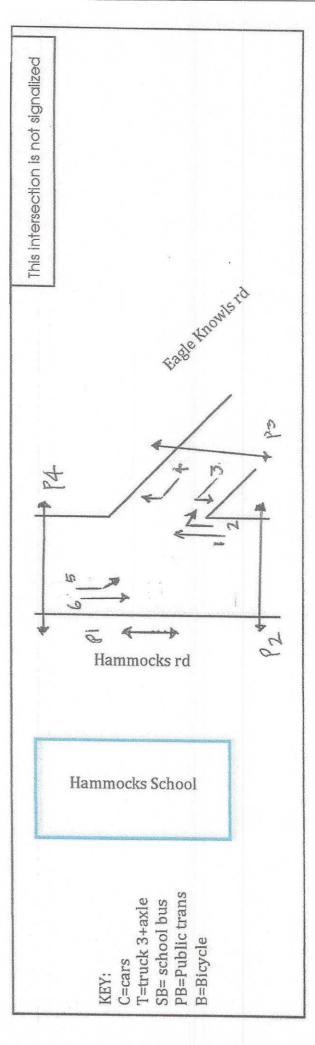


Weather	Field Tech	DAY# 1
Mamaroneck Project	Location: Mamaroneck ny	Date: 3 /15/ 2016 Tuesday
<b>Job Title:</b>	Location:	Date: 3 /1

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TRAFFIC DATA COLLECTIONS 914-302-6326 fax-914-629-6815 cell

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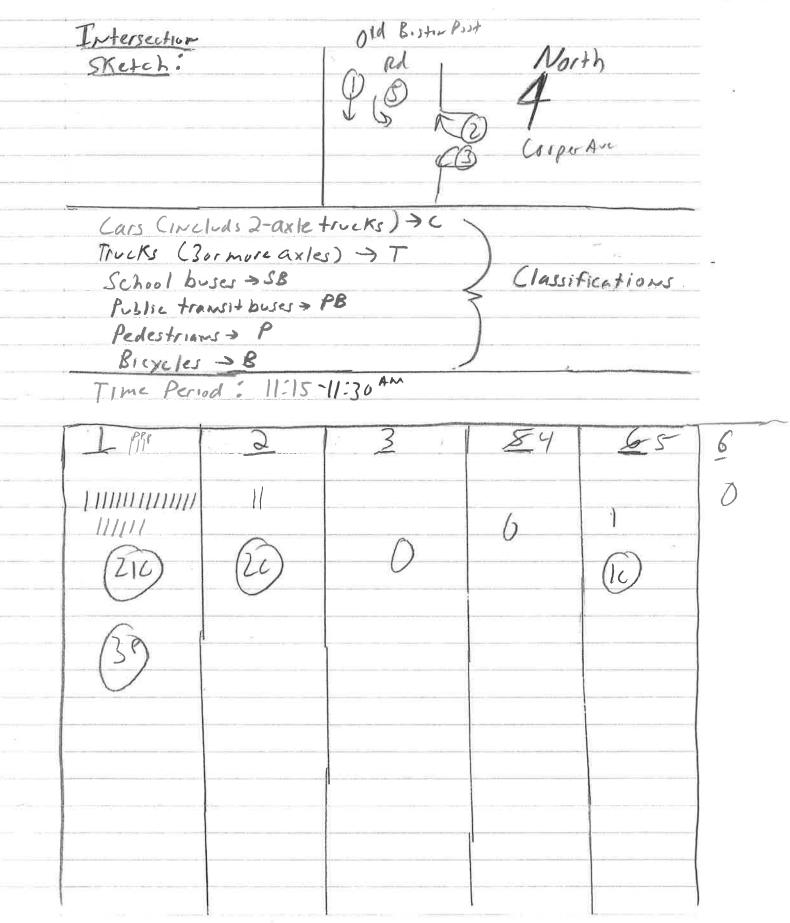
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Job Title: Mamaroneck Proj Location: Mamaroneck ny Date: 3 /16/ 2016 Wednesday		TIME	2- 2:15p	2:30	2:45	3:00	3:15	3:30	3:45	4:00	4:15	4:30	4:45	5:00	5:15	5:30	5:45	6:00	6:15	<u>Old Post In</u>
DLČ																				KEY: C=cars T=truck 3+axle SB= school bus PB=Public trans B=Bicycle

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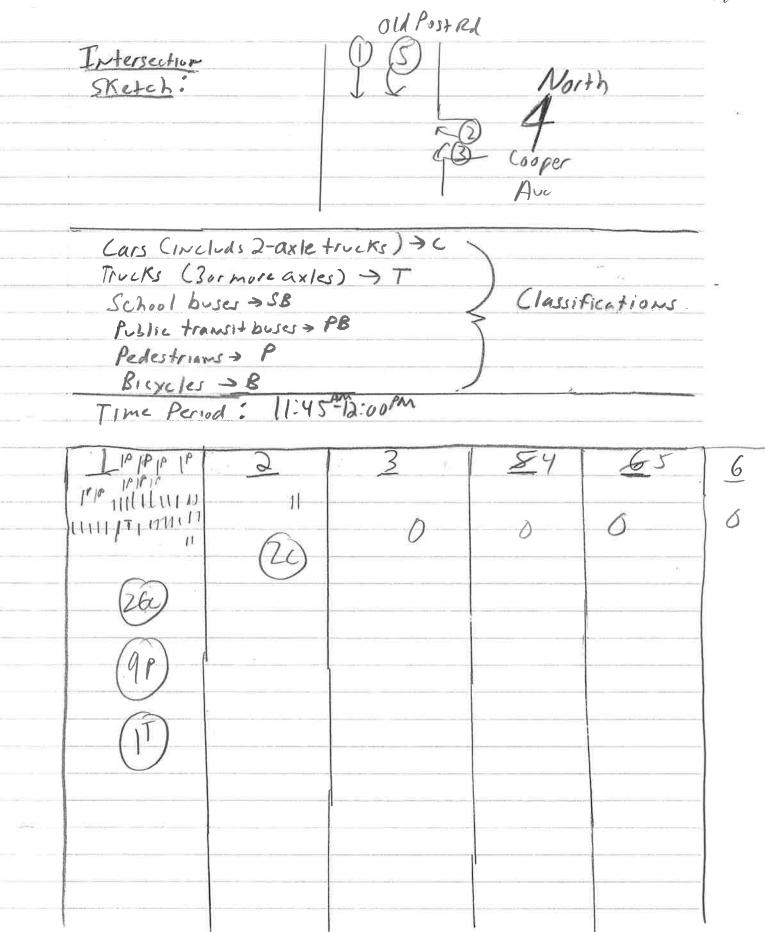
TRAFFIC DATA COLLEC IONS 914-302-6326 fax-914-629-6815 cell	5 6 PEDS	P         B         C         T         S         P         B         C         T         S         P         B         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P         P	. 4 2 4 0 0 0	· · · · · · · · ·		. 1 3 3 1 2 2 0 1	1 22 - 4 1 0 8 12	1	2		0				· · 3 - · 3 2 · · · 1 0 1 0	. 1 7 6 0 0 0 0	.34.5.7.7.20	0	· [ 3 - ] : [ 2 · ] - [ 0 0 0 0	This intersection is not signalized
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Walk ON Grass And On The Path Pedestrians 48 Old Biston Post Intersection Sketch: North Rd , O XFor All 15minute Increments About Cooper Au Everly Split Between Everly Split Between Gress and Path Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestrians > P Bicycles >B TIME Period: 11-11:15 AM 65 - IP 6 84 2 3 hinna 11111 0 te 7.06



3/8 OldPoster Intersection Sketch:  $(\mathbf{D})$ 5 North B (Inpes Au Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buser >SB Public transit buses > PB Pedestrians > P Bicycles -> B Time Period: 11:30-11:45 AM 84 3 15 6 2 L-MIHII ALL FULLING 0 1B IP 0 0 2 220 B

4/8



5/8 Old PistRl Intersection Sketch: North (5 Corper Ave Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period: 12-12:15pm 3 TP IP 2 65 84 6 1111111111 FILLLINI 1 11 6 20 C

6/8 OID Postad Ż Intersection Sketch: North K (3)_ Cooper Au Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) -> T Classifications. School buses > SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 12:15-12:30 pm 191911191 3 65 6 2 A 11111111 1111111 10 10 216

7/8 oldPistRL Intersection Sketch: North (oopen Au Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications School buses >SB Public transit buses > PB Pedestrians > P Bicycles -> B Time Period : 12:30 -12:45PM 6 65 3 84 IP IPIP 2 11111/1/11/11 1 1111/11/17

old PostRI Intersection Sketch: North 2) Cooper Au Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses + PB Pedestriams > P Bicycles -> B Time Period : 12:45-1:00PM LIPIPIPIB 2 3 84 6 11/11/11/11/1 Ò 0 ð 1 111 1 180 B

Intersection SKetch: North Orierth Grhss median OF E (1) OF E (1) Co Ge Ad pd Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses = PB Pedestriams > P Bisycles -> B Time Period : 11-11:15 Am 3 18 65 2 6 IP 16 10 10 10 12 10 12 10 16 10 C (ic) 10 Ċ 22 20, C (160) 50 C C C C C C C C Ĉ Ĉ 10 ſ C 6 ſ 1 4 10

2/8 Intersection Sketch: North orientia TFG (ars (includes 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buser & SB Public transit buses + PB Pedestriams > P Bicycles -> B Time Period : 11:15-11:30 AM 4 6 3 2  $|^{C}$ 10 С C C C C 1.0 C Ċ IC C C IC 10 IC C C 20 C 20 10 C C C 10 C Ċ IC 1C  $\int_{C}$ C C C Ĉ С ic ic C [90 10 C C C C C C C P C 204 P

3/8 Intersection North SKetch: orientia Grass Medion Auc 5°G C Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses = PB Pedestrians > P Bicycles >B Time Period : 11:30-11:45 AM 31010 2 \$4 \$5 6 C K C C C C С С IC C C С C С C ic C. C C P C C IC C С C ( CCBB C C C C 1C 500 C 220 C ZB C 1C C 1C C C Ċ

Intersection Sketch: North Orientia GV255 Moder POGTO Cars (Include 2-axle trucks) > c Trucks (Bormore axles) -> T Classifications. School buses -> SB Public transit buses > PB Pedestriams > P Bicycles > B Time Period: 11:45-12:00m 20 夏1 300 袤5 6 C C IC Ċ C C 1C C C C C 1C Ċ the IC Ċ C C IP C Zc 30 IC IP C 10 C C Ú 1C C IC C 1C IC 2º С C 1C 160 IC IC С с 140 C 1C C (C 16 C IC C C 10

Intersection Sketch: North Orientia Grass Median Ave 23 3 Onto Cars Cincluds 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses = PB Pedestrians > P Bicycles -> B Time Period : 12-12:15m 3 X4 2 A 5 6 IC P C IC IP P C CCP C  $\int_{1}^{1}$ Ċ 160 С C 10 C c (150) Ċ 30 c C 2P 50 0000 C C C I.C C C

618 Intersection Sketch: North Orientia GIOS Medon Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 12:15-12:30 pm 1C 210 \$ 5 <u>3</u> 10 |0 6 C IC G 1C C .C C (90) C B IC P ıΡ P C с С (160) C 10 C C Ċ P E.  $(\mathbb{I})$ 10 10 0 50 C., C 2% C C C Ċ C P IC C C IC 60 IC C C IC 38 C  $\mathcal{C}$ C IC IC

Intersection Sketch: North Orientia. Auc Grass Modial E CORE Cars Cincludes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buser >SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 12:30 -12:45PM 3 YZ 2 5 6 CCC 10 C C 10 C C C C C C С 20 C 30 0000 ZP zic, C C C 10 C C C 1C

8/8 Intersection Sketch: North orientia Grass Median Cars Cincluds 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses > PB Pedestriams > P Bisycles >B Time Period: 12:45-1:00PM F4 2 PUUUU 5 6 IC IC С С P O10 20 (Sc 2P 000000 4000 180 C. С C 8 C 00000 1C

Ensle Knolls Rd 6 Intersection SKetch: Hommark Rd Ś North 3 2 Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles >B Time Period : 11-11:15 Am 4 5 3 6 2 P iti 11111 11 60 20 C P

2/8 Eagle Knillsed Intersection Sketch: 10 3 North Hommeckt D-3 D Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 11:15-11:30 AM 4 5 6 2 nin IIIII 1111 1111 Sé 40

318 Engle Knullser Intersection Sketch: 6 North 5 Hommicks  $(\mathcal{D})$ 3) Ð 6 Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses > SB Public transit buses > PB Pedestriams > P Bicycles > B Time Period : 11:30-11:45 AM withiler 3 4_____ 5 2 6 1111 PBPP Pi 11 1 20) 52 CC. 38 B В

# 418

Eagle Karlls el Intersection Sketch: 0 5 North Hommock Rd 3 Ø Ð  $\geq$ Cars (includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buser asB Public transit buses > PB Pedestriams > P Bicycles > B Time Period : 11:45-12:00PM 4 S 1111 3 2 milli 1111 111 10 L ZP

5/8

Easte Kn. 115 Q1 Intersection Sketch: North 60 Hommicks Q1 0_7 3  $\bigcirc \rightarrow$ -9 Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) -> T Classifications. School buses >SB Public transit buses = PB Pedestriams > P Bisycles > B Time Period : 12-12:15pm PP 4 3 6 2 minin 1 IIIII 1.F 11 16 D

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7/8

Eagle KnillsRI Intersection Sketch: North 6 S HommicksRA (D_7 (3)  $\bigcirc \rightarrow$ E-(4) Cars Cincluds 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses > PB Pedestriams + P Bicycles -> B Time Period : 12:30 -12:45PM <u>Lu</u> pp <u>5</u> 4_ 2 < p P 1111 11 mmir 32 2P 20 IP 80

818

Intersection Sketch: Engle Kaulls RA North S Hommicky Q>> 3 4 1-Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses = PB Pedestriams > P Bicycles >B Time Period: 12:45-1:00pm S 4 2 6 PP ITII BBBB 1111 11 11 111 LU 10 7.P 1B

1/8 Intersection North Sketch: Grassmediar B Orverten Ave Cars (Includes 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 11-11:15 AM 80 2 -65 6 111/1/11/1 HILLING. 10 1111-1111 B 1'C 0 1111 80, 180 IP

2/8 Intersection Sketch: North Oriento Grass Medica Ave 2567 F. Otherwiy Lane Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) -> T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 11:15-11:30 Am 4 5 3 6 2 MUTTELIU 10  $\bigcirc$  $\bigcap$  $\mathcal{O}$ 11 IN INEN 1111111 1111121 11111 299 280

3/8

Intersection Sketch: North Grass Media-() FICANC FairwayLave Cars (Includes 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestrians > P Bicycles > B Time Period : 11:30-11:45 AM 5 4 6 3 2 инили 11 111111111 D  $\mathcal{O}$ 1010111 66111111 11111111 20 1111111 joc 236 2B

418 Intersection Sketch: North Grassmillur 2200 (Faurry Lr Oriente Cars (Includs 2-axle trucks) > c Trucks (Bormoreaxles) > T Classifications. School buses >SB Public transit buses + PB Pedestriams > P Bicycles > B Time Period : 11:45-12:00PM 5 4 2 6 B 1111111111 (1)\ Ň 11/11/11 11111111 10 11 11111 B 10 270 230

5/8 North Grassmediar 5-3 S G FAIrwy Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bicycles > B Time Period: 12-12:15PM 4

Intersection Sketch:

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Intersection Sketch: North Grass Mediar Drunte 127 × 701 634 Auc Fairway Lan Cars (Includes 2-axie trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses > PB Pedestriams > P Bicycles > B Time Period : 12:15-12:30 pm 5 4 6 2 11818111111 11 11111111 11)11111  $\mathcal{A}$ D D 20, 1111111 1//1 281 1P B

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Intersection Sketch: North Driento Grassmellar 122 Sel Arc Fallwey Care Cars (Includs 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses asB Public transit buses > PB Pedestriams > P Bicycles -> B Time Period : 12:30 -12:45PM 2 3 S 6 4 MUNICIPAL I 11111111111 1/11/11/1 111-111 160 270

Intersection Sketch: North Oricrta Grassmedian Au 127 561 FairwayLand Cars Cincluds 2-axle trucks) > c Trucks (Bormore axles) > T Classifications. School buses >SB Public transit buses > PB Pedestriams > P Bisycles > B Time Period : 12:45-1:00PM 2 6 11111111111 11111111 ()- AP19A 1[11/1/11] 210

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Site Code: Station ID: Station ID: MAMARONECK,NY Latitude: 0'0.0000 Undefined

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Sun	13-Mar-16	68	42	29	19	18	38	125	251	442	468	648	617	650	587	584	509	463	421	323	263	163	163	43	32	6966		79.2%	10:00	648	12:00	650
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Fri	11-Mar-16	33	14	11	7	30	86	174	722	598	544	582	676	680	679	658	678	760	852	723	484	326	255	203	124	9899	102.7%	112.6%	07:00	722	17:00	852
Thu	10-Mar-16	*	*	*	*	*	*	171	673	579	566	545	624	577	567	683	641	931	828	614	465	311	219	133	67	9194	95.4%	104.6%	07:00	673	16:00	931
	09-Mar-16 1	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	%0.0				
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		12:00 AM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	Vol.	PM Peak	Vol.

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6966	
8937	
9636	
6686	
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0	ADT 8,801
0	
Grand Total	ADT

Week Average	48	21	13	16	29 📕	53	143	506	482	516	515	578	597	590	636	608	610	621	548	400	291	191	135	75	8222			- 11:00	- 578	- 14:00
Sun 20-Mar-16	66	40	24	27	19	14	62	121	278	419	464	517	540	502	486	458	484	415	350	289	174	130	81	55	6048		73.6%	11:00	517	12:00
Sat 19-Mar-16 20	09	32	24	21	25	22	75	213	393	556	678	698	200	675	609	588	556	505	441	357	267	177	183	119	7974		97.0%	- 11:00	- 698	- 12:00
Average Day	32	15	6	12	31	67	172	641	540	528	493	567	587	590	672	642	646	686	609	431	320	205	136	71	8702		105.8%	- 07:00	- 641	- 17:00
Fri 18-Mar-16	33	24	14	12	46	73	154	637	580	561	559	642	623	642	723	700	644	740	592	426	334	256	216	111	9342	107.4%	113.6%	11:00	642	17:00
Thu 17-Mar-16	33	17	9	10	31	74	166	652	446		394			586		578		674	615					93	8245	94.7%	100.3%	01:00	652	17:00
Wed 16-Mar-16	28	14	6	13	26	09	167	625	561	550	544	555	583	619	691	640	678	746	674	447	318	211	137	56	8952	102.9%	108.9%	07:00	625	17:00
Tue 15-Mar-16	34	12	5	15	25	62	186	656	572	559	486	556	581	583	683	610	659	636	607	447	335	196	102	51	8658	99.5%	105.3%	07:00	656	14:00
Mon 14-Mar-16	*	10	6	11	28	66	189	637	542	511	483	554	563	522	642	684	664	632	557	387	273	172	108	43	8287	95.2%	100.8%	07:00	637	15:00
Start Time	12:00 AM	01:00	02:00	03:00	04:00	05:00	00:90	01:00	08:00	00:00	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	Vol.	PM Peak

Traffic Databank 716 S Sixth Ave Mount Vernon, NY 10550 Site Code: Station ID: Station ID: SB BOSTON POST RD S OF ORIENTA AVE MAMARONECK,NY Latitude: 0'0.0000 Undefined

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Sun 27-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0		0.0%			I		6048
Sat 26-Mar-16 2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0		0.0%	•				7974
Average Day	7	ç	0	0	0	0	0	0	0	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	10		100.0%	- 00:00		•	1	8712
Fri 25-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%			·		9342
Thu 24-Mar-16 2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%					8245
Wed 23-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	%0.0		T			8952
Tue 22-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%					8658
Mon 21-Mar-16	7	ę	0	0	0	0	0	0	0	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	10	100.0%	100.0%	00:00	_			8297
Start Time	12:00 AM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:60	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	V 01.	PM Peak	- O.	Total

Traffic Databank 716 S Sixth Ave Mount Vernon, NY 10550 Site Code: Station ID: Station ID: MAMARONECK,NY Latitude: 0'0.0000 Undefined

ADT 8,042 ADT

AADT 8,042

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/10/1																
6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
01:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
02:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
03:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
04:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
05:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
06:00	0	126	37	2	2	0	0	2	1	0	1	0	0	0	0	171
07:00	0	498	148	3	17	0	0	6	0	0	0	0	0	1	0	673
08:00	0	431	127	1	14	0	0	6	0	0	0	0	0	0	0	579
09:00	0	424	125	1	13	0	0	2	0	0	0	0	0	1	0	566
10:00	0	403	119	0	16	0	0	7	0	0	0	0	0	0	0	545
11:00	0	459	137	1	21	0	0	6	0	0	0	0	0	0	0	624
12 PM	0	426	127	1	13	0	0	9	0	0	0	0	0	1	0	577
13:00	0	418	124	2	17	0	0	6	0	0	0	0	0	0	0	567
14:00	0	501	149	4	18	0	0	10	1	0	0	0	0	0	0	683
15:00	0	475	142	0	16	0	0	8	0	0	0	0	0	0	0	641
16:00	0	690	206	1	24	0	0	10	0	0	0	0	0	0	0	931
17:00	0	609	181	1	22	0	0	14	0	0	0	1	0	0	0	828
18:00	0	453	135	0	12	0	0	12	0	0	1	0	0	1	0	614
19:00	0	344	102	0	14	0	0	4	0	0	0	0	0	1	0	465
20:00	0	232	69	0	5	0	0	5	0	0	0	0	0	0	0	311
21:00	0	164	48	0	4	0	0	3	0	0	0	0	0	0	0	219
22:00	0	102	30	0	0	0	0	1	0	0	0	0	0	0	0	133
23:00	0	52	15	0	0	0	0	0	0	0	0	0	0	0	0	67
Total	0	6807	2021	17	228	0	0	111	2	0	2	1	0	5	0	9194
Percent	0.0%	74.0%	22.0%	0.2%	2.5%	0.0%	0.0%	1.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.1%	0.0%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/11/1						_										
6	0	25	8	0	0	0	0	0	0	0	0	0	0	0	0	33
01:00	0	11	3	0	0	0	0	0	0	0	0	0	0	0	0	14
02:00	0	8	3	0	0	0	0	0	0	0	0	0	0	0	0	11
03:00	0	6	1	0	0	0	0	0	0	0	0	0	0	0	0	7
04:00	0	23	7	0	0	0	0	0	0	0	0	0	0	0	0	30
05:00	0	64	20	0	0	0	0	1	1	0	0	0	0	0	0	86
06:00	0	129	39	1	2	0	0	3	0	0	0	0	0	0	0	174
07:00	0	527	157	6	20	0	0	12	0	0	0	0	0	0	0	722
08:00	0	439	130	1	18	0	0	9	0	0	1	0	0	0	0	598
09:00	0	403	119	1	13	0	0	8	0	0	0	0	0	0	0	544
10:00	0	430	128	1	15	0	0	7	0	0	0	0	0	1	0	582
11:00	0	494	147	0	21	0	0	12	1	0	0	0	0	1	0	676
12 PM	0	499	148	1	21	0	0	10	0	0	1	0	0	0	0	680
13:00	0	501	148	1	18	0	0	10	0	0	0	1	0	0	0	679
14:00	0	490	146	0	16	0	0	6	0	0	0	0	0	0	0	658
15:00	0	504	149	3	17	0	0	5	0	0	0	0	0	0	0	678
16:00	0	564	168	2	17	0	0	8	0	0	0	0	0	1	0	760
17:00	0	628	187	0	26	0	0	11	0	0	0	0	0	0	0	852
18:00	0	542	161	0	13	0	0	6	0	0	0	0	0	1	0	723
19:00	0	356	106	0	16	0	0	5	1	0	0	0	0	0	0	484
20:00	0	241	72	0	9	0	0	4	0	0	0	0	0	0	0	326
21:00	0	191	56	0	6	0	0	2	0	0	0	0	0	0	0	255
22:00	0	156	46	0	1	0	0	0	0	0	0	0	0	0	0	203
23:00	0	96	28	0	0	0	0	0	0	0	0	0	0	0	0	124
Total	0	7327	2177	17	249	0	0	119	3	0	2	1	0	4	0	9899
Percent	0.0%	74.0%	22.0%	0.2%	2.5%	0.0%	0.0%	1.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/12/1						_										
6	0	43	13	0	0	0	0	0	0	0	0	0	0	0	0	56
01:00	0	28	7	0	0	0	0	0	0	0	0	0	0	0	0	35
02:00	0	13	5	0	0	0	0	0	0	0	0	0	0	0	0	18
03:00	0	12	4	0	0	0	0	0	0	0	0	0	0	0	0	16
04:00	0	14	4	0	0	0	0	0	0	0	0	0	0	0	0	18
05:00	0	19	6	0	0	0	0	0	0	0	0	0	0	0	0	25
06:00	0	70	22	0	2	0	0	3	0	0	0	0	0	0	0	97
07:00	0	193	57	4	6	0	0	2	0	0	0	0	0	1	0	263
08:00	0	322	95	1	9	0	0	3	0	0	0	0	0	0	0	430
09:00	0	536	160	0	20	0	0	1	0	0	0	0	0	0	0	717
10:00	0	550	162	2	21	0	0	4	0	0	0	0	0	1	0	740
11:00	0	588	175	0	13	0	0	5	1	0	1	0	0	0	0	783
12 PM	0	552	164	0	25	0	0	1	0	0	0	0	0	0	0	742
13:00	0	545	162	3	14	0	0	7	0	0	0	0	0	0	0	731
14:00	0	520	155	2	20	0	0	9	1	0	0	1	0	0	0	708
15:00	0	498	148	0	14	0	0	3	0	0	1	0	0	1	0	665
16:00	0	476	142	1	16	0	0	8	0	0	0	0	0	0	0	643
17:00	0	409	121	1	14	0	0	4	0	0	0	0	0	0	0	549
18:00	0	392	117	0	12	0	0	4	0	0	0	0	0	0	0	525
19:00	0	269	79	0	12	0	0	3	0	0	0	0	0	0	0	363
20:00	0	211	63	0	4	0	0	3	0	0	1	0	0	0	0	282
21:00	0	161	48	0	6	0	0	3	0	0	0	0	0	0	0	218
22:00	0	139	41	0	2	0	0	1	0	0	0	0	0	0	0	183
23:00	0	100	30	0	0	0	0	0	0	0	0	0	0	0	0	130
Total	0	6660	1980	14	210	0	0 0.0%	64	2	0	3 0.0%	1	0	3	0	8937
Percent	0.0%	74.5%	22.2%	0.2%	2.3%	0.0%	0.0%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau			lacineu
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/13/1			0													
6	0	52	16	0	0	0	0	0	0	0	0	0	0	0	0	68
01:00	0	32	10	0	0	0	0	0	0	0	0	0	0	0	0	42
02:00	0	22	7	0	0	0	0	0	0	0	0	0	0	0	0	29
03:00	0	15	4	0	0	0	0	0	0	0	0	0	0	0	0	19
04:00	0	14	4	0	0	0	0	0	0	0	0	0	0	0	0	18
05:00	0	29	9	0	0	0	0	0	0	0	0	0	0	0	0	38
06:00	0	90	27	4	3	0	0	1	0	0	0	0	0	0	0	125
07:00	0	186	55	3	4	0	0	3	0	0	0	0	0	0	0	251
08:00	0	325	97	0	12	0	0	8	0	0	0	0	0	0	0	442
09:00	0	346	103	0	15	0	0	2	0	0	1	0	0	1	0	468
10:00	0	481	143	0	16	0	0	8	0	0	0	0	0	0	0	648
11:00	0	463	137	1	11	0	0	5	0	0	0	0	0	0	0	617
12 PM	1	480	134	2	6	0	0	2	0	0	1	0	0	1	23	650
13:00	1	443	112	0	7	0	0	4	0	0	0	0	0	2	18	587
14:00	1	435	126	0	8	0	0	5	0	0	0	0	0	0	9	584
15:00	1	396	96	0	6	0	0	2	0	0	1	0	0	1	6	509
16:00	1	352	97	0	5	0	0	0	0	1	0	0	0	0	7	463
17:00	0	318	85	0	8	0	0	1	0	0	0	0	0	1	8	421
18:00	0	231	83	0	3	0	0	1	0	0	1	0	0	1	3	323
19:00	0	196	54	0	8	0	0	1	0	0	0	0	0	0	4	263
20:00	1	119	34	0	5	0	0	0	0	0	0	0	0	0	4	163
21:00	0	110	31	1	14	1	0	1	0	0	0	0	1	0	4	163
22:00	0	34	9	0	0	0	0	0	0	0	0	0	0	0	0	43
23:00	0	26	5	0	1	0	0	0	0	0	0	0	0	0	0	32
Total	6	5195	1478	11	132	1	0	44	0	1	4	0	1	7	86	6966
Percent	0.1%	74.6%	21.2%	0.2%	1.9%	0.0%	0.0%	0.6%	0.0%	0.0%	0.1%	0.0%	0.0%	0.1%	1.2%	
Total	6	25989	7656	59	819	1	0	338	7	1	11	3	1	19	86	34996
Percent	0.0%	74.3%	21.9%	0.2%	2.3%	0.0%	0.0%	1.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.1%	0.2%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/14/1						_										
6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
01:00	0	6	4	0	0	0	0	0	0	0	0	0	0	0	0	10
02:00	0	4	3	0	0	1	0	1	0	0	0	0	0	0	0	9
03:00	0	5	3	1	1	0	0	1	0	0	0	0	0	0	0	11
04:00	0	15	7	0	5	0	0	0	0	0	0	0	0	0	1	28
05:00	0	41	16	0	7	1	0	0	1	0	0	0	0	0	0	66
06:00	0	124	47	1	7	1	0	2	1	0	0	0	0	0	6	189
07:00	0	474	113	3	21	1	0	3	1	0	1	0	0	0	20	637
08:00	1	359	140	4	19	2	0	2	0	1	0	0	0	0	14	542
09:00	0	310	167	1	20	1	0	2	0	0	0	0	0	0	10	511
10:00	0	317	127	6	23	2	0	0	1	0	1	0	0	0	6	483
11:00	0	362	155	1	24	1	0	4	0	0	0	0	1	0	6	554
12 PM	1	389	139	4	15	0	0	8	0	0	1	0	0	0	6	563
13:00	0	368	115	3	20	0	0	8	0	0	0	0	0	0	8	522
14:00	0	458	134	1	15	1	0	3	0	0	0	0	0	0	30	642
15:00	0	480	158	9	13	0	0	6	1	1	2	0	0	0	14	684
16:00	0	470	160	2	15	2	0	5	0	0	0	0	0	0	10	664
17:00	0	461	140	1	5	0	0	10	0	0	0	0	0	0	15	632
18:00	0	432	102	0	9	0	0	1	0	0	0	0	0	0	13	557
19:00	0	301	73	0	6	0	0	2	0	0	1	0	0	0	4	387
20:00	0	197	57	0	12	0	0	1	0	1	0	0	0	0	5	273
21:00	0	136	32	0	3	0	0	0	0	0	0	0	0	0	1	172
22:00	0	84	21	0	3	0	0	0	0	0	0	0	0	0	0	108
23:00	0	32	11	0	0	0	0	0	0	0	0	0	0	0	0	43
Total Percent	2 0.0%	5825 70.3%	1924 23.2%	37 0.4%	243 2.9%	13 0.2%	0 0.0%	59 0.7%	5 0.1%	3 0.0%	6 0.1%	0 0.0%	1 0.0%	0 0.0%	169 2.0%	8287

Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/15/1						-										
6	0	25	9	0	0	0	0	0	0	0	0	0	0	0	0	34
01:00	0	7	5	0	0	0	0	0	0	0	0	0	0	0	0	12
02:00	0	3	2	0	0	0	0	0	0	0	0	0	0	0	0	5
03:00	0	9	3	1	1	0	0	1	0	0	0	0	0	0	0	15
04:00	0	11	10	0	3	1	0	0	0	0	0	0	0	0	0	25
05:00	0	51	7	0	2	1	0	0	1	0	0	0	0	0	0	62
06:00	0	115	38	2	15	2	0	0	1	0	0	0	0	1	12	186
07:00	2	462	119	3	22	2	0	9	0	0	0	0	0	1	36	656
08:00	0	373	128	9	25	3	1	5	1	0	1	0	0	0	26	572
09:00	1	367	128	4	29	1	0	8	1	0	0	0	0	1	19	559
10:00	1	323	121	4	20	1	0	7	1	0	0	0	0	0	8	486
11:00	0	390	124	5	16	0	0	8	1	0	0	0	1	0	11	556
12 PM	0	374	156	3	28	0	0	8	0	1	0	1	0	0	10	581
13:00	3	402	142	3	17	0	0	4	0	1	0	0	0	1	10	583
14:00	3	463	144	1	26	0	0	5	0	0	0	0	0	2	39	683
15:00	0	428	134	5	13	0	0	13	0	0	0	0	0	1	16	610
16:00	0	474	146	1	19	0	0	3	0	0	0	0	0	0	16	659
17:00	0	491	110	0	17	0	0	6	0	0	0	0	0	0	12	636
18:00	0	460	116	0	9	1	0	4	1	0	0	0	0	0	16	607
19:00	1	347	82	0	5	0	0	2	0	0	0	0	0	1	9	447
20:00	0	252	69	0	7	0	0	1	0	0	0	0	0	0	6	335
21:00	0	159	29	0	4	0	0	1	0	0	0	0	0	0	3	196
22:00	0	79	21	0	2	0	0	0	0	0	0	0	0	0	0	102
23:00	0	31	19	0	1	0	0	0	0	0	0	0	0	0	0	51
Total	11	6096	1862	41	281	12	1	85	7	2	1	1	1	8	249	8658
Percent	0.1%	70.4%	21.5%	0.5%	3.2%	0.1%	0.0%	1.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.1%	2.9%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau	1000.00	.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/16/1																
6	0	21	6	0	1	0	0	0	0	0	0	0	0	0	0	28
01:00	0	10	4	0	0	0	0	0	0	0	0	0	0	0	0	14
02:00	0	8	0	0	0	1	0	0	0	0	0	0	0	0	0	9
03:00	0	6	4	1	0	0	0	2	0	0	0	0	0	0	0	13
04:00	0	16	9	0	0	0	0	0	0	0	0	0	0	0	1	26
05:00	0	36	14	0	4	3	0	0	1	0	0	0	0	0	2	60
06:00	1	98	37	4	16	1	0	2	1	0	0	0	0	0	7	167
07:00	1	453	90	2	19	2	0	5	0	0	0	0	0	4	49	625
08:00	1	361	150	3	26	1	0	4	2	0	0	0	0	0	13	561
09:00	0	388	123	2	15	3	0	5	0	0	0	0	0	0	14	550
10:00	0	377	131	4	19	0	0	5	0	0	0	0	0	0	8	544
11:00	1	381	132	4	19	1	0	2	1	0	0	0	0	1	13	555
12 PM	0	378	166	5	20	0	0	7	0	0	0	0	0	0	7	583
13:00	0	418	145	4	28	0	0	9	0	0	0	0	0	2	13	619
14:00	1	496	127	7	24	0	0	4	0	0	0	0	0	4	28	691
15:00	0	448	143	5	12	0	0	9	0	0	0	0	0	2	21	640
16:00	0	497	135	2	14	1	0	15	0	1	0	0	0	0	13	678
17:00	0	545	152	0	11	0	0	8	0	0	0	0	0	1	29	746
18:00	0	518	123	0	10	1	0	4	0	0	0	0	0	0	18	674
19:00	0	328	93	0	11	0	0	2	0	1	0	0	0	0	12	447
20:00	0	222	73	0	13	1	0	3	0	0	0	0	0	0	6	318
21:00	0	151	53	0	4	0	0	0	0	0	0	0	0	0	3	211
22:00	0	113	21	0	1	0	0	0	0	0	0	0	0	0	2	137
23:00	0	37	19	0	0	0	0	0	0	0	0	0	0	0	0	56
Total	5	6306	1950	43	267	15	0	86	5	2	0	0	0	14	259	8952
Percent	0.1%	70.4%	21.8%	0.5%	3.0%	0.2%	0.0%	1.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.2%	2.9%	

Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/17/1						_										
6	0	18	13	0	1	0	0	0	0	0	0	0	0	0	1	33
01:00	0	15	1	0	1	0	0	0	0	0	0	0	0	0	0	17
02:00	0	4	2	0	0	0	0	0	0	0	0	0	0	0	0	6
03:00	0	9	0	1	0	0	0	0	0	0	0	0	0	0	0	10
04:00	0	20	7	0	1	1	0	2	0	0	0	0	0	0	0	31
05:00	0	53	8	1	8	2	0	1	1	0	0	0	0	0	0	74
06:00	0	115	32	1	7	2	0	0	0	0	0	0	0	0	9	166
07:00	1	431	106	6	22	3	0	3	1	0	0	0	0	1	78	652
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	446	446
09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	459	459
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	394	394
11:00	0	234	81	2	15	0	0	2	0	0	0	0	0	4	188	526
12 PM	2	406	126	8	19	0	0	2	3	0	0	0	0	0	20	586
13:00	0	395	144	4	26	0	0	3	0	1	0	0	1	2	10	586
14:00	0	425	137	1	23	0	0	6	0	0	0	0	0	0	27	619
15:00	0	114	43	1	7	1	0	3	0	0	0	0	0	1	408	578
16:00	0	145	40	0	4	0	0	1	0	0	0	0	0	0	397	587
17:00	2	488	138	0	19	0	0	7	0	0	0	0	0	0	20	674
18:00	1	469	126	1	6	0	0	3	0	0	0	0	0	0	9	615
19:00	0	324	97	0	9	0	0	5	0	0	0	0	0	0	11	446
20:00	0	265	62	0	7	0	0	2	0	0	1	0	0	0	2	339
21:00	0	150	37	0	3	0	0	0	0	0	0	0	0	0	2	192
22:00	0	91	20	0	2	0	0	0	0	0	0	0	0	0	3	116
23:00	0	67	23	0	2	0	0	0	0	0	0	0	0	0	1	93
Total	6	4238	1243	26	182	9	0	40	5	1	1	0	1	8	2485	8245
Percent	0.1%	51.4%	15.1%	0.3%	2.2%	0.1%	0.0%	0.5%	0.1%	0.0%	0.0%	0.0%	0.0%	0.1%	30.1%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/18/1						_										
6	0	27	5	0	0	0	0	0	0	0	0	0	0	0	1	33
01:00	0	17	5	0	1	0	0	1	0	0	0	0	0	0	0	24
02:00	0	8	4	0	1	1	0	0	0	0	0	0	0	0	0	14
03:00	0	9	2	1	0	0	0	0	0	0	0	0	0	0	0	12
04:00	0	27	11	0	2	0	0	2	2	0	0	0	0	1	1	46
05:00	0	51	13	2	4	1	0	0	1	0	0	0	0	0	1	73
06:00	0	106	26	0	14	1	0	2	1	0	0	0	0	0	4	154
07:00	1	471	102	4	24	2	0	3	1	0	0	0	0	1	28	637
08:00	2	389	141	6	17	3	0	6	0	0	0	0	0	0	16	580
09:00	2	369	137	1	26	2	0	6	0	0	0	0	0	0	18	561
10:00	0	390	119	5	24	0	0	5	1	0	0	0	1	0	14	559
11:00	0	450	137	2	23	2	0	10	1	0	0	0	0	0	17	642
12 PM	0	457	134	1	19	0	0	7	0	0	1	0	0	0	4	623
13:00	1	438	148	3	23	0	0	12	0	0	1	0	0	0	16	642
14:00	1	517	143	4	20	1	0	7	0	0	0	0	0	0	30	723
15:00	1	500	148	2	20	0	0	7	0	0	1	0	1	1	19	700
16:00	0	462	141	1	18	1	0	7	0	0	0	0	0	0	14	644
17:00	1	572	127	0	10	0	0	6	0	0	0	0	0	3	21	740
18:00	0	446	111	0	10	0	0	8	0	0	0	0	0	0	17	592
19:00	0	321	85	0	7	0	0	3	0	0	1	0	0	0	9	426
20:00	2	238	81	0	9	0	0	3	0	0	0	0	0	0	1	334
21:00	0	184	60	0	7	0	0	1	0	0	0	0	0	0	4	256
22:00	0	169	42	0	3	0	0	0	0	0	0	0	0	0	2	216
23:00	0	87	21	0	3	0	0	0	0	0	0	0	0	0	0	111
Total	11	6705	1943	32	285	14	0	96	7	0	4	0	2	6	237	9342
Percent	0.1%	71.8%	20.8%	0.3%	3.1%	0.1%	0.0%	1.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.1%	2.5%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/19/1						-										
6	0	42	15	0	1	0	0	0	0	0	0	0	0	0	2	60
01:00	0	22	10	0	0	0	0	0	0	0	0	0	0	0	0	32
02:00	0	18	4	0	2	0	0	0	0	0	0	0	0	0	0	24
03:00	1	10	8	1	1	0	0	0	0	0	0	0	0	0	0	21
04:00	0	16	5	0	3	1	0	0	0	0	0	0	0	0	0	25
05:00	0	14	6	0	1	1	0	0	0	0	0	0	0	0	0	22
06:00	0	47	17	1	6	1	0	1	1	0	0	0	0	0	1	75
07:00	0	142	50	0	12	0	0	2	1	1	0	0	0	1	4	213
08:00	1	277	91	0	15	0	0	5	0	0	0	0	0	0	4	393
09:00	0	398	125	1	17	0	0	5	0	1	0	0	0	0	9	556
10:00	0	499	137	1	10	1	0	10	0	0	0	0	0	1	19	678
11:00	0	520	117	1	15	1	0	4	1	0	0	0	0	8	31	698
12 PM	2	515	140	1	14	0	0	3	1	1	0	0	0	0	23	700
13:00	1	500	139	0	12	0	0	6	0	0	0	0	0	0	17	675
14:00	0	448	118	0	8	0	0	9	0	0	0	0	0	0	26	609
15:00	1	420	135	1	12	0	0	7	0	1	0	0	0	0	11	588
16:00	0	406	125	0	8	0	0	2	0	1	0	0	0	2	12	556
17:00	1	394	95	0	5	0	0	5	0	0	0	0	0	0	5	505
18:00	0	354	74	0	5	0	0	1	0	0	1	0	0	0	6	441
19:00	0	275	69	0	6	0	0	3	1	0	0	0	0	0	3	357
20:00	0	200	60	0	6	0	0	0	0	0	0	0	0	0	1	267
21:00	0	144	31	0	1	0	0	0	0	0	0	0	0	0	1	177
22:00	0	133	43	0	3	0	0	1	0	0	0	0	0	0	3	183
23:00	0	81	35	0	1	0	0	2	0	0	0	0	0	0	0	119
Total	7	5875	1649	7	164	5	0	66	5	5	1	0	0	12	178	7974
Percent	0.1%	73.7%	20.7%	0.1%	2.1%	0.1%	0.0%	0.8%	0.1%	0.1%	0.0%	0.0%	0.0%	0.2%	2.2%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	lacinica
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/20/1						-										
6	1	71	26	0	1	0	0	0	0	0	0	0	0	0	0	99
01:00	0	29	11	0	0	0	0	0	0	0	0	0	0	0	0	40
02:00	0	18	4	0	2	0	0	0	0	0	0	0	0	0	0	24
03:00	0	21	4	0	1	0	0	1	0	0	0	0	0	0	0	27
04:00	0	14	2	0	2	0	0	0	0	0	0	0	0	1	0	19
05:00	0	11	3	0	0	0	0	0	0	0	0	0	0	0	0	14
06:00	0	45	16	0	1	0	0	0	0	0	0	0	0	0	0	62
07:00	0	85	26	0	6	0	0	0	0	0	0	0	0	0	4	121
08:00	0	208	59	0	7	0	0	1	0	0	0	0	0	0	3	278
09:00	0	322	81	0	6	0	0	3	0	0	0	0	0	0	7	419
10:00	0	347	96	0	7	0	0	3	0	0	0	0	0	1	10	464
11:00	1	386	101	0	10	0	0	4	0	0	0	0	0	2	13	517
12 PM	0	394	113	0	7	1	0	8	0	1	1	0	0	1	14	540
13:00	0	390	88	0	3	0	0	4	0	1	0	0	0	1	15	502
14:00	0	361	100	0	5	0	0	2	1	1	1	0	0	0	15	486
15:00	1	358	90	0	3	0	0	0	0	0	0	0	0	0	6	458
16:00	0	365	97	1	10	0	0	2	0	0	0	1	0	0	8	484
17:00	0	300	99	0	4	0	0	3	0	0	0	0	0	1	8	415
18:00	0	282	53	0	9	0	0	1	0	0	0	0	0	0	5	350
19:00	2	220	57	0	7	0	0	1	0	0	0	0	0	0	2	289
20:00	0	137	35	0	2	0	0	0	0	0	0	0	0	0	0	174
21:00	0	106	22	0	1	0	0	0	0	0	0	0	0	0	1	130
22:00	0	61	17	0	3	0	0	0	0	0	0	0	0	0	0	81
23:00	0	34	13	0	1	0	0	0	0	0	0	0	0	0	7	55
Total	5	4565	1213	1	98	1	0	33	1	3	2	1	0	7	118	6048
Percent	0.1%	75.5%	20.1%	0.0%	1.6%	0.0%	0.0%	0.5%	0.0%	0.0%	0.0%	0.0%	0.0%	0.1%	2.0%	

716 S Sixth Ave Mount Vernon, NY 10550

SB													Lau		.0000 01	luenneu
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/21/1																
6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	7
01:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	3
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
10:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
11:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
12 PM	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
13:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
14:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
15:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
16:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
17:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
18:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
19:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
20:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
21:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
22:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
23:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	10
Percent	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%	
Total	47	39610	11784	187	1520	69	1	465	35	16	15	2	5	55	3705	57516
Percent	0.1%	68.9%	20.5%	0.3%	2.6%	0.1%	0.0%	0.8%	0.1%	0.0%	0.0%	0.0%	0.0%	0.1%	6.4%	

th Ave NY 1	
	3 S Sixth

																												•	ı	1	
Week	91 0	44	21	21	24 📗	48	165	419	602	613	650	749	758	740	739	804	764	695	629	514	355	290	208	132	10075			11:00	749	15:00	804
																													ı		
Sun 12 Mar 16	110 110	09	26	27	25	42	106	227	385	519	632	209	709	728	721	648	614	517	423	354	221	197	87	38	8125		80.6%	11:00	709	13:00	728
Sat		47	23	21	25	40	115	272	483	647	761	858	911	828	769	798	798	696	664	518	395	294	282	214	10569		104.9%	11:00	858	12:00	911
	-																														
Average	сау 53	25	13	16	23	61	219	588	770	642	604	714	705	702	732	884	823	783	714	592	402	335	232	137	10769		106.9%	08:00	770	15:00	884
Fri 14 Mor 16	53	25	13	16	23	61	211	656	0/1	663	608	702	743	724	722	907	864	805	733	615	406	350	244	156	11070	102.8%	109.9%	08:00	770	15:00	607
Thu 10 Mar 16		*	*	*	*	*	227	521	771	622	009	726	667	679	743	862	782	761	695	568	397	320	220	118	10279	95.4%	102.0%	08:00	771	15:00	862
Wed Mor 16		*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	%0.0	0.0%				
Tue Do Mor 16		*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	%0.0	0.0%				
Mon Mor 16		*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%				
Start	MA	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	Vol.	PM Peak	Vol.

10075	
8125	
10569	
10769	
11070	
10279	AADT 10,078
0	
0	ADT 10,078
Grand Total	ADT

**Traffic Databank** 716 S Sixth Ave Mount Vernon, NY 10550 Site Code: Station ID: NB BOSTON POST RD S OF ORIENTA AVE MAMARONECK,NY Latitude: 0' 0.0000 Undefined

Week Average	71	32	22	19	27	49 📕	168	479	605	597	574	644	685	685	725	795	734	702	628	512	403	275	195	109	9735			- 11:00	- 644	- 15:00	- 795
Sun 20-Mar-16	119	55	33	25	21	27	65	125	230	427	522	583	653	645	622	558	555	487	471	375	275	176	102	71	7222		74.2%	11:00	583	12:00	653
Sat 19-Mar-16 20	95	46	40	25	29	34	88	246	413	609	672	829	805	788	766	969	645	628	569	480	342	291	233	191	9554		98.1%	- 11:00	- 829	- 12:00	- 805
Average Dav	53	25	17	17	27	57	204	597	718	628	565	619	667	673	738	863	788	760	671	546	440	291	206	101	10271		105.5%	- 08:00	- 718 -	- 15:00 -	- 863 -
Fri 18-Mar-16	62	36	21	15	37	65	196	604	686	624	592	678	737	669	775	888	785	768	651	579	435	344	290	169	10736	104.5%	110.3%	08:00	686	15:00	888
Thu 17-Mar-16 1		21	11	17	20	09	222	587	678	652	598	575	706	679	711	855	781	798	663	564	457	312	218	95	10330	100.6%	106.1%	08:00	678	15:00	855
Wed 16-Mar-16	51	25	18	22	33	45	192	631	734	647	530	650	654	703	781	826	826	828	746	543	472	274	182	76	10489	102.1%	107.7%	08:00	734	17:00	828
Tue 15-Mar-16	_	20	13	12	24	53	210	566	764	618	549	633	636	673	752	890	791	969	681	578	445	288	171	60	10196	99.3%	104.7%	08:00	764	15:00	890
Mon 14-Mar-16		24	20	17	23	62	202	597	728	601	555	561	604	609	699	857	758	718	615	468	392	239	170	74	9563	93.1%	98.2%	08:00	728	15:00	857
Start Time	M	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:60	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	Vol.	PM Peak	Vol.

																													•			
Week Average	6	4	0	0	0	0	0	0	0	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	10				- 9 -	ı		9745
Sun 27-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0		%0.0			ı		7222
Sat 26-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0		0.0%	1				9554
Average Day	9	4	0	0	0	0	0	0	0	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	10		100.0%	00:00 -	- 6			10281
Fri 25-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%			ı		10736
Thu 24-Mar-16 2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%	1		'		10330
Wed 23-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	0.0%	1		ı		10489
Tue 22-Mar-16	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%	%0.0				•	10196
Mon 21-Mar-16	9	4	0	0	0	0	0	0	0	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	10	100.0%	100.0%	00:00	9	I		9573
Start Time	12:00 AM	01:00	02:00	03:00	04:00	05:00	00:90	01:00	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	05:00	00:90	07:00	08:00	00:00	10:00	11:00	Day Total	% Avg. WkDay	% Avg. Week	AM Peak	Vol.	PM Peak	Crond	Grand T · ·

Traffic Databank 716 S Sixth Ave Mount Vernon, NY 10550 Site Code: Station ID: NB BOSTON POST RD S OF ORIENTA AVE MAMARONECK,NY Latitude: 0'0.0000 Undefined

ADT 9,516 ADT

AADT 9,516

716 S Sixth Ave Mount Vernon, NY 10550

NB													Edi			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/10/1						_										
6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
01:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
02:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
03:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
04:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
05:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
06:00	0	156	60	0	6	0	0	5	0	0	0	0	0	0	0	227
07:00	0	355	136	0	16	0	0	12	1	1	0	0	0	0	0	521
08:00	0	530	203	0	21	0	0	16	0	0	1	0	0	0	0	771
09:00	0	427	164	0	20	0	0	10	0	1	0	0	0	0	0	622
10:00	0	409	156	0	19	0	0	15	0	0	0	0	0	1	0	600
11:00	0	497	191	0	21	0	0	16	0	1	0	0	0	0	0	726
12 PM	0	461	176	0	18	0	0	11	0	0	0	0	0	1	0	667
13:00	0	467	180	0	17	0	0	14	0	0	1	0	0	0	0	679
14:00	0	511	195	0	21	0	0	16	0	0	0	0	0	0	0	743
15:00	0	589	226	0	25	0	0	17	1	0	3	0	0	1	0	862
16:00	0	539	205	0	20	0	0	17	0	0	0	1	0	0	0	782
17:00	0	523	199	0	23	0	0	15	0	1	0	0	0	0	0	761
18:00	0	481	183	0	17	0	0	13	0	0	0	0	1	0	0	695
19:00	0	387	148	0	20	0	0	12	0	0	1	0	0	0	0	568
20:00	0	270	102	0	12	0	0	9	1	1	1	0	0	1	0	397
21:00	0	219	84	0	8	0	0	8	0	0	0	1	0	0	0	320
22:00	0	151	58	0	4	0	0	4	1	1	1	0	0	0	0	220
23:00	0	80	31	0	4	0	0	3	0	0	0	0	0	0	0	118
Total	0	7052	2697	0	292	0	0	213	4	6	8	2	1	4	0	10279
Percent	0.0%	68.6%	26.2%	0.0%	2.8%	0.0%	0.0%	2.1%	0.0%	0.1%	0.1%	0.0%	0.0%	0.0%	0.0%	

Mount Vernon, NY 10550

NB													Lati			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/11/1						_										
6	0	37	15	0	1	0	0	0	0	0	0	0	0	0	0	53
01:00	0	18	7	0	0	0	0	0	0	0	0	0	0	0	0	25
02:00	0	9	4	0	0	0	0	0	0	0	0	0	0	0	0	13
03:00	0	13	3	0	0	0	0	0	0	0	0	0	0	0	0	16
04:00	0	16	7	0	0	0	0	0	0	0	0	0	0	0	0	23
05:00	0	42	17	0	2	0	0	0	0	0	0	0	0	0	0	61
06:00	0	146	56	0	6	0	0	3	0	0	0	0	0	0	0	211
07:00	0	451	173	0	19	0	0	12	1	0	0	0	0	0	0	656
08:00	0	529	202	0	22	0	0	17	0	0	0	0	0	0	0	770
09:00	0	458	175	0	18	0	0	12	0	0	0	0	0	0	0	663
10:00	0	415	158	0	18	0	0	12	1	1	2	0	0	1	0	608
11:00	0	481	183	0	23	0	0	14	0	0	0	1	0	0	0	702
12 PM	0	513	196	0	18	0	0	14	1	0	0	0	0	1	0	743
13:00	0	498	191	0	22	0	0	13	0	0	0	0	0	0	0	724
14:00	0	495	189	0	21	0	0	16	0	1	0	0	0	0	0	722
15:00	0	627	239	0	23	0	0	16	0	0	2	0	0	0	0	907
16:00	0	593	226	0	22	0	0	19	1	0	1	1	1	0	0	864
17:00	0	555	212	0	21	0	0	13	0	2	0	2	0	0	0	805
18:00	0	503	193	0	21	0	0	16	0	0	0	0	0	0	0	733
19:00	0	422	161	0	17	0	0	14	0	1	0	0	0	0	0	615
20:00	0	279	107	0	14	0	0	5	0	0	1	0	0	0	0	406
21:00	0	242	92	0	11	0	0	5	0	0	0	0	0	0	0	350
22:00	0	168	64	0	7	0	0	5	0	0	0	0	0	0	0	244
23:00	0	108	41	0	4	0	0	2	1	0	0	0	0	0	0	156
Total	0	7618	2911	0	310	0	0	208	5	5	6	4	1	2	0	11070
Percent	0.0%	68.8%	26.3%	0.0%	2.8%	0.0%	0.0%	1.9%	0.0%	0.0%	0.1%	0.0%	0.0%	0.0%	0.0%	

Mount Vernon, NY 10550

NB													Lati			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/12/1						_										
6	0	75	29	0	3	0	0	3	0	0	0	0	0	0	0	110
01:00	0	34	12	0	1	0	0	0	0	0	0	0	0	0	0	47
02:00	0	17	6	0	0	0	0	0	0	0	0	0	0	0	0	23
03:00	0	16	5	0	0	0	0	0	0	0	0	0	0	0	0	21
04:00	0	19	6	0	0	0	0	0	0	0	0	0	0	0	0	25
05:00	0	28	11	0	0	0	0	0	0	1	0	0	0	0	0	40
06:00	0	80	30	0	2	0	0	2	0	0	1	0	0	0	0	115
07:00	0	187	72	0	8	0	0	4	0	0	0	1	0	0	0	272
08:00	0	331	126	0	16	0	0	10	0	0	0	0	0	0	0	483
09:00	0	447	171	0	15	0	0	12	0	1	0	1	0	0	0	647
10:00	0	517	198	0	24	0	0	19	0	1	0	1	0	1	0	761
11:00	0	594	226	0	22	0	0	16	0	0	0	0	0	0	0	858
12 PM	0	629	240	0	22	0	0	19	0	0	1	0	0	0	0	911
13:00	0	571	217	0	24	0	0	15	0	0	0	1	0	0	0	828
14:00	0	531	202	0	21	0	0	13	0	0	0	0	1	1	0	769
15:00	0	551	210	0	20	0	0	16	0	0	0	0	0	1	0	798
16:00	0	553	212	0	18	0	0	14	0	1	0	0	0	0	0	798
17:00	0	479	183	0	17	0	0	15	0	0	0	1	1	0	0	696
18:00	0	457	174	0	20	0	0	13	0	0	0	0	0	0	0	664
19:00	0	356	135	0	18	0	0	9	0	0	0	0	0	0	0	518
20:00	0	274	104	0	11	0	0	5	0	0	0	0	0	1	0	395
21:00	0	203	78	0	7	0	0	6	0	0	0	0	0	0	0	294
22:00	0	195	76	0	5	0	0	6	0	0	0	0	0	0	0	282
23:00	0	150	57	0	4	0	0	3	0	0	0	0	0	0	0	214
Total	0	7294	2780	0	278	0	0	200	0	4	2	5	2	4	0	10569
Percent	0.0%	69.0%	26.3%	0.0%	2.6%	0.0%	0.0%	1.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

716 S Sixth Ave Mount Vernon, NY 10550

NB													Lau			lacinica
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/13/1																
6	0	76	29	0	3	0	0	2	0	0	0	0	0	0	0	110
01:00	0	42	17	0	1	0	0	0	0	0	0	0	0	0	0	60
02:00	0	21	5	0	0	0	0	0	0	0	0	0	0	0	0	26
03:00	0	20	7	0	0	0	0	0	0	0	0	0	0	0	0	27
04:00	0	18	7	0	0	0	0	0	0	0	0	0	0	0	0	25
05:00	0	31	10	0	1	0	0	0	0	0	0	0	0	0	0	42
06:00	0	75	26	0	3	0	0	2	0	0	0	0	0	0	0	106
07:00	0	156	60	0	7	0	0	4	0	0	0	0	0	0	0	227
08:00	0	263	101	0	13	0	0	7	0	1	0	0	0	0	0	385
09:00	0	355	136	0	19	0	0	9	0	0	0	0	0	0	0	519
10:00	0	433	165	0	19	0	0	12	0	0	1	1	0	1	0	632
11:00	0	487	186	0	20	0	0	14	0	0	0	0	1	1	0	709
12 PM	0	486	171	0	14	0	0	16	0	0	1	0	0	2	19	709
13:00	5	532	140	0	13	0	0	13	0	0	1	0	0	0	24	728
14:00	1	523	137	0	10	0	0	16	0	4	0	0	1	1	28	721
15:00	1	476	132	0	3	0	0	10	1	2	0	0	0	1	22	648
16:00	1	452	128	1	7	1	0	5	0	1	0	0	0	2	16	614
17:00	2	375	119	0	7	0	0	4	0	0	0	0	0	1	9	517
18:00	0	321	85	0	5	0	0	3	0	0	0	0	0	0	9	423
19:00	0	255	83	0	10	0	0	2	0	0	0	0	0	0	4	354
20:00	0	159	48	0	6	0	0	3	0	0	0	0	0	0	5	221
21:00	0	137	47	0	9	1	0	0	0	0	0	0	0	0	3	197
22:00	0	57	22	1	7	0	0	0	0	0	0	0	0	0	0	87
23:00	0	29	7	0	2	0	0	0	0	0	0	0	0	0	0	38
Total	10	5779	1868	2	179	2	0	122	1	8	3	1	2	9	139	8125
Percent	0.1%	71.1%	23.0%	0.0%	2.2%	0.0%	0.0%	1.5%	0.0%	0.1%	0.0%	0.0%	0.0%	0.1%	1.7%	
Total	10	27743	10256	2	1059	2	0	743	10	23	19	12	6	19	139	40043
Percent	0.0%	69.3%	25.6%	0.0%	2.6%	0.0%	0.0%	1.9%	0.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.3%	

716 S Sixth Ave Mount Vernon, NY 10550

NB													Lui			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/14/1																
6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
01:00	0	14	8	0	2	0	0	0	0	0	0	0	0	0	0	24
02:00	0	14	4	0	1	0	0	1	0	0	0	0	0	0	0	20
03:00	0	8	4	0	3	1	0	0	0	0	0	0	0	0	1	17
04:00	0	9	3	2	5	0	0	1	1	0	0	0	0	0	2	23
05:00	0	34	20	0	5	0	0	1	0	0	1	0	0	0	1	62
06:00	0	130	57	0	9	0	0	1	0	1	0	0	0	1	3	202
07:00	2	398	139	5	20	1	0	4	1	2	0	0	1	0	24	597
08:00	1	464	186	5	24	2	0	16	1	5	0	0	0	0	24	728
09:00	3	365	152	2	21	3	0	20	0	2	0	0	0	1	32	601
10:00	0	325	149	1	30	2	0	19	0	1	0	0	0	2	26	555
11:00	0	340	153	3	28	1	0	15	1	1	0	0	0	0	19	561
12 PM	0	416	135	4	19	3	0	10	0	3	0	0	0	1	13	604
13:00	0	409	137	1	26	1	0	14	0	3	1	0	0	0	17	609
14:00	1	417	151	2	20	0	0	15	0	1	0	0	0	0	62	669
15:00	0	574	191	6	18	0	0	24	1	0	1	0	0	1	41	857
16:00	1	507	174	2	23	2	0	12	0	0	0	0	0	1	36	758
17:00	0	519	144	2	12	1	0	8	0	2	2	0	0	0	28	718
18:00	0	465	120	0	9	0	0	7	0	0	0	0	1	0	13	615
19:00	0	273	91	1	4	0	0	1	0	0	0	0	0	0	98	468
20:00	0	287	79	0	14	0	0	0	0	2	0	0	0	0	10	392
21:00	0	163	66	0	3	0	0	1	0	0	0	0	0	0	6	239
22:00	0	112	45	1	6	0	0	0	0	0	0	0	0	0	6	170
23:00	0	49	22	0	2	0	0	1	0	0	0	0	0	0	0	74
Total	8	6292	2230	37	304	17	0	171	5	23	5	0	2	7	462	9563
Percent	0.1%	65.8%	23.3%	0.4%	3.2%	0.2%	0.0%	1.8%	0.1%	0.2%	0.1%	0.0%	0.0%	0.1%	4.8%	

Mount Vernon, NY 10550

NB													Lau			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/15/1																
6	0	33	13	0	2	0	0	0	0	0	0	0	0	0	1	49
01:00	0	8	10	0	2	0	0	0	0	0	0	0	0	0	0	20
02:00	0	6	6	0	1	0	0	0	0	0	0	0	0	0	0	13
03:00	0	5	4	0	3	0	0	0	0	0	0	0	0	0	0	12
04:00	0	10	7	2	5	0	0	0	0	0	0	0	0	0	0	24
05:00	0	31	15	0	5	0	0	2	0	0	0	0	0	0	0	53
06:00	0	131	54	2	10	2	0	3	1	1	0	0	0	0	6	210
07:00	1	322	95	4	12	3	0	7	0	1	0	1	0	0	120	566
08:00	0	470	186	6	29	2	0	21	4	2	0	0	0	0	44	764
09:00	0	384	161	1	33	1	0	19	2	1	0	0	0	0	16	618
10:00	1	347	133	4	32	1	0	11	0	0	0	0	0	0	20	549
11:00	0	398	158	4	23	3	0	16	1	4	0	1	0	0	25	633
12 PM	1	409	142	6	35	0	0	12	0	3	1	0	1	0	26	636
13:00	0	444	148	3	31	2	0	13	0	3	1	0	0	4	24	673
14:00	4	471	188	3	41	0	0	7	0	0	2	0	1	0	35	752
15:00	2	561	214	6	32	1	0	21	1	4	2	0	0	2	44	890
16:00	2	501	193	5	32	0	0	14	0	2	1	0	2	0	39	791
17:00	0	460	144	0	7	0	0	15	0	3	2	0	0	0	59	690
18:00	3	481	146	0	11	0	0	14	1	3	0	0	0	0	22	681
19:00	1	399	137	0	8	0	0	14	1	0	0	0	0	0	18	578
20:00	1	305	115	0	11	0	0	2	0	0	1	0	0	0	10	445
21:00	0	201	76	0	2	0	0	4	1	0	0	0	0	0	4	288
22:00	0	105	56	0	5	0	0	0	0	0	0	0	0	0	5	171
23:00	0	51	34	0	1	0	0	0	0	0	0	0	0	0	4	90
Total	16	6533	2435	46	373	15	0	195	12	27	10	2	4	6	522	10196
Percent	0.2%	64.1%	23.9%	0.5%	3.7%	0.1%	0.0%	1.9%	0.1%	0.3%	0.1%	0.0%	0.0%	0.1%	5.1%	

Mount Vernon, NY 10550

NB													Lui			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/16/1																
6	0	29	17	0	5	0	0	0	0	0	0	0	0	0	0	51
01:00	0	14	8	1	2	0	0	0	0	0	0	0	0	0	0	25
02:00	0	9	8	0	1	0	0	0	0	0	0	0	0	0	0	18
03:00	0	7	11	0	3	0	0	0	0	0	0	0	0	0	1	22
04:00	0	11	16	1	4	0	0	1	0	0	0	0	0	0	0	33
05:00	0	21	18	0	4	1	0	0	0	0	0	0	0	0	1	45
06:00	1	105	60	3	8	0	0	3	1	0	0	0	0	1	10	192
07:00	0	415	158	6	22	1	0	10	1	2	0	0	0	0	16	631
08:00	1	460	182	5	34	2	0	11	0	3	2	0	0	0	34	734
09:00	0	414	164	8	27	2	0	15	1	0	0	0	0	1	15	647
10:00	2	332	144	0	29	1	0	7	0	1	0	0	0	0	14	530
11:00	3	406	164	9	25	1	0	15	0	3	0	0	0	1	23	650
12 PM	1	409	162	7	28	0	0	13	1	0	0	0	0	0	33	654
13:00	2	426	188	3	39	0	0	14	0	3	1	0	0	2	25	703
14:00	1	509	166	4	39	0	0	24	0	2	0	0	0	0	36	781
15:00	0	532	178	9	27	1	0	13	1	3	2	0	0	3	57	826
16:00	1	541	189	3	23	1	0	23	1	4	0	0	0	3	37	826
17:00	1	558	169	2	19	0	0	14	0	0	0	0	0	6	59	828
18:00	1	507	160	1	17	0	1	14	0	3	1	0	0	1	40	746
19:00	0	371	132	0	7	0	0	5	0	1	0	0	0	2	25	543
20:00	0	320	112	0	14	0	0	9	0	1	0	0	0	0	16	472
21:00	0	187	71	0	9	0	0	2	0	1	0	0	0	1	3	274
22:00	0	120	47	0	7	0	0	2	2	0	0	0	0	0	4	182
23:00	0	47	23	0	4	0	0	0	0	0	0	0	0	0	2	76
Total Percent	14 0.1%	6750 64.4%	2547 24.3%	62 0.6%	397 3.8%	10 0.1%	1 0.0%	195 1.9%	8 0.1%	27 0.3%	6 0.1%	0 0.0%	0 0.0%	21 0.2%	451 4.3%	10489

Mount Vernon, NY 10550

NB													Lati			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/17/1																
6	0	27	20	0	2	0	0	0	0	0	0	0	0	0	1	50
01:00	0	14	3	0	1	0	0	1	0	0	0	0	0	0	2	21
02:00	0	5	5	1	0	0	0	0	0	0	0	0	0	0	0	11
03:00	0	7	5	0	5	0	0	0	0	0	0	0	0	0	0	17
04:00	0	8	6	1	4	1	0	0	0	0	0	0	0	0	0	20
05:00	0	33	24	0	1	0	0	1	1	0	0	0	0	0	0	60
06:00	1	120	71	1	13	1	0	1	1	1	0	0	0	0	12	222
07:00	0	364	164	5	17	2	0	12	0	2	0	1	0	0	20	587
08:00	0	407	172	5	25	2	1	22	0	5	0	0	1	1	37	678
09:00	1	404	167	5	33	2	0	16	0	1	0	0	0	2	21	652
10:00	1	369	146	3	34	1	0	11	0	3	0	0	0	0	30	598
11:00	1	361	141	5	25	1	0	16	1	4	0	0	0	0	20	575
12 PM	1	438	193	3	37	1	0	8	0	6	1	1	0	1	16	706
13:00	0	432	166	4	31	1	0	15	0	1	1	0	0	0	28	679
14:00	1	448	177	5	39	1	0	14	1	0	0	0	0	2	23	711
15:00	0	551	198	4	25	1	0	24	1	1	2	0	2	0	46	855
16:00	0	531	174	4	24	1	0	14	0	3	0	0	1	1	28	781
17:00	1	564	164	0	28	0	0	12	0	4	1	0	0	0	24	798
18:00	2	463	149	0	12	1	1	8	0	1	1	0	0	1	24	663
19:00	0	387	142	0	17	0	0	9	0	0	0	0	0	0	9	564
20:00	0	327	116	0	4	0	0	2	0	0	0	0	0	0	8	457
21:00	0	217	79	0	14	0	0	0	0	0	0	0	0	0	2	312
22:00	0	148	63	0	4	0	0	0	0	0	0	0	0	1	2	218
23:00	0	67	26	0	1	0	0	1	0	0	0	0	0	0	0	95
Total	9	6692	2571	46	396	16	2	187	5	32	6	2	4	9	353	10330
Percent	0.1%	64.8%	24.9%	0.4%	3.8%	0.2%	0.0%	1.8%	0.0%	0.3%	0.1%	0.0%	0.0%	0.1%	3.4%	

Mount Vernon, NY 10550

NB													Lati			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/18/1						_										
6	0	41	17	0	3	0	0	0	0	0	0	0	0	0	1	62
01:00	0	23	10	1	1	0	0	0	0	0	0	0	0	0	1	36
02:00	0	12	8	0	1	0	0	0	0	0	0	0	0	0	0	21
03:00	0	10	3	0	1	0	0	1	0	0	0	0	0	0	0	15
04:00	0	14	12	1	7	0	0	0	2	0	0	0	0	0	1	37
05:00	0	35	23	0	5	0	0	1	0	0	0	0	0	0	1	65
06:00	0	110	66	2	11	2	0	3	0	1	0	0	0	0	1	196
07:00	0	396	144	6	22	1	0	14	0	0	0	0	0	0	21	604
08:00	0	395	209	4	27	2	0	18	1	1	1	0	0	0	28	686
09:00	3	343	191	5	34	2	0	17	0	2	0	1	0	0	26	624
10:00	0	367	145	5	22	1	0	24	1	1	0	0	0	1	25	592
11:00	1	437	144	4	31	1	0	31	1	0	0	0	0	0	28	678
12 PM	0	470	170	3	44	1	0	14	1	2	1	0	0	2	29	737
13:00	2	448	166	7	31	1	1	12	0	2	1	0	1	2	25	699
14:00	2	491	183	2	29	2	0	11	1	3	1	0	1	5	44	775
15:00	1	570	221	3	23	1	0	16	0	4	1	0	1	3	44	888
16:00	2	535	180	1	26	0	0	12	0	1	1	0	0	0	27	785
17:00	2	517	173	1	17	1	1	15	1	5	0	1	0	0	34	768
18:00	0	456	148	1	15	0	0	11	0	2	0	0	0	1	17	651
19:00	0	373	176	0	13	0	0	9	0	0	0	0	0	0	8	579
20:00	0	312	106	0	8	0	0	3	0	0	0	0	1	0	5	435
21:00	0	249	80	0	9	0	0	2	0	1	1	0	0	0	2	344
22:00	0	176	100	0	7	0	0	0	0	0	0	0	0	0	7	290
23:00	0	111	50	0	5	0	0	1	0	0	0	0	0	0	2	169
Total	13	6891	2725	46	392	15	2	215	8	25	7	2	4	14	377	10736
Percent	0.1%	64.2%	25.4%	0.4%	3.7%	0.1%	0.0%	2.0%	0.1%	0.2%	0.1%	0.0%	0.0%	0.1%	3.5%	

Mount Vernon, NY 10550

NB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/19/1																
6	0	65	25	0	5	0	0	0	0	0	0	0	0	0	0	95
01:00	0	28	16	0	2	0	0	0	0	0	0	0	0	0	0	46
02:00	0	23	12	0	1	0	0	1	0	0	0	0	0	0	3	40
03:00	0	9	12	0	3	0	0	0	0	0	0	0	0	0	1	25
04:00	0	9	15	1	3	0	0	0	0	0	0	0	0	0	1	29
05:00	0	16	9	0	5	0	0	2	0	0	0	0	0	0	2	34
06:00	0	55	24	0	8	0	0	1	0	0	0	0	0	0	0	88
07:00	0	152	73	1	13	1	0	3	1	0	0	0	0	0	2	246
08:00	0	253	125	0	21	0	0	7	0	0	0	0	0	0	7	413
09:00	1	407	165	0	17	0	0	8	1	0	0	0	0	0	10	609
10:00	1	430	185	2	25	0	0	9	1	1	0	0	0	0	18	672
11:00	1	553	203	4	16	1	1	16	0	1	1	0	0	0	32	829
12 PM	3	511	197	3	22	0	1	18	0	1	0	0	0	0	49	805
13:00	1	526	198	1	13	0	0	13	0	4	1	0	0	0	31	788
14:00	1	525	180	4	17	0	0	10	0	1	0	0	0	0	28	766
15:00	1	463	185	0	9	2	0	7	0	1	1	0	0	1	20	690
16:00	0	430	169	0	13	0	0	10	0	0	0	0	0	1	22	645
17:00	2	404	169	1	14	0	0	13	0	0	0	0	0	0	25	628
18:00	0	386	145	0	10	0	0	6	0	2	2	0	0	0	18	569
19:00	1	334	121	1	7	0	0	7	0	1	0	0	0	2	6	480
20:00	0	229	97	0	7	0	1	3	1	0	0	0	0	0	4	342
21:00	0	182	93	0	9	0	0	3	1	0	0	0	0	0	3	291
22:00	0	154	72	0	4	0	0	1	0	0	0	0	0	0	2	233
23:00	0	108	72	0	6	0	0	0	0	0	0	0	0	0	5	191
Total Percent	12 0.1%	6252 65.4%	2562 26.8%	18 0.2%	250 2.6%	4 0.0%	3 0.0%	138 1.4%	5 0.1%	12 0.1%	5 0.1%	0 0.0%	0 0.0%	4 0.0%	289 3.0%	9554

Mount Vernon, NY 10550

NB													Lui			
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/20/1						_										
6	0	75	40	0	3	0	0	0	0	0	0	0	0	0	1	119
01:00	0	36	15	0	3	0	0	0	0	0	0	0	0	0	1	55
02:00	0	16	16	0	1	0	0	0	0	0	0	0	0	0	0	33
03:00	0	15	9	0	1	0	0	0	0	0	0	0	0	0	0	25
04:00	0	8	10	0	3	0	0	0	0	0	0	0	0	0	0	21
05:00	0	13	10	0	4	0	0	0	0	0	0	0	0	0	0	27
06:00	0	35	25	0	3	0	0	0	0	0	0	0	0	0	2	65
07:00	0	80	40	0	4	0	0	0	0	0	0	0	0	0	1	125
08:00	0	141	75	1	4	0	0	3	0	1	0	0	0	0	5	230
09:00	0	278	126	0	11	1	0	1	1	0	0	0	0	1	8	427
10:00	0	353	145	0	11	0	0	5	0	0	0	0	0	0	8	522
11:00	0	367	189	1	12	0	0	5	0	1	0	0	0	0	8	583
12 PM	0	438	182	0	7	0	0	11	0	1	0	0	0	0	14	653
13:00	2	445	168	0	7	1	0	6	0	0	0	0	0	0	16	645
14:00	0	425	165	0	9	0	0	12	0	1	0	0	0	0	10	622
15:00	1	378	152	0	11	0	0	5	0	0	0	0	0	0	11	558
16:00	0	376	154	0	10	0	0	3	0	0	0	0	0	1	11	555
17:00	0	336	126	0	9	0	0	6	0	0	0	0	0	0	10	487
18:00	0	317	132	0	9	0	0	8	0	1	0	0	0	0	4	471
19:00	0	272	89	0	5	0	0	3	0	0	1	0	0	0	5	375
20:00	0	185	76	0	6	0	0	1	0	0	0	0	0	0	7	275
21:00	0	111	60	0	2	0	0	0	0	0	0	0	0	0	3	176
22:00	0	67	31	0	2	0	0	0	0	0	0	0	0	0	2	102
23:00	0	53	15	0	3	0	0	0	0	0	0	0	0	0	0	71
Total	3	4820	2050	2	140	2	0	69	1	5	1	0	0	2	127	7222
Percent	0.0%	66.7%	28.4%	0.0%	1.9%	0.0%	0.0%	1.0%	0.0%	0.1%	0.0%	0.0%	0.0%	0.0%	1.8%	

716 S Sixth Ave Mount Vernon, NY 10550

NB													Lau		.0000 01	
Start		Cars	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl		Not	
Time	Motor	Trailer	Long	Buses	6 Tire	Single	Single	Doubl	Doubl	Doubl	Multi	Multi	Multi	Bicycl	Class	Total
03/21/1			-													
6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	6
01:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	4
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
10:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
11:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
12 PM	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
13:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
14:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
15:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
16:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
17:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
18:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
19:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
20:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
21:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
22:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
23:00	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	10
Percent	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%	
Total	75	44230	17120	257	2252	79	8	1170	44	151	40	6	14	63	2591	68100
Percent	0.1%	64.9%	25.1%	0.4%	3.3%	0.1%	0.0%	1.7%	0.1%	0.2%	0.1%	0.0%	0.0%	0.1%	3.8%	



# Appendix

Description

Parking Surveys

### Parking Surveys

### August 17 & 19, 2017

Job Title: Hampshire Country Club Project<br/>Location: Mamaroneck NYWeather:TEAFFIC DATA COLLECTIONSDate: 8/17 Thurs.. 8/19 Sat., 2017DAY# 1 and 2914-302-6326 fax-914-629-6815 cell

	#					17, 2017 es 1-6 at time	es	
TIME	1		2		3	4	5	6
12:00pm	39	4	bus \	15	bus O	0	Z4	2
1:00pm	48	5	bus l	17	bus O	0	22	2
	#				0	19, 2017 es 1-6 at tim	es	
TIME	1		2	:	3	4	5	6
8:30pm	65	5	bus ()	12	bus Z	0	6	0

